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LOAD DIFFUSION FROM AN ADHESIVE-BONDED
STRINGER TO AN INFINITE SHEET

Thesis for the Degree of M. S.
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Lynn C. Lewis

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ABSTRACT

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by Lynn C. Lewis

The case considered here is that of a single stiffener of uniform cross-section and finite length, bonded to an infinite sheet by means of an elastic adhesive. The objective is to investigate the shear stress or shear flow in the adhesive layer, treating all members as linearly elastic. The interaction between the sheet and stiffener is idealized as a line loading. An integral equation in one variable is then formulated for the shear flow in the adhesive layer. This equation is solved in closed form for the case of a rigid sheet, but in general must be solved by numerical means. The solution reveals that the maximum stress concentration occurs at the loaded end of the stringer, with a smaller stress concentration appearing at the opposite end. These stress concentrations may be quite large when stiff adhesive layers are used. In this case the load transfer occurs mainly at the ends of the stringer. Thirty solutions have been tabulated together with instructions for interpolation.



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TO AN INFINITE SHEET

By

Lynn C. Lewis

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NOMENCLATURE

b	width of adhesive layer
q	shear flow in adhesive layer
t	thickness of sheet
u_1	displacement of stiffener
u_2	displacement of sheet
y	dimensionless axial coordinate
A	cross-sectional area of stiffener
E_1	Young's modulus for stiffener
E_2	Young's modulus for sheet
G	adhesive shear modulus
L	length of stiffener
P	external concentrated load
Q	dimensionless shear flow in the adhesive layer
T	axial force in stiffener
X, Y, Z	coordinate directions
β, γ	dimensionless parameters
η	thickness of adhesive layer
ν	Poisson's ratio for sheet
σ_X	sheet normal stress in X-direction
σ_Y	sheet normal stress in Y-direction
τ_{XY}	sheet shear stress
τ	shear stress in adhesive

I. INTRODUCTION

The use of sheet and stringer construction in airframe design has motivated considerable interest in a class of problems dealing with load diffusion from a stiffener into a sheet. This interest has been a result of the demands of the aerospace industry for lightweight, high-load-capacity structures.

Monolithic construction (sheet with integral stiffener) has been the subject of much investigation. The advent of chemical milling in the aerospace industry has made the monolithic case a very important and practical problem. Buell (1) investigated a semi-infinite sheet with a semi-infinite edge stiffener by means of a complex stress function for the sheet. Bescoter (2) considered an infinite sheet with a finite stiffener. He observed that the integral equation defining the shear flow between stiffener and sheet was formally identical with Prandtl's equation for the aerodynamic load distribution over a wing of finite span. This problem was solved approximately, using standard methods for the Prandtl equation. Koiter (3) investigated the case of an infinite sheet with a semi-infinite stiffener. He based his work on Bescoter's equation and presented a rigorous solution based on the application of Mellin transforms.

Riveting has also been a common means by which stiffeners have been attached to sheets. Bloom (4) investigated riveted sheet-and-stringer construction using the complex variable method of Muskhelishvili. He found that applying the method of compatible deformations to the sheet and stringer produces an infinite system of

coupled algebraic equations in terms of the unknown rivet loads, solvable by truncation. Budiansky and Wu (5) have also contributed to the riveted-stringer load diffusion problem.

With the development of better adhesives, a large proportion of the structural connections in aerospace structures have been accomplished by adhesive bonding. Not only does the use of adhesives produce more uniform load diffusion in the joint and better fatigue life, but in the case of closed structural systems the adhesive provides adequate sealing for pressurization, fuel reservoirs, etc. The stress distribution in the adhesive layer of cemented lap joints has been investigated by Goland and Reissner (6). Solutions were obtained for two limiting cases, i. e., where the cement layer has negligible effect on the joint flexibility; and where joint flexibility is mainly due to the adhesive layer. This and other adhesive joint literature is reviewed by Benson (7).

In view of the foregoing, the problem which we consider here is of fundamental technical importance in aerospace construction. This is the case of a single stiffener, of constant cross-section and finite length, bonded by means of an adhesive to a sheet of infinite extent. The system is loaded by the application of an external force, acting at one end of the stiffener and in a direction parallel to the longitudinal axis of the stiffener. Our primary objective is to determine the shear flow or shear stress in the adhesive layer in the longitudinal direction. (Benthem (9) has examined the shear flow in a direction transverse to this longitudinal axis, for the case of a bonded hat-shaped stiffener.) In the present problem, the longitudinal shear flow is found to be governed by a one-dimensional integral equation, which appears to be analytically intractable and is therefore solved by a direct numerical

procedure, for various combinations of the two dimensionless parameters governing the problem. Any bending moment introduced by the external force will be neglected, and the sheet, stringer, and adhesive layer are all assumed to be composed of linearly elastic materials. This neglect of bending implies that we are effectively splitting the stiffener into halves, one on each side of the sheet, with each side loaded by half the applied force. The sketches in the following section show the stiffener on one side of the sheet only, however,

Having made these assumptions we now proceed with the development of the governing equations.

II. MATHEMATICAL FORMULATION

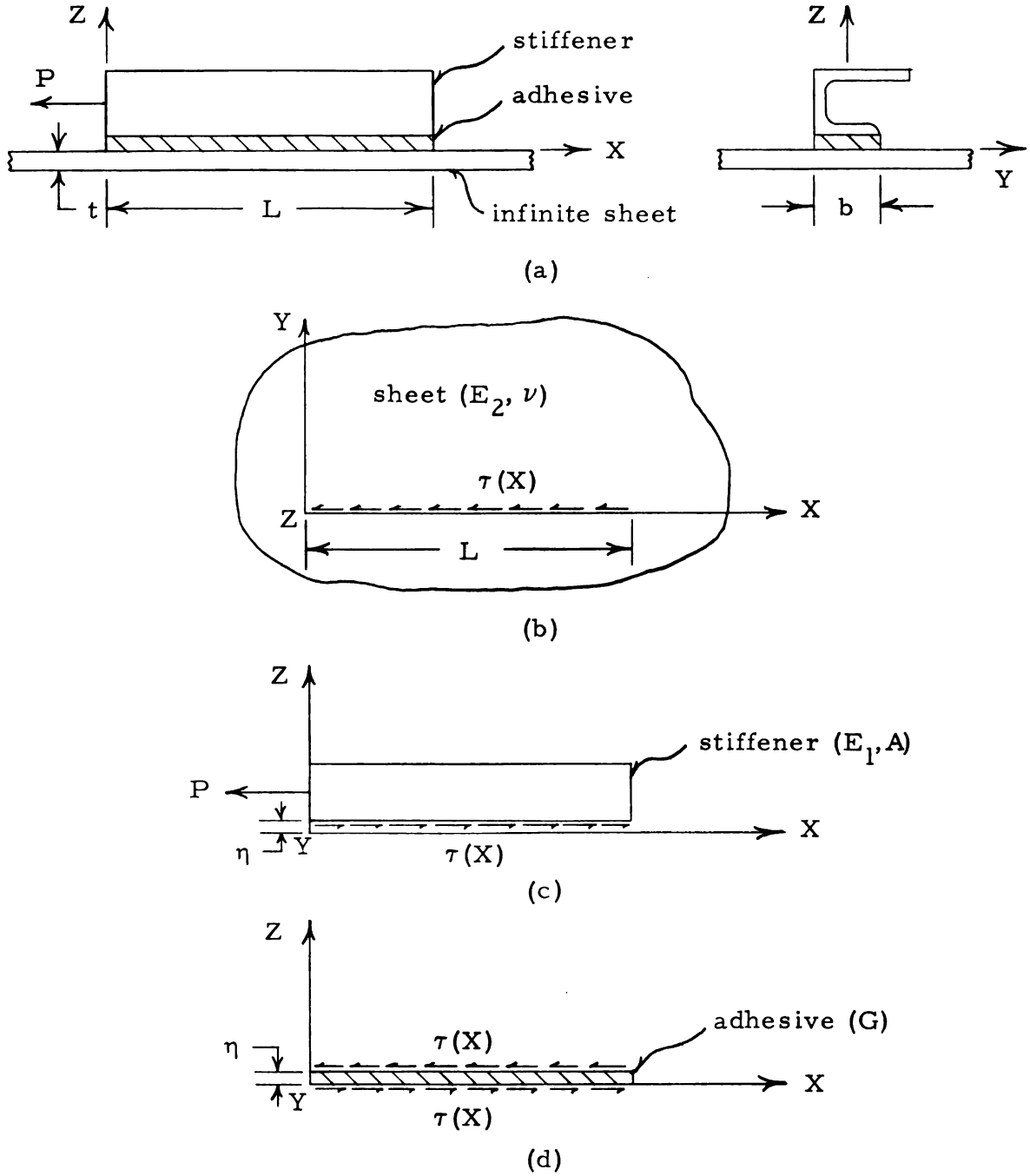


Figure 2-1. Infinite sheet with finite stiffener attached by means of an adhesive.

Referring to Figure 2-1, we consider an adhesive layer of small, constant thickness η , situated between a uniform reinforcing rib or stiffener of finite length L and an infinite elastic sheet. The adhesive layer is loaded by a system of continuous shear tractions τ , acting in the $Z = \eta$ plane and distributed from $X = 0$ to $X = L$. These stresses are applied by the stiffener, and their reactions act on the stiffener, both having been induced in the first instance by the external load P . The sheet is likewise loaded by a system of continuous tractions applied by the adhesive layer. These are distributed from $X = 0$ to $X = L$ in the plane $Z = 0$. The reactions to these stresses in turn act on the adhesive layer. It is now assumed that the adhesive layer is so thin (reflecting practice) that the adhesive stresses τ are uniform across the thickness of the adhesive layer. The complex three-dimensional stress disturbances at the ends $X = 0$ and $X = L$ of the adhesive layer are not examined in this study of the overall behavior of the joint.

Adopting a one-dimensional theory and neglecting bending, the adhesive shear strain may be expressed in the form

$$\gamma(X) = [u_2(X) - u_1(X)] / \eta \quad , \quad (2-1)$$

where $u_2(X)$ and $u_1(X)$ are the displacements of the sheet and stiffener respectively. The shear stress in the adhesive layer is given by

$$\tau(X) = G \gamma(X) \quad , \quad (2-2)$$

where G is the adhesive shear modulus. Axial equilibrium of a stiffener element of length dX requires that

$$dT(X)/dX + b\tau(X) = 0 \quad , \quad (2-3)$$

where $T(X)$ is the axial force in the stiffener and b is the width of the adhesive layer in the Y -direction; see Fig. 2-1(a). The strain in the stiffener is given by the expression

$$du_1/dX = T(X)/E_1 A \quad , \quad (2-4)$$

which integrates to

$$u_1(X) = (1/E_1 A) \int_0^X T(X) dX + u_{10} \quad , \quad (2-5)$$

where u_{10} is the rigid-body displacement of the stiffener.

The sheet displacement u_2 along the line $X = 0$ may be obtained by means of a routine integration of the known strain distribution in an infinite sheet subjected to a concentrated load acting in the X -direction, together with the principle of superposition. The stress distribution for a concentrated force P_0 per unit sheet thickness, applied at the origin and acting in the positive X -direction, is given by Timoshenko (8).

$$\sigma_X = - (P_0 X / 2\pi r^2) [(\nu + 3)/2 - (\nu + 1) Y^2 / r^2] \quad (2-6a)$$

$$\sigma_Y = - (P_0 X / 2\pi r^2) [(\nu - 1)/2 + (\nu + 1) Y^2 / r^2] \quad (2-6b)$$

$$\tau_{XY} = - (P_0 Y / 2\pi r^2) [(1 - \nu)/2 + (\nu + 1) X^2 / r^2] \quad (2-6c)$$

Here r is the radial coordinate with respect to the origin:

$$r = (X^2 + Y^2)^{1/2} .$$

The normal strains for an arbitrary point are obtained by applying Hooke's law for plane stress, and these are readily integrated to obtain the sheet displacements. Specializing the result to the line $X = 0$, we get

$$u_2(X) = u_{20} - [(1 + \nu) (3 - \nu) / 4\pi E_2] P_0 \ln |X| , \quad (2-7)$$

where u_{20} is a rigid-body displacement constant. Let the adhesive shear flow q be defined by

$$q(X) = \tau(X) \cdot b . \quad (2-8)$$

Replacing P_0 in Equation (2-7) by $-q(\xi)d\xi/t$ at $x = \xi$ and applying the principle of superposition, the sheet displacement for this problem becomes

$$u_2(X) = [(1 + \nu) (3 - \nu)/4\pi E_2 t] \int_0^L q(\xi) \ln |X - \xi| d\xi + u_{20}. \quad (2-9)$$

Integrating differential Equation (2-3), and using the boundary condition

$$T(0) = P, \quad ,$$

we get

$$T(X) = P - \int_0^X \tau b d\xi \quad . \quad (2-10)$$

Since $q = \tau b$ Equation (2-10) becomes

$$T(X) = P - \int_0^X q(\xi) d\xi \quad . \quad (2-11)$$

Since

$$T(L) = 0,$$

we get

$$P = \int_0^L q(\xi) d\xi \quad . \quad (2-12)$$

Substituting, Equation (2-11) becomes

$$T(X) = \int_0^L q(\xi) d\xi - \int_0^X q(\xi) d\xi \quad ,$$

which may be written

$$T(X) = \int_X^L q(\xi) d\xi \quad . \quad (2-13)$$

Substituting in Equation (2-5) we get

$$u_1(X) = (1/E_1 A) \int_0^X dx_1 \int_{x_1}^L q(x_2) dx_2 + u_{10} \quad . \quad (2-14)$$

where x_1 and x_2 are dummy variables. We can arbitrarily dispose of one of the two rigid-body displacements. Taking $u_1(0) = 0$, Equation (2-9) and (2-14) become

$$u_1(X) = (1/E_1 A) \int_0^X dx_1 \int_{x_1}^L q(x_2) dx_2 \quad , \quad (2-15a)$$

$$u_2(X) = [(1 + \nu) (3 - \nu) / 4\pi E_2 t] \int_0^L q(\xi) \ln |X - \xi| d\xi + u_{20} , \quad (2-15b)$$

Substituting Equations (2-15) into (2-1) and combining this result with Equations (2-2) and (2-8) we obtain an integral equation for q

$$\begin{aligned} q(X) = B_0 - (Gb/E_1 A \eta) \int_0^X dx_1 \int_{x_1}^L q(x_2) dx_2 \\ + [Gb (1 + \nu) (3 - \nu) / 4\pi E_2 t \eta] \int_0^L q(\xi) \ln |X - \xi| d\xi , \end{aligned} \quad (2-16)$$

where

$$B_0 = u_{20} Gb / \eta .$$

Making a change of variables to facilitate the use of dimensionless parameters, we define a dimensionless coordinate y and several dummy variables of integration by

$$y = X/L , \quad y_i = x_i/L \quad : \quad (i = 1, 2) , \quad u = \xi/L .$$

A dimensionless shear flow Q and constant C_0 are defined by

$$Q(y) = q(x) L/P \quad , \quad C_0 = B_0 L/P \quad .$$

Equation (2-16) becomes

$$\begin{aligned} Q(y) = C_0 - (GbL^2/\eta E_1 A) \int_0^y dy_1 \int_{y_1}^1 Q(y_2) dy_2 \\ + [GbL(1 + \nu)(3 - \nu)/\eta 4\pi E_2 t] \int_0^1 Q(u) [\ln |y - u| + \ln L] du \quad . \end{aligned} \quad (2-17)$$

Making the change of variables in Equation (2-12) we see that overall equilibrium requires

$$\int_0^1 Q(y) dy = 1 \quad . \quad (2-18)$$

Therefore (2-17) may be written

$$Q(y) = K - \beta^2 \int_0^y dy_1 \int_{y_1}^1 Q(y_2) dy_2 + \gamma^2 \int_0^1 Q(u) \ln |y - u| du \quad , \quad (2-19)$$

where

$$K = C_0 + \gamma^2 \ln L \quad , \quad (2-20)$$

and

$$\beta^2 = GbL^2/\eta E_1 A \quad , \quad (2-21a)$$

$$\gamma^2 = GbL(1 + \nu) (3 - \nu)/4\pi\eta E_2 t \quad . \quad (2-21b)$$

The dimensionless parameters β and γ are the fundamental physical parameters of this problem.

At $y = 0$, the first integral vanishes and we see that K may also be expressed in terms of $Q(0)$.

$$K = Q(0) - \gamma^2 \int_0^1 Q(u) \ln u \, du \quad . \quad (2-22)$$

Integrating the first integral in Equation (2-19) by parts with respect to y_1 , and applying Leibnitz's rule, the resulting equation, together with Equation (2-18), completely defines the function Q and the constant K .

$$\begin{aligned} Q(y) = K - \beta^2 y + \beta^2 \int_0^y (y - u) Q(u) \, du \\ + \gamma^2 \int_0^1 Q(u) \ln |y - u| \, du \end{aligned} \quad (2-23a)$$

$$1 = \int_0^1 Q(y) \, dy \quad (2-23b)$$

Equation (2-23a) would be a Volterra integral equation, except for the presence of the logarithmic integral. By substituting (2-23a) into (2-23b) we get

$$1 = K - \beta^2/2 + \beta^2 \int_0^1 \int_0^y (y - u) Q(u) du dy + \gamma^2 \int_0^1 \int_0^1 Q(u) \ln |y - u| du dy \quad . \quad (2-24)$$

This gives an explicit expression for eliminating K if desired. In the actual determination of Q , however, it proves to be simpler to avoid this procedure. The integral equation with K eliminated is therefore not given here.

Consider first the case where $\gamma = 0$. Equation (2-19) with $\gamma = 0$ can then be differentiated twice to produce

$$d^2 Q / dy^2 - \beta^2 Q = 0 \quad , \quad (2-25)$$

which has the solution

$$Q = B_1 \sinh \beta y + B_2 \cosh \beta y \quad .$$

This suggests that we assume Equation (2-19) or (2-23a) to have a solution of the form

$$Q(y) = A_1 \sinh \beta y + A_2 \cosh \beta y + A_3 y + A_4 \quad . \quad (2-26)$$

Substituting into Equation (2-23a) with $\gamma = 0$, carrying out the integration, and equating coefficients of like powers of y , we find

$$A_1 = -K \tanh \beta \quad ,$$

$$A_2 = K \quad ,$$

$$A_3 = A_4 = 0 \quad .$$

It follows that

$$Q(y) = K (\cosh \beta y - \tanh \beta \sinh \beta y) \quad . \quad (2-27)$$

Substituting this result into Equation (2-23b) and integrating we get

$$K = \beta \coth \beta \quad ,$$

and thus we have the complete solution in closed form for the case $\gamma = 0$:

$$Q(y) = \beta \coth \beta (\cosh \beta y - \tanh \beta \sinh \beta y) \quad . \quad (2-28)$$

This solution could form the basis for a perturbation-type analysis, if γ is small. The sheet is effectively rigid when $\gamma = 0$, if we retain finite adhesive flexibility; see Equation (2-21b). The solution for $\gamma = 0$ thus reduces to the well-known Volkersen adhesive lap joint theory for the special case of one rigid member; this theory is documented in Benson (7).

The solution of Equations (2-23) for nonzero γ is obtained numerically here. This is accomplished by dividing the dimensionless adhesive length of unity into N equal increments of length h . The integrals are evaluated numerically, thus generating a system of linear algebraic equations in Q and K . The relation between N and h is given by

$$N = 1/h \quad .$$

The process of integration is carried out by a routine application of the trapezoidal rule, with the exception of the two increments on either side of the logarithmic singularity at $y = u$. In this region the integration is carried out with Q assumed constant and equal to its value at the singularity. Details of the numerical work are presented in Appendix B.

III. RESULTS AND DISCUSSION

3.1. Computational Results

The shear stress distribution in an adhesive layer bonding a finite stringer to an infinite sheet may be computed by a direct numerical process. The system of linear equations discussed in Chapter II and Appendix B may be solved by means of a Jordan pivotal-condensation technique. A solution must be obtained for each set of values of the parameters β and γ .

Considering again the two expressions (2-21) defining the nondimensional physical parameters β and γ we write

$$\beta = (k_a / k_1)^{1/2} \quad (3-1a)$$

$$\gamma = (k_a / k_2)^{1/2} \quad (3-1b)$$

where,

$$k_a = GLb/\eta \quad , \quad (3-2a)$$

$$k_1 = E_1 A/L \quad , \quad (3-2b)$$

$$k_2 = 4\pi E_2 t/\lambda \quad , \quad (3-2c)$$

and

$$\lambda = (1 + \nu) (3 - \nu) = -\nu^2 + 2\nu + 3 \quad . \quad (3-3)$$

The parameters k_a , k_1 , and k_2 are the effective "spring constants" of the adhesive layer, stringer, and sheet respectively. The parameter β may then be interpreted as specifying the relative stiffnesses of the adhesive layer and the stringer, while γ represents the relative stiffnesses of the adhesive and the sheet.

The ratio of γ^2 to β^2 may be expressed by

$$\epsilon = \gamma^2 / \beta^2 = k_1 / k_2 \quad , \quad (3-4)$$

and may be interpreted as the relative stiffness of the stringer with respect to that of the sheet. Substituting Equations (3-2) into (3-4) we get

$$\epsilon = (\lambda / 4\pi) (E_1 / E_2) (A / tL) \quad . \quad (3-5)$$

Since the value of Poisson's ratio lies in the interval $0 < \nu < 0.5$, it follows that the value of λ lies in the interval $3 < \lambda < 3.75$. For purposes of estimating the size of ϵ , we will assume that $\lambda / \pi \approx 1$. Therefore, Equation (3-5) becomes

$$\epsilon \approx (1/4) (E_1/E_2) (A/tL) \quad . \quad (3-6)$$

In most situations $E_1 \approx E_2$, so that the parameter ϵ depends primarily upon the geometry of the stringer and the adjacent sheet. Therefore Equation (3-6) becomes

$$\epsilon \approx A/4tL \quad . \quad (3-7)$$

In selecting a range for β , the constants which define β were given values typical for aerospace applications. While some practical problems may have been excluded, it seems likely that the principal phenomena of the problem have been exposed by the present choice. In order to facilitate interpolation in the calculated results, successive values of β increase by a factor of $\sqrt{10}$, so that $\log \beta$ increases by uniform increments. The five values chosen are $\beta = (10)^{-1/2}$, $(10)^0$, $(10)^{1/2}$, $(10)^{2/2}$, and $(10)^{3/2}$.

The parameter β is here regarded as the fundamental one, but only because the case $\gamma = 0$ ("pure- β problem") offers an exact solution. It appears that γ is likely to be comparable to (or smaller than) β in many physical problems, although the exceptional "pure- γ problem" ($\beta = 0$) is also conceivable in practice. Once again, the choice of the range of γ is thought to expose the characteristic behavior of this parameter sufficiently well for the present purposes. In the present calculations, γ appears indirectly: $\epsilon = \gamma^2/\beta^2$ is used as the primary parameter, and the uniform intervals $\sqrt{\epsilon} = 0(0.25)1.0$ are found to

give a good spread of the curves $Q(y)$ for any fixed value of β . Thus interpolation in the tabulated results with respect to $\sqrt{\epsilon}$ is facilitated. Note that the size of ϵ indicates the importance of the second integral of Equation (2-23a) in the solution.

The curves in Figures 3-1, 3-2, 3-3, and 3-4 show representative calculated distributions of dimensionless adhesive shear flow Q , as a function of the dimensionless coordinate y , for various values of β and ϵ . (Since the curves were plotted by the computer, only Y is available.) The rest of the calculations performed are tabulated in Appendix A. In each figure β is held constant and three curves are given, for the cases $\sqrt{\epsilon} = 0, 0.5$, and 1.0 . (The corresponding values of γ are listed at the top of each Figure.)

The function $Q(y)$ may be interpreted as the stress concentration distribution in the adhesive since

$$Q = qL/P \quad , \quad (3-8)$$

where P/L is the average shear flow.

The shear flow Q may be interpreted in terms of two stressing mechanisms. Referring to the $\gamma = 0$ curves in Figures 3-1, 3-2, 3-3, and 3-4, we observe the asymmetric stress distribution characteristic of the pure- β mechanism. The curves show that the largest stress concentration is at the loaded end of the stringer ($y = 0$), and we observe a more-or-less exponential decline as y increases to one. For $\beta > 10$, the infinite-stringer case is effectively attained. When β is very large, all load is transferred in the immediate vicinity of the loaded end.

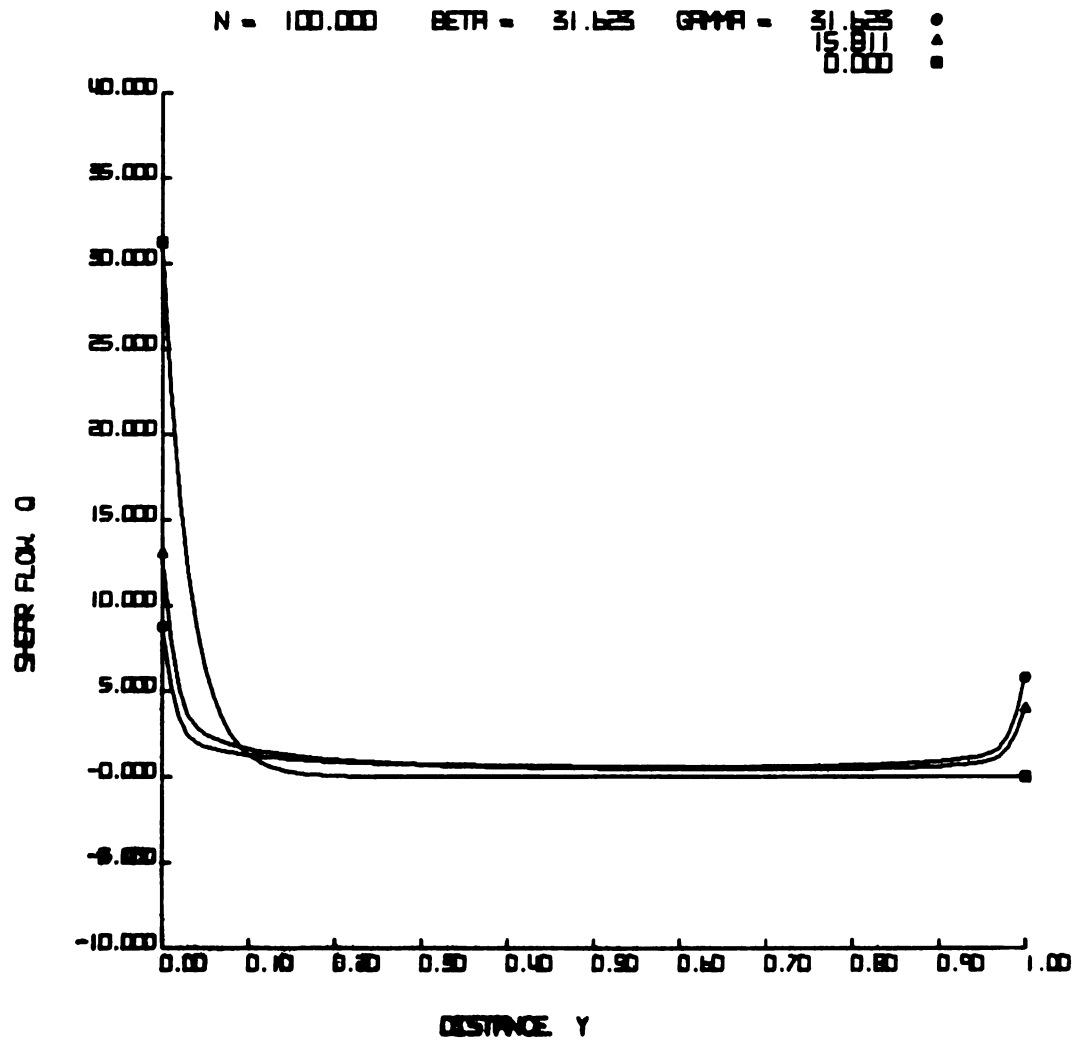


Figure 3-1. Shear flow Q for $\beta = 31.623$

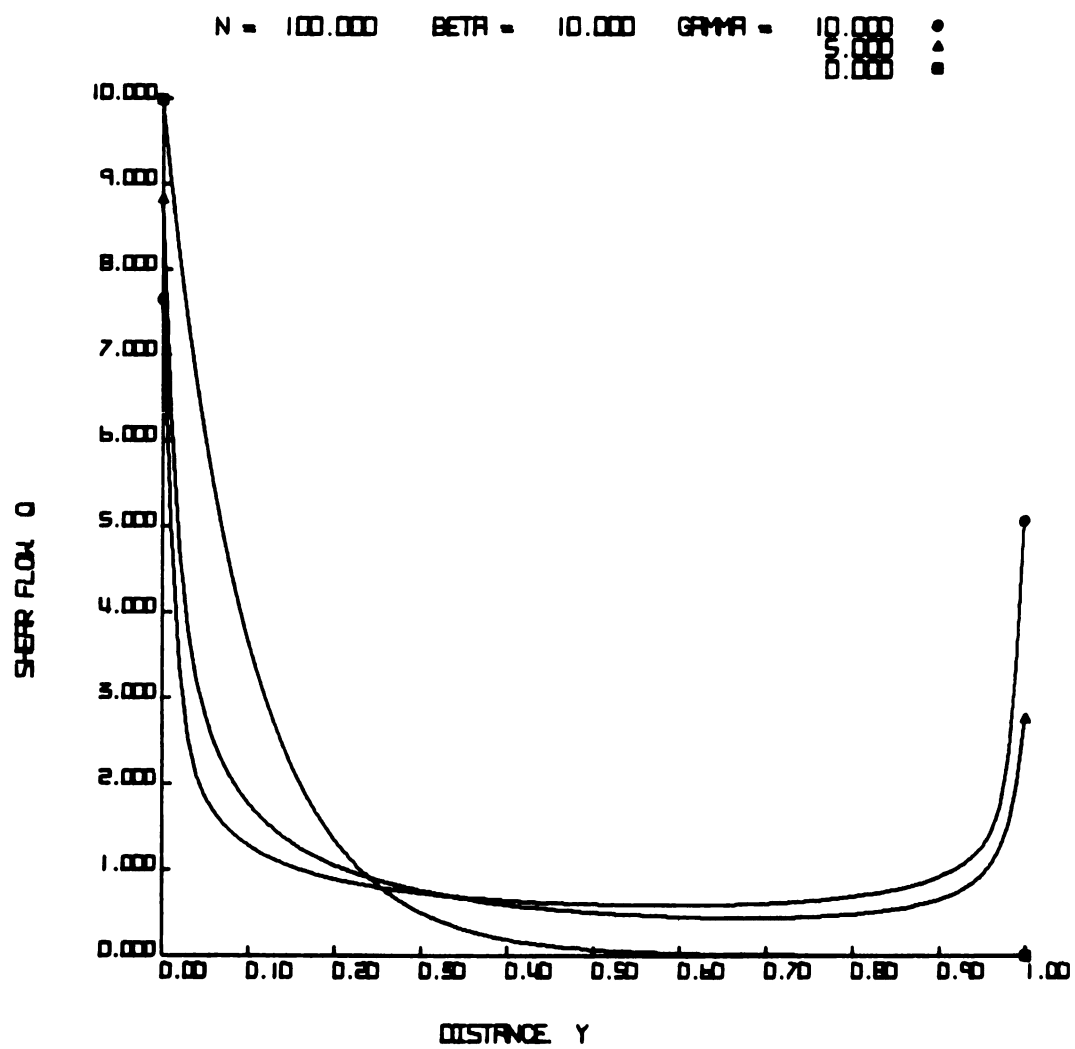


Figure 3-2. Shear flow Q for $\beta = 10.0$

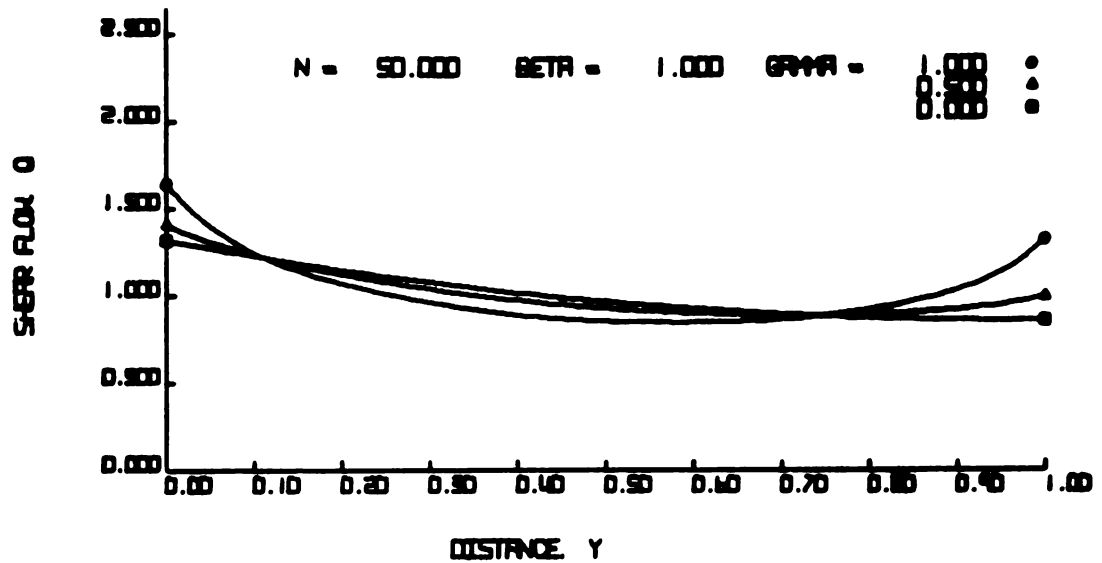


Figure 3-3. Shear flow Q for $\beta = 1.0$

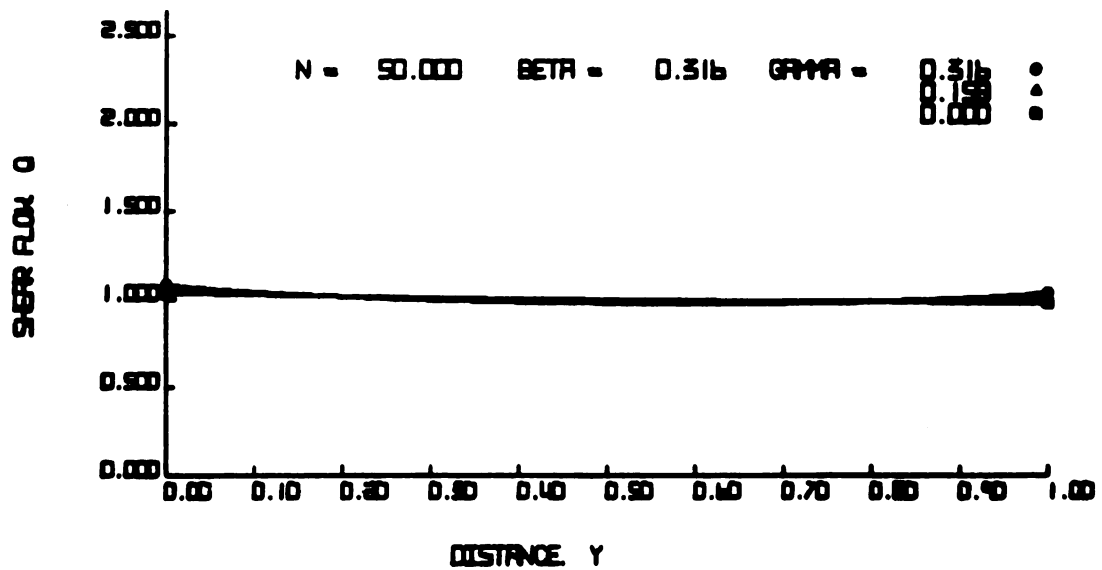


Figure 3-4. Shear flow Q for $\beta = 0.316$

The pure- β integral equation occurs when $\gamma = 0$ in Equation (2-23a). By direct differentiation, it may then be reduced to a Volkersen-type differential equation which may be solved exactly, as noted in Chapter II. From this solution, Equation (2-28), we see that the maximum shear flow occurs at $y = 0$ and may be expressed in terms of β .

$$Q_{\text{MAX}} = Q(0) = \beta \coth \beta \quad . \quad (3-9)$$

For $\beta > 3$

$$Q_{\text{MAX}} \approx \beta \quad . \quad (3-10)$$

Physically, the pure- β problem occurs when k_2 becomes large with respect to k_a ; i. e., when a flexible stringer is attached to a rigid sheet by means of a flexible adhesive.

Representative solutions for the pure- γ problem are shown in Figure 3-5. The shear flow distribution here is a symmetric one, with stress concentrations at each end of the joint. This limiting case is achieved when k_1 becomes very large in comparison with k_a , as in the case of a rigid stringer attached to a flexible sheet by means of a flexible adhesive. The symmetry of $Q(y)$ in this case may be deduced from physical considerations, by imagining a rigid stringer with loads $P/2$ at each end, both acting in the negative y -direction. In the case of

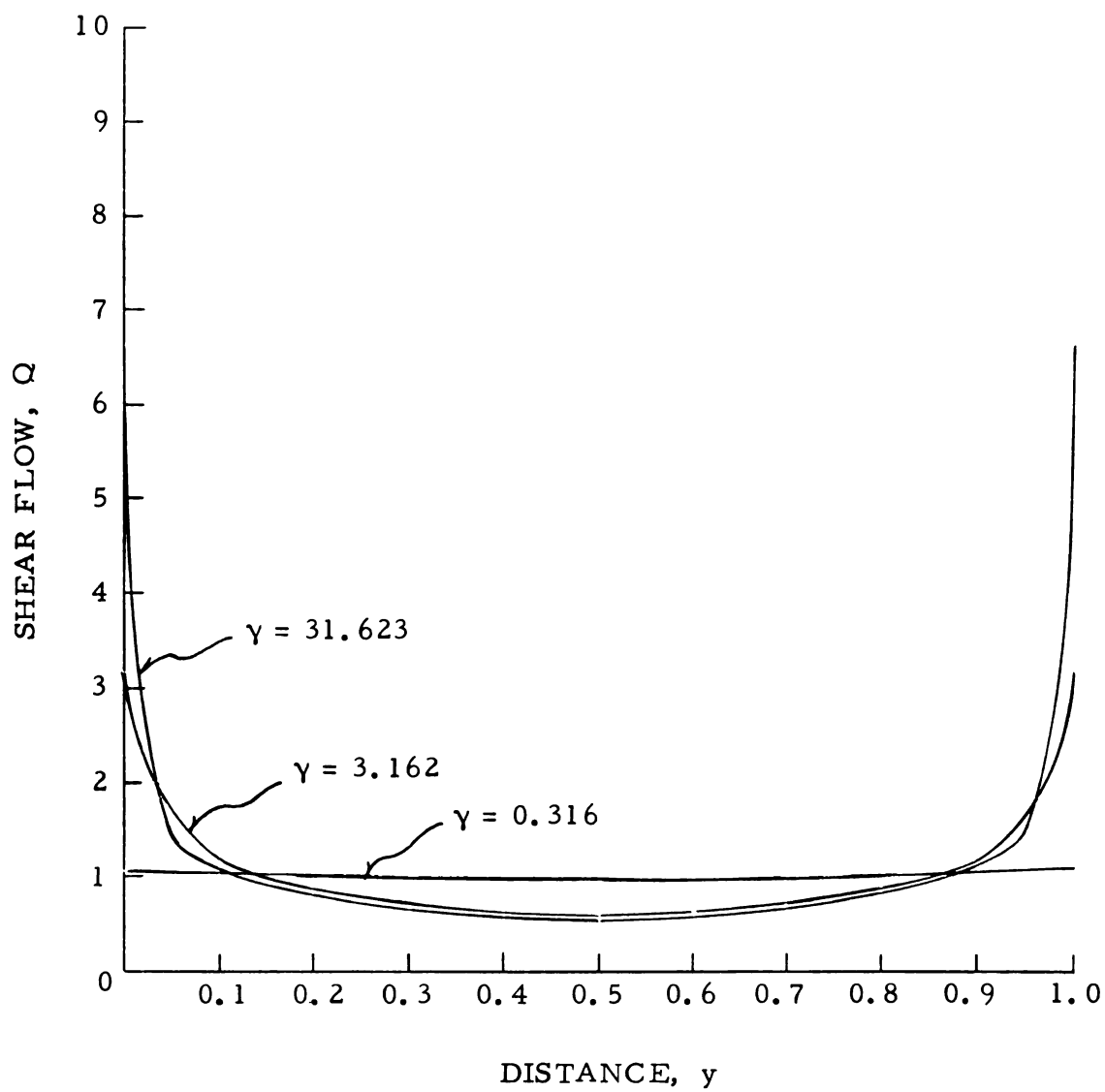


Figure 3-5. Shear flow Q for $\beta = 0$ and various values of γ .

large γ , the end stress concentrations indicate that load transfer is accomplished at the ends of the stiffener, for the most part.

Referring again to Figures 3-1, 3-2, 3-3, and 3-4, or to the integral Equation (2-23a), we see that the geometric parameter $\epsilon = \gamma^2/\beta^2$ controls the admixture of the two stressing mechanisms. As has been pointed out, ϵ specifies the relative stiffnesses of the stringer and sheet. On any one of these fixed- β plots, as ϵ increases an adhesive stress concentration appears at $y = 1$, accompanied by a significant change in the stress concentration at $y = 0$. This is the effect of adding the γ -mechanism. When β is large and the stresses are large, it is physically reasonable that an increase in ϵ (or γ), i. e., an increase in the flexibility of the sheet from the rigid state, should cause a decrease in stress concentration. This is observed for $\beta = 10^{3/2}$ and $\beta = 10$. For smaller values of β , the addition of the γ -mechanism causes an increase in the stress at $y = 0$. It is difficult to reason physically about the interplay of mechanisms involved in this phenomenon.

The maximum stress concentration occurs at $y = 0$ in all cases. As ϵ approaches unity and the γ -mechanism increases in importance, the stress distribution becomes more nearly symmetric with the interior of the joint experiencing a nearly uniform shear flow. As β decreases (more flexible adhesive or shorter stiffener), the magnitudes of the stress concentrations decrease, and in the limiting case when $\beta = \gamma = 0$, the shear flow becomes uniformly equal to unity: there is no stress concentration at all.

The monolithic case discussed in the Introduction is attained when the stringer becomes an integral part of the sheet, with the adhesive layer absent. We can achieve this condition in our present analysis

if we multiply both sides of the integral equation (2-23a) by η/G . Now let the adhesive shear modulus G approach infinity and the thickness η of the adhesive layer approach zero. Recalling the definition of ϵ in Equation (3-4), we have

$$\int_0^y (y - u) Q(u) du + \epsilon \int_0^1 Q(u) \ln |y - u| du - y = 0 \quad (3-11)$$

By differentiating this with respect to y , redefining the variables suitably, and letting $L \rightarrow \infty$, the present equation can be reduced to that studied by Koiter (3). Since computation has been carried out here only for finite L and finite adhesive flexibility, the two problems are radically different and cannot be compared.

The tables in Appendix A present 25 sets of values of the dimensionless shear flow Q as a function of y , for uniform intervals of $\sqrt{\epsilon}$ and $\log \beta$. This is done to facilitate two-way interpolation by the user. Five-point Lagrangian interpolation with respect to $\sqrt{\epsilon}$ and $\log \beta$ is thus possible for at least 51 values of y . Normally, only the values for $y = 0$ and $y = 1$ will be of interest in stress analysis. Appendix A also gives five values of the pure- γ solution, only three of which are plotted in Figure 3-5. Interpolation here is best done with respect to $\log \gamma$.

While no exhaustive error analysis has been carried out, the results are believed to be reasonably accurate up to $\beta = 10$, for all values of ϵ . Basing any conclusions on the established accuracy of the $\gamma = 0$ family of solutions is admittedly doubtful unless ϵ is of moderate size, unfortunately. This is because the numerical procedure for the

γ -integral is the one most open to question. One possible way to test the solution is to fit calculated functions $Q(y)$ accurately with polynomials in y , by sections. The γ -integral can then be evaluated analytically and the error in the integral equation for $Q(y)$ assessed.

Another way is to increase the number of intervals and examine the results for changes. The sequence $N = (\text{number of intervals}) = 75, 100, 125$ would be illuminating, and would also offer the possibility of a Richardson-type extrapolation for the critical value of primary interest: $Q(0)$. Improved accuracy, in principle, can be achieved for cases of large β and γ by using smaller intervals at the ends of the stringer, where the stress gradients are large. In the central regions where the variation of Q is small, a coarser interval is practicable to keep the total number of equations to be solved within reasonable limits. The available time has not permitted these refinements. The purpose of this thesis has been to offer a preliminary investigation of a very complex problem, and it is judged that the principal features of the physical problem have been adequately exposed.

3.2. Suggestions for Further Research

The principle of superposition allows us to use the solution of this fundamental problem for solving other problems of a more complex nature; for example, a compressive load on one end of the stringer and a tensile load on the other, two end tensile loads, etc.

Further research into the distribution of adhesive shear stresses in the transverse or Y -direction (throughout the width of the adhesive layer) should be considered. In the analysis presented here, the shear tractions applied to the sheet are assumed to be continuously distributed

along a line coincident with the X-axis (dimensionless y-axis). Line loading is essentially singular in nature as far as sheet behavior is concerned and violates the real physical problem. It is a necessary recourse in a first study, to avoid the complexities of a truly two-dimensional problem. However, the more realistic problem warrants investigation.

The effect of bending and the elastic stability of the system should also be considered; bending in particular is of great practical importance. The consideration of the region of influence of the stiffener in the sheet would be useful for application to problems where several stringers are attached to the same sheet. For example, such a study would reveal when it is necessary to consider stringer interaction. Another obvious area for further analysis is the finite sheet problem, where the stiffener acts throughout the entire length of the sheet.

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Appendix A. TABULATED VALUES OF THE ADHESIVE SHEAR FLOW

The following tables give values for dimensionless shear flow, $Q = Q(y)$. Each table provides this information for a specific value of the parameter β and five values of the parameter $\epsilon = \gamma^2/\beta^2$. The values of ϵ are invariably taken as follows: $\sqrt{\epsilon} = 0, 0.25, 0.50, 0.75, 1.0$. The values of β are: $10^{-1/2}, 10^0, 10^{1/2}, 10^{2/2}, 10^{3/2}$. In the actual tabulation, the values of γ corresponding to the given values of ϵ are shown as column headings. The variable y is the dimensionless axial coordinate, which ranges from 0 to 1, in some cases by intervals of 0.01 and in others of 0.02. (Since the tables are printed by computer, only a capital Y is available.)

The solution $Q(y)$ is also given when $\beta = 0$ ("pure- γ " case), for $\gamma = 10^{-1/2}, 10^0, 10^{1/2}, 10^{2/2}, 10^{3/2}$.

To interpolate, calculate β and γ from the physical problem, then form $\sqrt{\epsilon} = \gamma/\beta$ and $p = 2 (\log_{10} \beta) - 1$. For the tabulated values of β , $p = -2, -1, 0, 1, 2$, representing equal intervals. Interpolation with respect to p can then be carried out using Lagrangian interpolation coefficients, up to the five-point formula. Similarly, up to five-point interpolation is possible with respect to $\sqrt{\epsilon}$.

Interpolation in the pure- γ case may be carried out with respect to $\log_{10} \gamma$, by a scheme similar to that used for β .

TABLE 1. DIMENSIONLESS SHEAR FLOW Q FOR BETA = 31.623

GAMMA =	31.623	23.717	15.811	7.906	0.000
Y					
0.00000	8.74969	10.06320	13.02620	20.72240	31.23480
0.01000	5.51104	6.34326	8.23971	13.39070	22.79650
0.02000	3.45091	3.97269	5.17851	8.64844	16.63790
0.03000	2.45299	2.81514	3.65494	6.11511	12.14310
0.04000	1.99644	2.27688	2.91951	4.73624	8.86253
0.05000	1.76383	1.99707	2.52064	3.90004	6.46826
0.06000	1.61612	1.81728	2.25880	3.32697	4.72082
0.07000	1.50307	1.67961	2.05882	2.89734	3.44546
0.08000	1.40868	1.56504	1.89418	2.55770	2.51465
0.09000	1.32770	1.46701	1.75459	2.28085	1.83530
0.10000	1.25767	1.38230	1.63468	2.05073	1.33948
0.11000	1.19673	1.30859	1.53070	1.85661	0.97761
0.12000	1.14333	1.24396	1.43974	1.69089	0.71350
0.13000	1.09616	1.18681	1.35947	1.54795	0.52075
0.14000	1.05417	1.13588	1.28805	1.42353	0.38006
0.15000	1.01652	1.09016	1.22406	1.31439	0.27739
0.16000	0.98255	1.04887	1.16634	1.21799	0.20245
0.17000	0.95173	1.01135	1.11399	1.13231	0.14776
0.18000	0.92362	0.97711	1.06628	1.05575	0.10784
0.19000	0.89788	0.94571	1.02259	0.98699	0.07871
0.20000	0.87423	0.91681	0.98244	0.92498	0.05744
0.21000	0.85242	0.89012	0.94540	0.86882	0.04192
0.22000	0.83224	0.86540	0.91112	0.81777	0.03060
0.23000	0.81353	0.84243	0.87931	0.77122	0.02233
0.24000	0.79613	0.82104	0.84971	0.72864	0.01630
0.25000	0.77993	0.80108	0.82211	0.68958	0.01190
0.26000	0.76481	0.78242	0.79631	0.65365	0.00868
0.27000	0.75068	0.76494	0.77215	0.62052	0.00634
0.28000	0.73746	0.74854	0.74949	0.58991	0.00462
0.29000	0.72507	0.73314	0.72819	0.56156	0.00338
0.30000	0.71345	0.71865	0.70816	0.53526	0.00246
0.31000	0.70254	0.70500	0.68928	0.51081	0.00180
0.32000	0.69229	0.69214	0.67147	0.48804	0.00131
0.33000	0.68266	0.68002	0.65465	0.46681	0.00096
0.34000	0.67362	0.66858	0.63875	0.44699	0.00070
0.35000	0.66511	0.65777	0.62372	0.42844	0.00051
0.36000	0.65712	0.64757	0.60948	0.41107	0.00037
0.37000	0.64961	0.63794	0.59599	0.39478	0.00027
0.38000	0.64255	0.62884	0.58321	0.37949	0.00020
0.39000	0.63594	0.62025	0.57110	0.36512	0.00014
0.40000	0.62974	0.61214	0.55960	0.35161	0.00011
0.41000	0.62393	0.60448	0.54870	0.33888	0.00008
0.42000	0.61851	0.59727	0.53836	0.32689	0.00006
0.43000	0.61345	0.59047	0.52855	0.31558	0.00004
0.44000	0.60875	0.58408	0.51924	0.30491	0.00003
0.45000	0.60438	0.57807	0.51042	0.29483	0.00002
0.46000	0.60036	0.57244	0.50206	0.28531	0.00002
0.47000	0.59665	0.56717	0.49414	0.27631	0.00001
0.48000	0.59326	0.56225	0.48665	0.26780	0.00001
0.49000	0.59018	0.55768	0.47957	0.25975	0.00001
0.50000	0.58740	0.55344	0.47288	0.25214	0.00000

TABLE 1. (CONT.)

GAMMA = Y	31.623	23.717	15.811	7.906	0.000
0.51000	0.58492	0.54953	0.46658	0.24493	0.00000
0.52000	0.58274	0.54594	0.46065	0.23812	0.00000
0.53000	0.58086	0.54267	0.45508	0.23167	0.00000
0.54000	0.57927	0.53972	0.44987	0.22558	0.00000
0.55000	0.57797	0.53708	0.44501	0.21982	0.00000
0.56000	0.57697	0.53476	0.44048	0.21438	0.00000
0.57000	0.57627	0.53275	0.43630	0.20924	0.00000
0.58000	0.57587	0.53105	0.43245	0.20440	0.00000
0.59000	0.57577	0.52966	0.42893	0.19984	0.00000
0.60000	0.57598	0.52860	0.42574	0.19556	0.00000
0.61000	0.57652	0.52787	0.42288	0.19154	0.00000
0.62000	0.57738	0.52746	0.42036	0.18779	0.00000
0.63000	0.57857	0.52739	0.41818	0.18428	0.00000
0.64000	0.58012	0.52768	0.41633	0.18103	0.00000
0.65000	0.58202	0.52832	0.41484	0.17802	0.00000
0.66000	0.58430	0.52934	0.41370	0.17526	0.00000
0.67000	0.58698	0.53074	0.41292	0.17274	0.00000
0.68000	0.59007	0.53255	0.41253	0.17047	0.00000
0.69000	0.59360	0.53479	0.41252	0.16844	0.00000
0.70000	0.59759	0.53747	0.41293	0.16667	0.00000
0.71000	0.60208	0.54063	0.41376	0.16516	-0.00000
0.72000	0.60709	0.54430	0.41505	0.16391	0.00000
0.73000	0.61267	0.54851	0.41681	0.16294	-0.00000
0.74000	0.61886	0.55330	0.41909	0.16226	0.00000
0.75000	0.62571	0.55872	0.42191	0.16189	0.00000
0.76000	0.63328	0.56482	0.42533	0.16184	-0.00000
0.77000	0.64164	0.57166	0.42938	0.16215	-0.00000
0.78000	0.65086	0.57932	0.43414	0.16284	-0.00000
0.79000	0.66105	0.58789	0.43966	0.16394	0.00000
0.80000	0.67230	0.59745	0.44603	0.16550	-0.00000
0.81000	0.68475	0.60813	0.45335	0.16757	0.00000
0.82000	0.69855	0.62007	0.46173	0.17022	-0.00000
0.83000	0.71389	0.63345	0.47132	0.17352	0.00000
0.84000	0.73098	0.64846	0.48227	0.17758	-0.00000
0.85000	0.75010	0.66536	0.49481	0.18250	0.00000
0.86000	0.77159	0.68446	0.50919	0.18843	-0.00000
0.87000	0.79588	0.70615	0.52575	0.19559	-0.00000
0.88000	0.82350	0.73095	0.54488	0.20420	0.00000
0.89000	0.85514	0.75946	0.56714	0.21459	-0.00000
0.90000	0.89163	0.79249	0.59318	0.22720	0.00000
0.91000	0.93398	0.83098	0.62385	0.24262	0.00000
0.92000	0.98337	0.87609	0.66024	0.26167	-0.00000
0.93000	1.04138	0.92941	0.70389	0.28560	0.00000
0.94000	1.11141	0.99420	0.75783	0.31655	-0.00000
0.95000	1.20386	1.08007	0.82990	0.35873	0.00000
0.96000	1.35159	1.21639	0.94254	0.42165	-0.00000
0.97000	1.64577	1.48391	1.15567	0.52768	-0.00000
0.98000	2.29490	2.06698	1.60542	0.72605	-0.00000
0.99000	3.64182	3.26987	2.51878	1.10219	0.00000
1.00000	5.76524	5.16529	3.95587	1.68883	-0.00000

TABLE 2. DIMENSIONLESS SHEAR FLOW Q FOR BETA = 10.000

GAMMA = Y	10.000	7.500	5.000	2.500	0.000
0.00000	7.65098	8.09888	8.82164	9.68315	9.98752
0.01000	5.04775	5.49772	6.34954	7.84993	9.03746
0.02000	3.39134	3.83613	4.74805	6.56691	8.17777
0.03000	2.54212	2.94647	3.80785	5.64236	7.39986
0.04000	2.10601	2.45476	3.21701	4.93080	6.69595
0.05000	1.85383	2.14991	2.80928	4.35986	6.05900
0.06000	1.68357	1.93639	2.50588	3.88944	5.48264
0.07000	1.55437	1.77317	2.26812	3.49462	4.96110
0.08000	1.44982	1.64174	2.07518	3.15859	4.48918
0.09000	1.36247	1.53259	1.91468	2.86943	4.06215
0.10000	1.28813	1.44008	1.77865	2.61834	3.67573
0.11000	1.22401	1.36047	1.66161	2.39861	3.32608
0.12000	1.16810	1.29110	1.55969	2.20507	3.00969
0.13000	1.11886	1.23003	1.47002	2.03360	2.72339
0.14000	1.07512	1.17577	1.39044	1.88092	2.46433
0.15000	1.03598	1.12720	1.31927	1.74435	2.22991
0.16000	1.00072	1.08342	1.25521	1.62168	2.01779
0.17000	0.96877	1.04374	1.19722	1.51109	1.82585
0.18000	0.93969	1.00759	1.14446	1.41105	1.65216
0.19000	0.91308	0.97450	1.09623	1.32026	1.49500
0.20000	0.88866	0.94409	1.05198	1.23764	1.35279
0.21000	0.86616	0.91605	1.01123	1.16224	1.22411
0.22000	0.84537	0.89010	0.97358	1.09327	1.10766
0.23000	0.82610	0.86603	0.93869	1.03003	1.00230
0.24000	0.80821	0.84365	0.90628	0.97191	0.90695
0.25000	0.79155	0.82278	0.87609	0.91840	0.82068
0.26000	0.77602	0.80329	0.84791	0.86903	0.74261
0.27000	0.76152	0.78505	0.82156	0.82340	0.67197
0.28000	0.74795	0.76795	0.79687	0.78115	0.60805
0.29000	0.73524	0.75190	0.77370	0.74198	0.55021
0.30000	0.72333	0.73682	0.75193	0.70560	0.49787
0.31000	0.71216	0.72264	0.73143	0.67176	0.45051
0.32000	0.70167	0.70928	0.71213	0.64025	0.40766
0.33000	0.69182	0.69669	0.69392	0.61086	0.36888
0.34000	0.68256	0.68482	0.67672	0.58342	0.33379
0.35000	0.67387	0.67362	0.66047	0.55778	0.30204
0.36000	0.66570	0.66305	0.64511	0.53378	0.27331
0.37000	0.65803	0.65308	0.63057	0.51131	0.24731
0.38000	0.65083	0.64367	0.61681	0.49024	0.22378
0.39000	0.64408	0.63479	0.60377	0.47046	0.20250
0.40000	0.63776	0.62641	0.59143	0.45190	0.18323
0.41000	0.63184	0.61851	0.57973	0.43445	0.16580
0.42000	0.62632	0.61107	0.56864	0.41804	0.15003
0.43000	0.62117	0.60407	0.55813	0.40259	0.13576
0.44000	0.61638	0.59749	0.54819	0.38805	0.12285
0.45000	0.61195	0.59131	0.53876	0.37436	0.11116
0.46000	0.60785	0.58552	0.52985	0.36145	0.10059
0.47000	0.60409	0.58011	0.52142	0.34928	0.09102
0.48000	0.60065	0.57507	0.51345	0.33781	0.08236
0.49000	0.59753	0.57039	0.50593	0.32700	0.07453
0.50000	0.59472	0.56606	0.49885	0.31680	0.06744

TABLE 2. (CONT.)

GAMMA = Y	10.000	7.500	5.000	2.500	0.000
0.51000	0.59222	0.56207	0.49219	0.30718	0.06102
0.52000	0.59003	0.55842	0.48593	0.29811	0.05522
0.53000	0.58813	0.55510	0.48007	0.28957	0.04997
0.54000	0.58654	0.55211	0.47460	0.28153	0.04522
0.55000	0.58525	0.54945	0.46951	0.27396	0.04092
0.56000	0.58427	0.54712	0.46479	0.26684	0.03702
0.57000	0.58359	0.54511	0.46045	0.26016	0.03350
0.58000	0.58322	0.54343	0.45647	0.25390	0.03032
0.59000	0.58316	0.54209	0.45286	0.24804	0.02744
0.60000	0.58342	0.54108	0.44961	0.24257	0.02483
0.61000	0.58402	0.54041	0.44672	0.23748	0.02247
0.62000	0.58494	0.54008	0.44421	0.23276	0.02033
0.63000	0.58621	0.54012	0.44207	0.22840	0.01840
0.64000	0.58785	0.54052	0.44030	0.22440	0.01665
0.65000	0.58985	0.54130	0.43892	0.22075	0.01507
0.66000	0.59224	0.54247	0.43794	0.21745	0.01364
0.67000	0.59504	0.54405	0.43736	0.21451	0.01234
0.68000	0.59827	0.54606	0.43720	0.21191	0.01117
0.69000	0.60195	0.54852	0.43749	0.20967	0.01011
0.70000	0.60611	0.55146	0.43823	0.20779	0.00916
0.71000	0.61078	0.55490	0.43945	0.20629	0.00829
0.72000	0.61600	0.55887	0.44118	0.20517	0.00751
0.73000	0.62181	0.56343	0.44345	0.20445	0.00680
0.74000	0.62824	0.56860	0.44630	0.20414	0.00616
0.75000	0.63537	0.57445	0.44978	0.20427	0.00558
0.76000	0.64325	0.58102	0.45392	0.20486	0.00506
0.77000	0.65195	0.58839	0.45879	0.20595	0.00458
0.78000	0.66155	0.59663	0.46446	0.20758	0.00416
0.79000	0.67216	0.60585	0.47102	0.20977	0.00377
0.80000	0.68389	0.61614	0.47855	0.21260	0.00342
0.81000	0.69687	0.62765	0.48716	0.21612	0.00311
0.82000	0.71127	0.64051	0.49701	0.22040	0.00283
0.83000	0.72728	0.65494	0.50824	0.22552	0.00257
0.84000	0.74515	0.67114	0.52107	0.23160	0.00235
0.85000	0.76517	0.68941	0.53574	0.23876	0.00214
0.86000	0.78770	0.71010	0.55255	0.24713	0.00196
0.87000	0.81320	0.73365	0.57190	0.25692	0.00179
0.88000	0.84227	0.76063	0.59428	0.26833	0.00165
0.89000	0.87565	0.79178	0.62034	0.28164	0.00152
0.90000	0.91434	0.82808	0.65094	0.29719	0.00140
0.91000	0.95966	0.87088	0.68724	0.31543	0.00130
0.92000	1.01341	0.92207	0.73091	0.33692	0.00122
0.93000	1.07831	0.98450	0.78434	0.36240	0.00114
0.94000	1.15922	1.06300	0.85132	0.39291	0.00108
0.95000	1.26688	1.16704	0.93819	0.42989	0.00103
0.96000	1.42811	1.31761	1.05680	0.47549	0.00098
0.97000	1.71001	1.56382	1.23140	0.53320	0.00095
0.98000	2.26367	2.01442	1.51341	0.60940	0.00093
0.99000	3.34945	2.86288	1.99990	0.71701	0.00092
1.00000	5.06151	4.19819	2.75823	0.87388	0.00091

TABLE 3. DIMENSIONLESS SHEAR FLOW Q FOR BETA = 3.162

GAMMA = Y	3.162	2.372	1.581	0.791	0.000
0.00000	3.81696	3.57507	3.34636	3.20460	3.17206
0.02000	2.80297	2.78844	2.81461	2.90578	2.97840
0.04000	2.17631	2.28668	2.45079	2.66710	2.79666
0.06000	1.80743	1.96198	2.18331	2.46298	2.62610
0.08000	1.56959	1.73194	1.97231	2.28302	2.46605
0.10000	1.40161	1.55782	1.79915	2.12206	2.31587
0.12000	1.27504	1.42024	1.65355	1.97682	2.17494
0.14000	1.17549	1.30830	1.52901	1.84499	2.04272
0.16000	1.09484	1.21528	1.42115	1.72479	1.91866
0.18000	1.02810	1.13672	1.32681	1.61484	1.80229
0.20000	0.97196	1.06950	1.24365	1.51400	1.69312
0.22000	0.92413	1.01141	1.16987	1.42132	1.59072
0.24000	0.88296	0.96078	1.10407	1.33598	1.49469
0.26000	0.84725	0.91636	1.04513	1.25730	1.40463
0.28000	0.81608	0.87717	0.99214	1.18467	1.32019
0.30000	0.78875	0.84246	0.94438	1.11757	1.24104
0.32000	0.76472	0.81162	0.90123	1.05553	1.16685
0.34000	0.74355	0.78416	0.86218	0.99814	1.09732
0.36000	0.72489	0.75969	0.82680	0.94504	1.03219
0.38000	0.70848	0.73789	0.79475	0.89590	0.97118
0.40000	0.69409	0.71849	0.76570	0.85044	0.91406
0.42000	0.68155	0.70128	0.73942	0.80838	0.86060
0.44000	0.67071	0.68609	0.71569	0.76951	0.81057
0.46000	0.66146	0.67278	0.69431	0.73360	0.76379
0.48000	0.65372	0.66123	0.67514	0.70047	0.72007
0.50000	0.64742	0.65136	0.65805	0.66996	0.67922
0.52000	0.64252	0.64310	0.64293	0.64190	0.64109
0.54000	0.63900	0.63642	0.62971	0.61617	0.60553
0.56000	0.63687	0.63129	0.61833	0.59266	0.57239
0.58000	0.63613	0.62770	0.60874	0.57124	0.54153
0.60000	0.63682	0.62570	0.60092	0.55184	0.51285
0.62000	0.63902	0.62531	0.59487	0.53438	0.48621
0.64000	0.64281	0.62660	0.59061	0.51879	0.46152
0.66000	0.64831	0.62969	0.58818	0.50501	0.43868
0.68000	0.65568	0.63470	0.58764	0.49300	0.41759
0.70000	0.66513	0.64180	0.58910	0.48274	0.39817
0.72000	0.67692	0.65124	0.59267	0.47421	0.38035
0.74000	0.69139	0.66329	0.59852	0.46740	0.36404
0.76000	0.70900	0.67834	0.60688	0.46233	0.34920
0.78000	0.73033	0.69687	0.61801	0.45902	0.33574
0.80000	0.75618	0.71955	0.63227	0.45752	0.32364
0.82000	0.78761	0.74723	0.65013	0.45789	0.31282
0.84000	0.82614	0.78108	0.67218	0.46022	0.30326
0.86000	0.87396	0.82275	0.69923	0.46465	0.29491
0.88000	0.93439	0.87462	0.73236	0.47134	0.28774
0.90000	1.01287	0.94031	0.77313	0.48052	0.28172
0.92000	1.11903	1.02570	0.82386	0.49253	0.27683
0.94000	1.27202	1.14132	0.88825	0.50784	0.27304
0.96000	1.51320	1.30837	0.97313	0.52729	0.27035
0.98000	1.92877	1.57240	1.09338	0.55263	0.26874
1.00000	2.60860	1.99480	1.27715	0.58877	0.26820

TABLE 4. DIMENSIONLESS SHEAR FLOW Q FOR BETA = 1.000

GAMMA = Y	1.000	0.750	0.500	0.250	0.000
0.00000	1.64582	1.50956	1.40346	1.33606	1.31298
0.02000	1.51594	1.42717	1.35569	1.30929	1.29324
0.04000	1.42507	1.36654	1.31767	1.28531	1.27402
0.06000	1.35344	1.31631	1.28431	1.26282	1.25531
0.08000	1.29352	1.27267	1.25407	1.24148	1.23711
0.10000	1.24193	1.23391	1.22623	1.22110	1.21939
0.12000	1.19675	1.19901	1.20040	1.20160	1.20217
0.14000	1.15671	1.16733	1.17629	1.18288	1.18542
0.16000	1.12093	1.13838	1.15370	1.16491	1.16915
0.18000	1.08878	1.11181	1.13248	1.14764	1.15335
0.20000	1.05976	1.08735	1.11250	1.13103	1.13801
0.22000	1.03347	1.06479	1.09369	1.11507	1.12312
0.24000	1.00962	1.04394	1.07594	1.09972	1.10868
0.26000	0.98795	1.02467	1.05920	1.08497	1.09469
0.28000	0.96825	1.00685	1.04341	1.07079	1.08113
0.30000	0.95036	0.99037	1.02852	1.05718	1.06801
0.32000	0.93412	0.97516	1.01449	1.04411	1.05531
0.34000	0.91943	0.96114	1.00128	1.03157	1.04304
0.36000	0.90616	0.94824	0.98886	1.01956	1.03118
0.38000	0.89426	0.93641	0.97720	1.00806	1.01974
0.40000	0.88363	0.92561	0.96628	0.99706	1.00870
0.42000	0.87422	0.91579	0.95608	0.98655	0.99807
0.44000	0.86599	0.90693	0.94658	0.97652	0.98783
0.46000	0.85888	0.89899	0.93776	0.96698	0.97799
0.48000	0.85288	0.89195	0.92961	0.95790	0.96855
0.50000	0.84795	0.88580	0.92212	0.94929	0.95949
0.52000	0.84409	0.88052	0.91528	0.94114	0.95081
0.54000	0.84128	0.87611	0.90909	0.93345	0.94251
0.56000	0.83953	0.87256	0.90353	0.92620	0.93459
0.58000	0.83883	0.86987	0.89861	0.91940	0.92705
0.60000	0.83922	0.86805	0.89432	0.91305	0.91988
0.62000	0.84071	0.86710	0.89067	0.90715	0.91307
0.64000	0.84333	0.86705	0.88766	0.90168	0.90663
0.66000	0.84713	0.86792	0.88529	0.89665	0.90055
0.68000	0.85216	0.86972	0.88358	0.89206	0.89483
0.70000	0.85849	0.87251	0.88254	0.88792	0.88947
0.72000	0.86620	0.87631	0.88218	0.88421	0.88447
0.74000	0.87539	0.88118	0.88252	0.88096	0.87982
0.76000	0.88618	0.88718	0.88358	0.87815	0.87552
0.78000	0.89873	0.89440	0.88540	0.87579	0.87157
0.80000	0.91322	0.90292	0.88801	0.87389	0.86797
0.82000	0.92987	0.91287	0.89145	0.87247	0.86472
0.84000	0.94898	0.92439	0.89578	0.87152	0.86181
0.86000	0.97093	0.93766	0.90108	0.87107	0.85925
0.88000	0.99620	0.95295	0.90744	0.87113	0.85703
0.90000	1.02547	0.97057	0.91497	0.87174	0.85515
0.92000	1.05972	0.99101	0.92387	0.87292	0.85362
0.94000	1.10044	1.01498	0.93441	0.87475	0.85243
0.96000	1.15029	1.04375	0.94706	0.87732	0.85157
0.98000	1.21518	1.08013	0.96286	0.88085	0.85106
1.00000	1.31104	1.13275	0.98547	0.88623	0.85089

TABLE 5. DIMENSIONLESS SHEAR FLOW Q FOR BETA = 0.316

GAMMA = Y	0.316	0.237	0.158	0.079	0.000
0.00000	1.08068	1.06011	1.04519	1.03614	1.03311
0.02000	1.06867	1.05250	1.04071	1.03354	1.03113
0.04000	1.05961	1.04656	1.03699	1.03116	1.02919
0.06000	1.05184	1.04134	1.03361	1.02889	1.02730
0.08000	1.04490	1.03660	1.03047	1.02671	1.02544
0.10000	1.03859	1.03223	1.02751	1.02461	1.02362
0.12000	1.03281	1.02817	1.02471	1.02257	1.02185
0.14000	1.02747	1.02437	1.02205	1.02061	1.02012
0.16000	1.02251	1.02081	1.01952	1.01870	1.01843
0.18000	1.01790	1.01746	1.01710	1.01686	1.01677
0.20000	1.01361	1.01432	1.01479	1.01507	1.01516
0.22000	1.00962	1.01135	1.01259	1.01334	1.01359
0.24000	1.00589	1.00856	1.01049	1.01167	1.01206
0.26000	1.00243	1.00593	1.00849	1.01005	1.01057
0.28000	0.99921	1.00346	1.00658	1.00848	1.00913
0.30000	0.99622	1.00113	1.00475	1.00697	1.00772
0.32000	0.99345	0.99896	1.00302	1.00551	1.00635
0.34000	0.99090	0.99692	1.00137	1.00410	1.00502
0.36000	0.98856	0.99503	0.99981	1.00274	1.00373
0.38000	0.98642	0.99326	0.99833	1.00144	1.00249
0.40000	0.98448	0.99163	0.99693	1.00018	1.00128
0.42000	0.98274	0.99013	0.99561	0.99897	1.00011
0.44000	0.98119	0.98876	0.99437	0.99782	0.99898
0.46000	0.97982	0.98751	0.99321	0.99671	0.99790
0.48000	0.97865	0.98639	0.99213	0.99566	0.99685
0.50000	0.97766	0.98539	0.99112	0.99465	0.99584
0.52000	0.97685	0.98452	0.99020	0.99369	0.99487
0.54000	0.97623	0.98377	0.98935	0.99279	0.99395
0.56000	0.97580	0.98315	0.98858	0.99193	0.99306
0.58000	0.97555	0.98265	0.98790	0.99112	0.99221
0.60000	0.97550	0.98228	0.98728	0.99036	0.99140
0.62000	0.97563	0.98203	0.98676	0.98965	0.99063
0.64000	0.97596	0.98191	0.98630	0.98900	0.98990
0.66000	0.97649	0.98193	0.98593	0.98839	0.98921
0.68000	0.97722	0.98208	0.98565	0.98783	0.98856
0.70000	0.97816	0.98237	0.98544	0.98732	0.98795
0.72000	0.97932	0.98279	0.98533	0.98686	0.98738
0.74000	0.98070	0.98337	0.98529	0.98646	0.98685
0.76000	0.98232	0.98409	0.98535	0.98611	0.98636
0.78000	0.98419	0.98497	0.98550	0.98581	0.98590
0.80000	0.98632	0.98602	0.98575	0.98556	0.98549
0.82000	0.98872	0.98725	0.98610	0.98537	0.98512
0.84000	0.99143	0.98866	0.98655	0.98523	0.98478
0.86000	0.99448	0.99028	0.98711	0.98515	0.98449
0.88000	0.99789	0.99212	0.98780	0.98513	0.98423
0.90000	1.00172	0.99421	0.98862	0.98518	0.98401
0.92000	1.00604	0.99659	0.98959	0.98529	0.98384
0.94000	1.01097	0.99932	0.99074	0.98547	0.98370
0.96000	1.01669	1.00251	0.99210	0.98574	0.98360
0.98000	1.02363	1.00639	0.99380	0.98612	0.98354
1.00000	1.03339	1.01185	0.99621	0.98671	0.98352

TABLE 6. DIMENSIONLESS SHEAR FLOW Q FOR BETA = 0.000

GAMMA = Y	31.623	10.000	3.162	1.000	0.316
0.00000	6.63950	5.81699	3.16707	1.41909	1.04900
0.01000	4.20694	3.86110	2.55542	1.35704	1.04332
0.02000	2.66511	2.62210	2.15887	1.31255	1.03904
0.03000	1.92244	1.99087	1.90015	1.27618	1.03535
0.04000	1.58600	1.66963	1.71613	1.24482	1.03204
0.05000	1.41701	1.48585	1.57644	1.21707	1.02901
0.06000	1.31113	1.36296	1.46576	1.19213	1.02620
0.07000	1.23075	1.27040	1.37544	1.16947	1.02358
0.08000	1.16396	1.19596	1.30012	1.14873	1.02112
0.09000	1.10692	1.13411	1.23626	1.12962	1.01880
0.10000	1.05781	1.08177	1.18136	1.11194	1.01660
0.11000	1.01531	1.03688	1.13365	1.09551	1.01452
0.12000	0.97828	0.99797	1.09179	1.08020	1.01254
0.13000	0.94577	0.96390	1.05478	1.06589	1.01066
0.14000	0.91701	0.93384	1.02182	1.05250	1.00887
0.15000	0.89138	0.90710	0.99230	1.03993	1.00717
0.16000	0.86841	0.88317	0.96573	1.02812	1.00554
0.17000	0.84770	0.86163	0.94170	1.01702	1.00399
0.18000	0.82896	0.84216	0.91988	1.00656	1.00251
0.19000	0.81192	0.82448	0.90000	0.99670	1.00109
0.20000	0.79638	0.80837	0.88184	0.98741	0.99974
0.21000	0.78216	0.79364	0.86520	0.97865	0.99845
0.22000	0.76911	0.78015	0.84992	0.97038	0.99722
0.23000	0.75712	0.76775	0.83586	0.96257	0.99605
0.24000	0.74608	0.75635	0.82291	0.95521	0.99493
0.25000	0.73590	0.74583	0.81095	0.94827	0.99387
0.26000	0.72649	0.73613	0.79991	0.94172	0.99286
0.27000	0.71780	0.72717	0.78971	0.93555	0.99189
0.28000	0.70976	0.71888	0.78027	0.92974	0.99098
0.29000	0.70233	0.71123	0.77154	0.92428	0.99012
0.30000	0.69545	0.70414	0.76346	0.91915	0.98930
0.31000	0.68910	0.69760	0.75599	0.91433	0.98852
0.32000	0.68322	0.69156	0.74910	0.90982	0.98780
0.33000	0.67780	0.68598	0.74273	0.90561	0.98711
0.34000	0.67281	0.68084	0.73687	0.90169	0.98647
0.35000	0.66821	0.67612	0.73148	0.89804	0.98587
0.36000	0.66400	0.67179	0.72653	0.89466	0.98531
0.37000	0.66015	0.66783	0.72201	0.89155	0.98479
0.38000	0.65664	0.66423	0.71790	0.88868	0.98432
0.39000	0.65346	0.66096	0.71417	0.88607	0.98388
0.40000	0.65060	0.65802	0.71081	0.88370	0.98348
0.41000	0.64804	0.65540	0.70781	0.88158	0.98312
0.42000	0.64578	0.65307	0.70516	0.87968	0.98280
0.43000	0.64380	0.65105	0.70284	0.87802	0.98252
0.44000	0.64211	0.64930	0.70085	0.87659	0.98228
0.45000	0.64068	0.64784	0.69918	0.87538	0.98207
0.46000	0.63952	0.64665	0.69782	0.87439	0.98190
0.47000	0.63862	0.64572	0.69677	0.87362	0.98177
0.48000	0.63798	0.64507	0.69601	0.87308	0.98168
0.49000	0.63760	0.64467	0.69557	0.87275	0.98162
0.50000	0.63747	0.64454	0.69542	0.87264	0.98160

TABLE 6. (CONT.)

GAMMA = Y	31.623	10.000	3.162	1.000	0.316
0.51000	0.63760	0.64467	0.69557	0.87275	0.98162
0.52000	0.63798	0.64507	0.69601	0.87308	0.98168
0.53000	0.63862	0.64572	0.69677	0.87362	0.98177
0.54000	0.63952	0.64665	0.69782	0.87439	0.98190
0.55000	0.64068	0.64784	0.69918	0.87538	0.98207
0.56000	0.64211	0.64930	0.70085	0.87659	0.98228
0.57000	0.64380	0.65105	0.70284	0.87802	0.98252
0.58000	0.64578	0.65307	0.70516	0.87968	0.98280
0.59000	0.64804	0.65540	0.70781	0.88158	0.98312
0.60000	0.65060	0.65802	0.71081	0.88370	0.98348
0.61000	0.65346	0.66096	0.71417	0.88607	0.98388
0.62000	0.65664	0.66423	0.71790	0.88868	0.98432
0.63000	0.66015	0.66783	0.72201	0.89155	0.98479
0.64000	0.66400	0.67179	0.72653	0.89466	0.98531
0.65000	0.66821	0.67612	0.73148	0.89804	0.98587
0.66000	0.67281	0.68084	0.73687	0.90169	0.98647
0.67000	0.67780	0.68598	0.74273	0.90561	0.98711
0.68000	0.68322	0.69156	0.74910	0.90982	0.98780
0.69000	0.68910	0.69760	0.75599	0.91433	0.98852
0.70000	0.69545	0.70414	0.76346	0.91915	0.98930
0.71000	0.70233	0.71123	0.77154	0.92428	0.99012
0.72000	0.70976	0.71888	0.78027	0.92974	0.99098
0.73000	0.71780	0.72717	0.78971	0.93555	0.99189
0.74000	0.72649	0.73613	0.79991	0.94172	0.99286
0.75000	0.73590	0.74583	0.81095	0.94827	0.99387
0.76000	0.74608	0.75635	0.82291	0.95521	0.99493
0.77000	0.75712	0.76775	0.83586	0.96257	0.99605
0.78000	0.76911	0.78015	0.84992	0.97038	0.99722
0.79000	0.78216	0.79364	0.86520	0.97865	0.99845
0.80000	0.79638	0.80837	0.88184	0.98741	0.99974
0.81000	0.81192	0.82448	0.90000	0.99670	1.00109
0.82000	0.82896	0.84216	0.91988	1.00656	1.00251
0.83000	0.84770	0.86163	0.94170	1.01702	1.00399
0.84000	0.86841	0.88317	0.96573	1.02812	1.00554
0.85000	0.89138	0.90710	0.99230	1.03993	1.00717
0.86000	0.91701	0.93384	1.02182	1.05250	1.00887
0.87000	0.94577	0.96390	1.05478	1.06589	1.01066
0.88000	0.97828	0.99797	1.09179	1.08020	1.01254
0.89000	1.01531	1.03688	1.13365	1.09551	1.01452
0.90000	1.05781	1.08177	1.18136	1.11194	1.01660
0.91000	1.10692	1.13411	1.23626	1.12962	1.01880
0.92000	1.16396	1.19596	1.30012	1.14873	1.02112
0.93000	1.23075	1.27040	1.37544	1.16947	1.02358
0.94000	1.31113	1.36296	1.46576	1.19213	1.02620
0.95000	1.41701	1.48585	1.57644	1.21707	1.02901
0.96000	1.58600	1.66963	1.71613	1.24482	1.03204
0.97000	1.92244	1.99087	1.90015	1.27618	1.03535
0.98000	2.66511	2.62210	2.15887	1.31255	1.03904
0.99000	4.20694	3.86110	2.55542	1.35704	1.04332
1.00000	6.63950	5.81699	3.16707	1.41909	1.04900

Appendix B. NUMERICAL SOLUTION OF THE INTEGRAL EQUATION FOR Q

Writing Equation (2-23a) in the form

$$Q(y) = K - \beta^2 y + \beta^2 I_1(y) + \gamma^2 I_2(y) \quad , \quad (B1)$$

where

$$I_1(y) = \int_0^y (y - u) Q(u) du \quad (B2)$$

$$I_2(y) = \int_0^1 Q(u) \ln |y - u| du \quad (B3)$$

we apply the trapezoidal rule for N equal subintervals of length

$h = 1/N$. From Equation (B2) we have in general

$$I_1(y_n) = (h^2/2) \left[nQ_0 + 2 \sum_{p=1}^{n-1} (n - p) Q_p \right] \quad (B4)$$

where

$$y_n = u_n = nh \quad ,$$

and

$$Q_i = Q(y_i) \quad .$$

To facilitate treatment of the logarithmic singularity write Equation (B3) as the sum of four integrals

$$I_2(y_n) = I_{21} + I_{22} + I_{23} + I_{24} \quad ,$$

where

$$I_{21} = \int_0^{y_{n-1}} F du \quad , \quad I_{22} = \int_{y_{n-1}}^{y_n} F du \quad ,$$

$$I_{23} = \int_{y_n}^{y_{n+1}} F du \quad , \quad I_{24} = \int_{y_{n+1}}^{y_N} F du \quad ,$$

and

$$F(u, y_n) = Q(u) \ln |y_n - u| \quad .$$

Note that $y_N = 1$.

Since the singularity occurs when $u = y_n$, it seems appropriate to give the greatest weight to the function $Q(u)$ at $u = y_n$. Thus in applying the

Theorem of the Mean to this integral we extract the function $Q(u)$ and let Q take on the value $Q(y_n) = Q_n$ in the expressions for I_{22} and I_{23} . The rest of the integrand is integrable in closed form, with the result that

$$I_{22} + I_{23} = 2hQ_n (\ln h - 1) \quad .$$

The expressions for I_{21} and I_{24} may be treated in the usual manner; in general, the quadrature formula for $I_2(y_n)$ becomes

$$\begin{aligned} I_2(y_n) = & 2hQ_n (\ln h - 1) + (h/2) \{ Q_0 \ln(hn) \\ & + 2 \sum_{p=1}^{n-2} Q_p \ln[h(n-p)] + Q_{n-1} \ln h \\ & + Q_{n+1} \ln h + \sum_{p=n+2}^{N-1} Q_p \ln[h(p-n)] + Q_N \ln(1-hn) \} . \end{aligned}$$

(B5)

Special versions of this expression must be written when $y_n = y_0, y_1, y_{N-1}$, and y_N , but these all follow the general pattern described. At this point we have the capacity to approximate Equation (B1) as a system of $N + 1$ linear equations in the $N + 2$ unknowns $Q_0, Q_1, Q_2, \dots, Q_N$, K , by writing Equation (B1) successively at $y_0 = 0, y_1 = h, y_2 = 2h, \dots, y_N = Nh = 1$.

The values of Q_n must also satisfy the equilibrium condition (2-23b), which furnishes the necessary $(N + 2)$ th equation

$$1 = \int_0^1 Q(y) dy \quad , \quad (B6)$$

From Equation (B6), applying the two-point quadrature formula we have

$$1 = (h/2)Q_0 + h \sum_{p=1}^{N-1} Q_p + (h/2) Q_N \quad . \quad (B7)$$

Combining Equation (B1) and (B7), we have a system of linear algebraic equations in Q_n and k , here written with all specialized forms given in detail:

$$(B_1 - 1) Q_0 + B_3 Q_1 + \sum_{p=2}^{N-1} V_{0p} Q_p + K = 0 \quad , \quad (B8a)$$

$$B_2 Q_0 + (2B_1 - 1) Q_1 + B_3 Q_2 + \sum_{p=3}^{N-1} V_{1p} Q_p$$

$$+ (V_{1N}/2) Q_N + K = \beta^2 h \quad , \quad (B8b)$$

$$(U_{20}/2) Q_0 + (2B_2 + B_3) Q_1 + (2B_1 - 1) Q_2$$

$$+ B_3 Q_3 + \sum_{p=4}^{N-1} V_{2p} Q_p + (V_{2N}/2) Q_N$$

$$+ K = \beta^2 h \cdot 2 \quad , \quad (B8c)$$

$$(U_{n0}/2) Q_0 + \sum_{p=1}^{n-2} U_{np} Q_p + (2B_2 + B_3) Q_{n-1} + (2B_1 - 1) Q_n$$

$$+ B_3 Q_{n+1} + \sum_{p=n+2}^{N-1} V_{np} Q_p + (V_{nN}/2) Q_N$$

$$+ K = \beta^2 h n \quad : \quad (n = 3, \dots, N - 3) \quad , \quad (B8d)$$

$$(U_{N-2, 0}/2) Q_0 + \sum_{p=1}^{N-4} U_{N-2, p} Q_p + (2B_2 + B_3) Q_{N-3}$$

$$+ (2B_1 - 1) Q_{N-2} + B_3 Q_{N-1} + (V_{N-2, N}/2) Q_N$$

$$+ K = \beta^2 h L(N - 2) \quad , \quad (B8e)$$

$$(U_{N-1, 0}/2) Q_0 + \sum_{p=1}^{N-3} U_{N-1, p} Q_p + (2B_2 + B_3) Q_{N-2}$$

$$+ (2B_1 - 1) Q_{N-1} + K = \beta^2 h(N - 1) \quad , \quad (\text{B8f})$$

$$(U_{N0}/2) Q_0 + \sum_{p=1}^{N-2} U_{Np} Q_p + (2B_2 + B_3) Q_{N-1}$$

$$+ (B_1 - 1) Q_N + K = \beta^2 \cdot 1 \quad , \quad (\text{B8g})$$

$$(h/2) Q_0 + h \sum_{p=1}^{N-1} Q_p + (h/2) Q_N = 1 \quad , \quad (\text{B8h})$$

where

$$B_1 = \gamma^2 h (\ln h - 1) \quad ,$$

$$B_2 = \beta^2 h/2 \quad ,$$

$$B_3 = (\gamma^2 h \ln h)/2 \quad ,$$

$$U_{np} = h \{ \beta^2 h(n - p) + \gamma^2 \ln [h(n - p)] \} \quad ,$$

$$V_{np} = \gamma^2 h \ln [h (p - n)] \quad .$$

The computing hardware which is used consists of a Control Data 3600 computer in conjunction with an x-y plotter. The software consists of a program in Fortran IV, and is presented on the following pages. Because of the characteristics of Fortran, it is necessary to index n and p from 1 instead of 0. The constant K is regarded as Q_{N+2} in the computation.

```

PROGRAM JOINT
DIMENSION Y(101),Q(101),BETA(10),GAMMA(10),LBL(15),NPC(4),
A      PRM(4,3),AH(101,4),AV(101,4),DET(100),QK(100),EPSQR(10)
COMMON/BLK1/A(102,102),C(102,1)
COMMON/BLK2/CF(100,10)
EQUIVALENCE (C,Q)
GENERATE A(I,J)
100 READ 1001, N, MB, MG, NCG, LN, LSQDEG
1001 FORMAT (6I10)
READ 1002, (BETA(K), K=1,MB)
READ 1002, (EPSQR(L), L=1,MG)
1002 FORMAT (8F10.0)
READ 2000, (LBL(M),M=1,13)
2000 FORMAT (3A8)
FN=N $ H=1.0/FN
NCRV=MB*MG
DO 121 K=1,MB
DO 120 L=1,MG
GAMMA(L)=EPSQR(L)*BETA(K)
NNC=(L+(K-1)*MG)-1 $ NN1=NNC/NCG $ NNT=NCG*NN1
IF (NNC-NNT) 2,1,2
1 NC=0
2 NC=NC+1
N2=N+2
DO 42 I=1, N2
DO 41 J=1, N2
A(I,J)=0.0
C
DIAG
IF (I.EQ.J) GO TO 11
GO TO 12
11 B1=(GAMMA(L)**2)*H*(LOGF(H)-1.0)
IF (I.GE.2.AND.I.LE.N) GO TO 27
IF (I.EQ.1.OR.I.EQ.N+1) GO TO 28
IF (I.EQ.N+2) GO TO 29
12 IF (J.EQ.I-1.OR.J.EQ.I+1) GO TO 13
GO TO 17

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C      LOWER DIAG
13 B2=((BETA(K)*H)**2)/2.0
   B3=((GAMMA(L)**2)*H*LOGF(H))/2.0
   IF (J.EQ.I-1) GO TO 14
   GO TO 15
14 IF (I.GE.3.AND.I.LE.N+1) GO TO 30
   IF (I.EQ.2) GO TO 38
   IF (I.EQ.N+2) GO TO 33
C      UPPER DIAG
15 IF (J.EQ.I+1) GO TO 16
   GO TO 17
16 IF (I.GE.1.AND.I.LE.N-1) GO TO 31
   IF (I.EQ.N) GO TO 29
   IF (I.EQ.N+1) GO TO 39
17 IF (J.LE.I-2) GO TO 21
C      UPPER TRI
   B4=(GAMMA(L)**2)*H*LOGF(H*(J-I))
   IF (J.LE.N) GO TO 34
   IF (J.EQ.N+1) GO TO 35
   IF (J.EQ.N+2) GO TO 39
C      LOWER TRI
21 B5=H*((BETA(K)**2)*H*(I-J)+(GAMMA(L)**2)*LOGF(H*(I-J)))
   IF (J.GE.2.AND.I.LE.N+1) GO TO 36
   IF (J.EQ.1.AND.I.LE.N+1) GO TO 37
C      EQUIL
25 IF (I.EQ.N+2) GO TO 26
   GO TO 40
26 IF (J.EQ.1) GO TO 33
   IF (J.GE.2.AND.J.LE.N) GO TO 32
   GO TO 40
27 A(I,J)=2.0*B1-1.0      $ GO TO 40
28 A(I,J)=B1-1.0         $ GO TO 40
29 A(I,J)=0.0            $ GO TO 40
30 A(I,J)=2.0*B2+B3      $ GO TO 40
31 A(I,J)=B3             $ GO TO 40
32 A(I,J)=H              $ GO TO 40

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33 A(I,J)=H/2.0          $ GO TO 40      1609
34 A(I,J)=B4             $ GO TO 40      1610
35 A(I,J)=B4/2.0         $ GO TO 40      1611
36 A(I,J)=B5             $ GO TO 40      1612
37 A(I,J)=B5/2.0         $ GO TO 40      1613
38 A(I,J)=B2             $ GO TO 40      1614
39 A(I,J)=1.0            1615
40 CONTINUE              1616
41 CONTINUE              1617
42 CONTINUE              1618
C      COMPUTE C(I)      1620
DO 43 I=1,N2            1625
C(I)=0.0                1630
IF (I.GE.1.AND.I.LE.N+1) C(I)=(BETA(K)**2)*H*(I-1) 1640
IF (I.EQ.N+2) C(I)=1    1650
43 CONTINUE              1655
CALL MATINV (N2,1,DETERM) 1750
NNC1=NNC+1              1755
QK(NNC1)=C(N2) $ DET(NNC1)=DETERM 1760
N1=N+1                  1762
DO 104 I=1,N1           1765
Y(I)=(I-1)*H            1766
AH(I,NC)=Y(I) $ AV(I,NC)=Q(I) 1767
104 CONTINUE            1768
PRINT 1003,NNC1         1769
1003 FORMAT (1H1,100X,I3) 1770
PRINT 1006, N, BETA(K), GAMMA(L) 1771
PUNCH 1006, N, BETA(K), GAMMA(L) 1772
1006 FORMAT ( 24X,8H      N = ,I10.,/,24X,8H BETA = ,F10.3,/,24X, 1773
A      8HGAMMA = ,F10.3,/) 1774
105 PRINT 1008, (I,Y(I),Q(I),I=1,N1) 1780
PUNCH 1008, (I,Y(I),Q(I),I=1,N1) 1785
1008 FORMAT (30X,10HDISTANCE,Y,10X,12HSHEAR FLOW,Q,/, 1790
A      (26X,I3,X,2(E12.5,8X))) 1795
NPC(NC)=N1              1796
PRM(NC,2)=BETA(K) $ PRM(NC,3)=GAMMA(L) $ PRM(NC,1)=N 1797

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1798 IF (NC.NE.NCG.AND.NNC.NE.NCRV-1) GO TO 120
1799 NCG=NC
1800 CALL GRAPH1(AV,AH,NPC,PRM,LBL,101,NCG,NCRV,LN,LSQDEG)
1810 120 CONTINUE
1811 121 CONTINUE
1812 PRINT 1010, (NNC1,DET(NNC1),QK(NNC1),NNC1=1,NCRV)
1813 1010 FORMAT (1H1,29X,6HDETERM,14X,2HQK,/, (26X,I3,X,2(E12.5,8X)))
1815 DO 125 K=1,MB
1820 DO 125 L=1,MG
1825 NNLS=L+(K-1)*MG
1830 125 PUNCH 1012, NNLS,BETA(K),EPSQR(L),(CF(NNLS,I),I=1,10)
1835 1012 FORMAT (110,/,2F12.5,/, (5(E16.8)))
1840 END

      SUBROUTINE MATINV (N,M,DETERM)
      DIMENSION IPIVOT(102),INDEX(102,2),PIVOT(102)
      COMMON/BLK1/A(102,102),B(102,1)
      C      INITIALIZATION
      10 DETERM=1.0
      15 DO 20 J=1,N
      20 IPIVOT(J)=0
      30 DO 550 I=1,N
      C      SEARCH FOR PIVOT ELEMENT
      40 AMAX=0.0
      45 DO 105 J=1,N
      50 IF (IPIVOT(J)-1) 60, 105, 60
      60 DO 100 K=1,N
      70 IF (IPIVOT(K)-1) 80, 100, 740
      80 IF (ABSF(AMAX)-ABSF(A(J,K))) 85, 100, 100
      85 IROW=J
      90 ICOLUM=K
      95 AMAX=A(J,K)
      100 CONTINUE
      105 CONTINUE
      110 IPIVOT(ICOLUM)=IPIVOT(ICOLUM)+1
      C      INTERCHANGE ROWS TO PUT PIVOT ELEMENT ON DIAGONAL
      130 IF (IROW-ICOLUM) 140, 260, 140

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140 DETERM=-DETERM
150 DO 200 L=1,N
160 SWAP=A(IROW,L)
170 A(IROW,LD)=A(ICOLUMN,L)
200 A(ICOLUMN,L)=SWAP
205 IF(M) 260, 260, 210
210 DO 250 L=1, M
220 SWAP=B(IROW,L)
230 B(IROW,L)=B(ICOLUMN,L)
250 B(ICOLUMN,L)=SWAP
260 INDEX(I,1)=IROW
270 INDEX(I,2)=ICOLUMN
310 PIVOT(I)=A(ICOLUMN,ICOLUMN)
320 DETERM=DETERM*PIVOT(I)
C   DIVIDE PIVOT ROW BY PIVOT ELEMENT
330 A(ICOLUMN,ICOLUMN)=1.0
340 DO 350 L=1,N
350 A(ICOLUMN,L)=A(ICOLUMN,L)/PIVOT(I)
355 IF(M) 380, 380, 360
360 DO 370 L=1,M
370 B(ICOLUMN,L)=B(ICOLUMN,L)/PIVOT(I)
C   REDUCE NON-PIVOT ROWS
380 DO 550 LI=1,N
390 IF(LI-ICOLUMN) 400, 550, 400
400 T=A(LI,ICOLUMN)
420 A(LI,ICOLUMN)=0.0
430 DO 450 L=1,N
450 A(LI,L)=A(LI,L)-A(ICOLUMN,L)*T
455 IF(M) 550, 550, 460
460 DO 500 L=1,M
500 B(LI,L)=B(LI,L)-B(ICOLUMN,L)*T
550 CONTINUE
C   INTERCHANGE COLUMNS
600 DO 710 I=1,N
610 L=N+1-I
620 IF (INDEX(L,1)-INDEX(L,2)) 630, 710, 630

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CALL PLOT (0.,A+1.,.1)
2 CONTINUE
CALL PLOT (0.,0.,.1)
DO 3 I=2,10,2
  A=I-1
  CALL PLOT (A,0,0,2)
  CALL PLOT (A,FLN,1)
  CALL PLOT (A+1.,FLN,2)
  CALL PLOT (A+1.,0.,.1)
3 CONTINUE
CALL PLOT (0.,0.,.1)
CALL PLOT (0.,0.,.2)
SCALE X
CALL CHAR (-1,0,2,00,LBL(4),8,0,0,0,15,.1)
CALL CHAR (-1,0,3,25,LBL(5),8,0,0,0,15,.1)
CALL CHAR (-1,0,4,50,LBL(6),8,0,0,0,15,.1)
CALL NUM (XH,XL,XS, NPC, NCG, MNP)
DTX=XL-XS $ IDTX=DTX $ IF (DTX.EQ.1.0) GO TO 52
IF (IDTX) 51,52,51
51 IDX=0 $ GO TO 55
52 DO 54 I=1,2 $ TT=10,0**I
  IDTX1=DTX*TT $ IDX=I
  IF (IDTX1) 53,54,53
53 GO TO 55
54 CONTINUE
55 CALL SCALE (XL,XS,IDX)
  C=(XL-XS)/10.0
  ENCODE (6,4,IS)XS
  4 FORMAT (F6.2)
  CALL CHAR (-0.25,-0.26,IS,6,0,0,1,8,.1,12.)
  B=XS
DO 5 I=1,10
  FF=I $ F=FF-0.26 $ G=B+C
  ENCODE (6,4,JS)G
  CALL CHAR (-0.25,F,JS,6,0,0,1,8,.1,12.)
  B=G

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C
5 CONTINUE
  SCALE Y
  CALL CHAR (2.00,-1.5,LBL(7),8.90,.15,.1)
  CALL CHAR (3.25,-1.5,LBL(8),8.90,.15,.1)
  CALL CHAR (4.50,-1.5,LBL(9),8.90,.15,.1)
  CALL NUM (YV,YL,YS, NPC, NCG, MNP)
  DTY=YL-YS $ IDTY=DTY
  IF (IDTY) 61,62,61
61 IDY=0 $ GO TO 65
62 DO 64 I=1,3 $ TT=10.0**I
  IDTY1=DTY*TT $ IDY=I
  IF (IDTY1) 63,64,63
63 GO TO 65
64 CONTINUE
65 CALL SCALE (YL,YS,IDY)
  C=(YL-YS)/10.0
  ENCODE (8,7,KS)YS
  7 FORMAT (F8.3)
  CALL CHAR (0.0,-1.10,KS,8.0.0.1./8.,1./12.)
  B=YS
  DO 9 I=1,10
  F=I $ G=B+C
  ENCODE (8,7,LS)G
  CALL CHAR (F,-1.10,LS,8.0.0.1./8.,1./12.)
  B=G
  9 CONTINUE
  C
  PARAMETER LIST
  DO 20 I=1,3
  YLB=FN*0.25+10.25 $ XLB=3*(I-1) $ LL=I+9
  CALL CHAR ( YLB,XLB,LBL(LL),8.0.0.,15.,1)
  DO 20 NC=1,NCG
  FN1=NCG-NC $ YP=FN1*0.25+10.25 $ XP=XLB+1.25
  IF (I.NE.3) GO TO 17
  NCURVE=NCURVE+1
  ENCODE (3,100,NCVE)NCURVE
  100 FORMAT (I3)

```



```

6010 SUBROUTINE SCALE (CL,CS,N)
6020 P=10.0**(N-1) $ I=P*CS $ A=I $ C2=A/P
6022 IF (C2.EQ.0.0) GO TO 2
6023 IF (CS-C2) 1,8,1
6024 1 CS=(A-1.0)/P $ GO TO 8
6026 2 IF (CS) 4,8,6
6028 4 CS=-10.0**(-(N-1)) $ GO TO 8
6030 6 CS=0.0
6035 8 CONTINUE
6038 DC=CL-CS $ J=P*DC $ B=J $ C1=B/P
6040 II=0
6050 10 P1=P*10.0**(-II) $ J1=P1*DC
6060 IF (J1.LT.10) GO TO 15
6070 II=II+1 $ GO TO 10
6075 15 IF (II) 17,16,17
6080 16 IF (DC.EQ.C1.AND.J1.EQ.1) GO TO 20
6083 17 IF (J1.LT.1) CL=1.0/P1+CS
6085 IF (J1.GE.1.AND.J1.LT.5) CL=5.0/P1+CS
6090 IF (J1.GE.5.AND.J1.LT.10) CL=10.0/P1+CS
6095 20 RETURN
6100 END
6110 SUBROUTINE CURVE1(YV,XH,NPC,MNPC,NCG,XL,XS,YL,YS)
6120 DIMENSION YV(MNPC,NCG),XH(MNPC,NCG),NPC(4)
6130 COMMON/CRV/Y(101),X(101),NCURVE
6134 DATA (NCURVE=0)
6140 SY=100./((YL-YS)/8.)
6150 SX=100./((XL-XS)/8.)
6160 CALL PLOT (YS,XS,0,SY,SX)
6170 DO 25 NC=1,NCG
6175 NCURVE=NCURVE+1
6180 K=NPC(NC)
6190 DO 3 J=1,K
6200 Y(J)=YV(J,NC)
6210 X(J)=XH(J,NC)
6290 CALL LSTSQ1(K,XL,XS)
6300 9 DO 10 I=1,101

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6310 IF (Y(I).GT.YL-0.005) Y(I)=YL
6320 IF (Y(I).LT.YS+0.005) Y(I)=YS
6330
6340 10 CONTINUE
6350 15 CALL PLOT (Y(I),X(I),2,SY,SX)
6360 IF (Y(I).NE.YS.AND.Y(I).NE.YL) CALL SYMBOL (NC,Y(I),X(I),SY,SX)
6370 NP=K
6380 IF (K.LT.101) NP=101
6390 DO 20 I=2,NP $ J1=1
6395 I1=I+1 $ IM1=I-1
6400 YI1=(Y(I)-YS)*(YL-Y(I)) $ YT2=(Y(IM1)-YS)*(YL-Y(IM1))
6405 IF (YT1.EQ.0.0.AND.YT2.EQ.0.0) J1=2
6410 CALL PLOT (Y(I),X(I),J1,SY,SX)
6420 IF (YT1.EQ.0.0.AND.I.NE.NP) GO TO 17
6430 IF (YT1.NE.0.0.AND.I.EQ.NP) GO TO 19 $ GO TO 20
6440 IF (Y(IM1).EQ.YL.AND.Y(I1).LT.YL) GO TO 19
6450 IF (Y(IM1).LT.YL.AND.Y(I1).EQ.YL) GO TO 19
6460 IF (Y(IM1).GT.YS.AND.Y(I1).EQ.YS) GO TO 19
6470 IF (Y(IM1).EQ.YS.AND.Y(I1).GT.YS) GO TO 19 $ GO TO 20
6480 19 CALL SYMBOL (NC,Y(I),X(I),SY,SX)
6510 20 CONTINUE
6520 25 CONTINUE
6530 CALL PLOT (YS,XS,2,SY,SX)
6540 END
6550 SUBROUTINE LSTSQ1(K,XL,XS)
6560 DIMENSION W(101),R(2,101),C(10)
6570 COMMON/CRV/Y(101),X(101),NCURVE
6571 COMMON/GRA/N
6572 COMMON/BLK2/CF(100,10)
6575 PRINT 100
6576 100 FORMAT (13H1LSTSQ OUTPUT,/)
6580 DO 5 I=1,K
6590 5 W(I)=1.0
6600 CALL MCPALS (K,N,0.0,W,X,Y,R,C,IDEG)
6604 DO 6 I=1,10
6606 6 CF(NCURVE,I)=C(I)
6610 PRINT 105, NCURVE,(I,R(1,I),R(2,I),I=1,K)

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105 FORMAT (4X,*CURVE NO.*,I3,/,42X,5HERROR,I5X,10HFRAC ERROR,/,/,
A      (38X,I3,X,2(E12.4,8X)))
      IDEG1=IDEG+1
      PRINT 110, IDEG,(C(I),I=1,IDEG1)
110 FORMAT (//,X,*IDEG=*,I2,/,X,*LSTSQ COEF*,/, (X,5(E16.8,X)))
      IF (K*GE.101) GO TO 12
      DO 10 J=1,101
      X(J)=(J-1)*0.01*(XL-XS)+XS $ Y(J)=0.0
      IF (X(J).EQ.0.0) GO TO 9
      DO 8 I=1,IDEG1
      Y(J)=Y(J)+C(I)*X(J)**(I-1)
      8 CONTINUE $ GO TO 10
      9 Y(J)=C(1)
10 CONTINUE
12 RETURN
      END
      SUBROUTINE SYMBOL (NC,YI,XI,SY,SX)
      R=0.04
      CALL PLOT (YI,XI,2,SY,SX)
      GO TO (1,2,3,4),NC
1 CALL CIRCLE (R,YI,XI) $ GO TO 5
2 CALL TRI (R,YI,XI) $ GO TO 5
3 CALL SQU (R,YI,XI) $ GO TO 5
4 CALL DIA (R,YI,XI)
5 CALL PLOT (YI,XI,1,SY,SX)
      END
      SUBROUTINE CIRCLE (R,YI,XI)
      CALL PLOT(YI,XI,2,100.,100.)
      CALL PLOT(YI,XI+R,2)
      DO 12 I=10,360,10
      A=I*3.1415926536/180.
      X=R*COSF(A)+XI $ Y=R*SINF(A)+YI
      CALL PLOT(Y,X,1)
12 CONTINUE
      CALL PLOT(YI,XI,2)
      END

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6920 SUBROUTINE TRI (R,YI,XI)
6930 CALL PLOT (YI,XI,2,100.,100.)
6940 CALL PLOT (-(2./3.)*0.866*R+YI,XI+R,2)
6950 CALL PLOT ((4./3.)*0.866*R+YI,XI,1)
6960 CALL PLOT (-(2./3.)*0.866*R+YI,XI-R,1)
6970 CALL PLOT (-(2./3.)*0.866*R+YI,XI+R,1)
6980 CALL PLOT (YI,XI,2)
6990 END
7000 SUBROUTINE SQU (R,YI,XI)
7010 CALL PLOT (YI,XI,2,100.,100.)
7020 CALL PLOT (YI-R,XI+R,2)
7030 CALL PLOT (YI+R,XI+R,1)
7040 CALL PLOT (YI+R,XI-R,1)
7050 CALL PLOT (YI-R,XI-R,1)
7060 CALL PLOT (YI-R,XI+R,1)
7070 CALL PLOT (YI,XI,2)
7080 END
7090 SUBROUTINE DIA (R,YI,XI)
7100 CALL PLOT (YI,XI,2,100.,100.)
7110 CALL PLOT (YI,XI+R,2)
7120 CALL PLOT (YI+R,XI,1)
7130 CALL PLOT (YI,XI-R,1)
7140 CALL PLOT (YI-R,XI,1)
7150 CALL PLOT (YI,XI+R,1)
7160 CALL PLOT (YI,XI,2)
7170 END
2010 SUBROUTINE MCPALS(M,N,EPS,W,X,Y,R,C,IDEG)
2020 DIMENSION W(M),X(M),Y(M),A(10,10),SUMXSQ(19),C(10),R(2,M),B(10)
2030 SUMXSQ(1)=B(1)=0
2040 NMX=N
2050 IF((M-N-1).LT.0) NMX=M-1
2060 NMX1=NMX+1
2070 DO 1 I=1,M
2080 R(2,I)=1.0
2090 B(1)=B(1)+Y(I)*W(I)
2100 SUMXSQ(1)=SUMXSQ(1)+W(I)

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2      R(1,1)=B(1)
      NMN=1
      IF(EPS.EQ.0) NMN=NMX
      DO 10 NN=NMN,NMX
        N2=2*NN
        N1=NN+1
        N2 =N2-1
        IF(EPS.EQ.0) N21=1
        DO 2 J=N21,N2
          J1=J+1
          IF(J1.LE.NMX1) B(J1)=0
          SUMXSQ(J1)=0
          DO 2 I=1,M
            R(2,I)=R(2,I)*X(I)
            SUM=R(2,I)*W(I)
            IF(J1.LE.NMX1) R(1,J1)=B(J1)+SUM*Y(I)
            SUMXSQ(J1)=SUMXSQ(J1)+SUM
          DO 3 I=1,N1
            J1=I-1
            DO 3 J=1,N1
              A(I,J)=SUMXSQ(J1+J)
            CALL GAUSS(N1,A,B,C)
            DO 4 I=1,N1
              B(I)=R(1,I)
            DO 8 I=1,M
              SUM=C(N1)
              DO 5 J=1,NN
                SUM=X(I)*SUM+C(N1-J)
              SUM=Y(I)-SUM
              IF((ABSF(SUM).LT.EPS).OR.(NN.EQ.NMX)) GO TO 7
              DO 6 J=1,NMX1
                R(1,J)=B(J)
              GO TO 10
            R(1,I)=SUM
            CONTINUE
          DO 9 I=1,M
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9      R(2,I)=R(1,I)/Y(I)
      IDEG=NN
      RETURN
10     CONTINUE
      RETURN
      END
      SUBROUTINE GAUSS(M,A,B,C)
      DIMENSION A(10,10),B(M),C(M)
      FORMAT (//53X,30H***SINGULAR MATRIX IN GAUSS***//)
      DO 6 K=1,M
      C(1)=0
      IMAX=K
      DO 1 I=K,M
      T=ABSF(A(I,K))
      IF(C(1).GE.T) GO TO 1
      C(1)=T
      IMAX=I
      CONTINUE
      IF(C(1).NE.0) GO TO 2
      PRINT 101
      RETURN
2     IF(K.EQ.IMAX) GO TO 4
      J=IMAX
      T=B(K)
      B(K)=B(J)
      B(J)=T
      DO 3 L=1,M
      T=A(K,L)
      A(K,L)=A(J,L)
      A(J,L)=T
      I=K+1
      DO 5 J=I,M
      T=A(J,K)/A(K,K)
      B(J)=B(J)-B(K)*T
      DO 5 L=I,M
      A(J,L)=A(J,L)-T*A(K,L)

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6	CONTINUE	2830
	J=M+1	2840
	DO 8 K=1,M	2850
	I=J-K	2860
	T=0	2870
	IMAX=I+1	2880
	DO 7 L=IMAX,M	2890
7	T=T+A(I,L)*C(L)	2900
8	C(I)=(B(I)-T)/A(I,I)	2910
	RETURN	2920
	END	2930

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