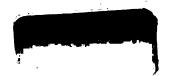


INCREASING THE TENSILE STRENGTH OF CONCRETE BY USE OF ADMIXTURES

Thesis for the Degree of M. S. MICHIGAN STATE COLLEGE Leonard A. Robert 1946

THESIS



## This is to certify that the

### thesis entitled

Increasing the Pensile Strength of Concrete by Use of Admixtures

presented by

Leonara A. Robert

has been accepted towards fulfillment of the requirements for

M. S. degree in Civil
Engineering

Major professor

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# INCREASING THE TENSILE STRENGTH OF CONCRETE BY USE OF ADMIXTURES

BY

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### A THESIS

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L. A. R.

To find an admixture for portland cement that will economically increase the tensile strength of the cement paste sufficiently to eliminate cracking in ordinary structures.

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#### BRIEF HISTORY

Egyptian massive masonry is the first record of present day methods of bonding stone with mortar. The cementing material was gypsum (burnt). Lime was not used in Egypt until the advent of the Roman period. The gypsum was quarried in an impure state and poorly burned so that it set unevenly and, not being totally decomposed, would give the impression that lime was used. Lime was more abundant but required more fuel, and fuel was scarce.

It is probable that the use of lime as a mortar came from Persia to Greece, thence to Rome. The mortar was prepared by slaking lime and mixing with sand (modern practice). The Romans had the art well perfected, but with the fall of the Roman empire all of this was lost. Medieval mortars were very poor.

In 1765 Loriot, engineer on the waterworks at

Versailles, added quicklime to the mortar to obtain increased strength and impermeability. Delafaye contested
this and recommended that egg-sized lumps of lime should
be immersed in water, transferred after a time to a cask
and there allowed to slake. Ancient writers state that
lime had to be slaked for years before using. Rondelet
discovered that it was not the method of slaking the lime,
but the thoroughness of mixing and ramming the mortar that
made the Roman work excel. Chemical analysis, texture, and
Indian practices confirm this. The tamping was so thorough
that even today the interior mortar is not set.

Greeks and Romans used a volcanic ash, finely ground and mixed with lime and sand, to make a mortar that was superior in strength and would resist water. Santorin earth in Greece, Pozzolana in Rome, and Trass in Russia and Germany are of this source. The Romans created artificial Pozzolana by grinding up tile.

The middle ages lost the art of burning lime and the knowledge of artificial or natural Pozzolana, Trass or Santorin earth. In evidence are the very poor buildings of the ninth, tenth and eleventh centuries. Beginning with the twelfth century we notice a gradual improvement in quality so that by the beginning of the fifteenth century excellent mortar is again in evidence.

The father of modern cement is John Smeaton. His investigations led to the use of "blue lias hydraulic lime" and possolana (1756).

The forerunner of "portland cement" is credited to L. J. Vicat. He prepared an artificial hydraulic lime by calcining an intimate mixture of limestone and clay, ground together in a wet mill. Many such attempts were being made during this period, the object being to duplicate what nature had done in creating a natural cement material in the volcanic ash.

Natural cements; i.e., Trass, Possalana, are materials formed by calcining a naturally occurring mixture of calcareous and argillaceous substances at a temperature below that at which sintering takes place.

Vicat, Fremy, Winkler, Michaelis and Le Chatelier all contributed to the physical and chemical analysis of portland cement, but systematic work on portland cement was begun in the United States in the Geophysical Laboratory of the Carnegie Institute at Washington, D. C. in 1906. Since 1926 the P. C. A. has done much to improve cement.

Concrete is described by Vitruvius and is found in the vaults of the Thermae and Basilica of Constantine; its use is at least that old. The Pantheon has concrete walls twenty feet thick. The Roman concrete was far superior to medieval concrete and was not equalled until the construction of the West India docks in 1800.

Chemical Analysis as Known Today

The chemicals in today's portland cement are CaO, CaOH, MgO, SiO<sub>2</sub>, Al<sub>2</sub>O<sub>3</sub> and FeO.

The compounds formed by the addition of water and letting set are: 30a0-SiO<sub>2</sub>, 2Ca0-SiO<sub>2</sub>, 3CaO-Al<sub>2</sub>O<sub>3</sub>, 3CaO-Al<sub>2</sub>O<sub>3</sub>, 3CaO-Al<sub>2</sub>O<sub>3</sub>, 3CaO-Al<sub>2</sub>O<sub>3</sub>, 2CaO-Al<sub>2</sub>O<sub>3</sub>, 3CaO-Al<sub>2</sub>O<sub>3</sub>, 2CaO-Al<sub>2</sub>O<sub>3</sub>, 4CaO-Al<sub>2</sub>O<sub>3</sub>-Fe<sub>2</sub>O<sub>3</sub>.

Tricalcium silicate (30a0-SiO<sub>2</sub>) has all the essential properties of portland cement. It has initial and final set within a few hours after gauging and shows no unsoundness. Gypsum affects time of setting.

Dicalcium silicate (2CaO-SiO<sub>2</sub>) has no definite setting time and sets only slowly over a period of days

and months. Gypsum does not affect time of setting.

Tricalcium Aluminate (30a0-Al<sub>2</sub>0<sub>3</sub>) gives a flash set.

The remaining compounds have been considered as negligible. Table I shows a strength analysis of the three major compounds.

The relation of the strength of cements to their composition has been a rather intractable problem and the factors determining strength apparently are so numerous that any solution has seemed far off. The increase in our knowledge of the constitution of cement and of the compounds present in it has, however, in recent times paved the way for some interesting work on this problem. The calculation of the contents of the various compounds in cement, erroneous to some extent as the values obtained may be, provides a preliminary basis for the allocation of strength. The compound contents, however, are far from providing all the data required for the purpose, because the strength of a cement is dependent also on the fineness to which it is ground and to some extent on the amount of gypsum added: it may perhaps be influenced to some degree by some effects, at present unknown, of minor components such as the alkalis.

Tests show that the strength of any mixture at any time might compare favorably with a ratio of the strengths of the separate parts as established in Table I. There are an insufficient number of tests to prove this; the

only thing we know is that tricalcium silicate is mainly responsible for strength development up to twenty-eight days and that dicalcium silicate makes increasing contributions from twenty-eight days on. See Table III. Reference 1, 19, 2.

Portland cement on contact with water produces hydrated compounds in solution which, owing to their insoluble nature, become highly supersaturated. The decomposing and dissolving influence of the water upon the cement going on for some time reaches a point at which the supersaturated solution surrounding the cement coagulates. The gel layer forms about the cement grains and makes it more difficult for the grains to become hydrated, dicalcium silicate taking years, tricalcium silicate a few weeks and tricalcium aluminate a very short time. The gel is apparently a silica gel which in its final geological process would be flint of agate. It seems that if a method of stepping up this process could be obtained then we would get a better concrete. The gel is apparently amorphous.

Tests made by Grun in which he reground cement which had set three days, and then reset three days, reground and reset again, show that in three days apparently only sixty-seven percent of the cement hydrated. See Table VII.

Although, as previously stated, gypsum probably affects the strength of cement to some extent, from the information available it is safe to say that gypsum has

no effect on the ultimate strength.

### Admixtures Tried and Results

Celite. Celite has no effect on the modulus or strength of concrete up to twenty-eight days.

Reference 14.

Vegetable Oils. Vegetable oils are destructive.
Reference 15.

Al. or Fe. Together or separately will increase strength. Reference 20.

Water glass. Harmful efflorescent action on concrete. Reference 24.

Sodium chloride. Increased the tensile strength of cement. The difference is decidedly significant.

Reference 25.

Tricosol. One percent tricosol will allow ten percent reduction in water, will increase compressive strength eighteen percent and tensile strength fortythree percent. Reference 17.

Gypsum, Sucrose, Calcium Chloride, Tannic Acid Triethanolamine, Ca (AcO)<sub>2</sub> H<sub>2</sub>SiF<sub>6</sub>. The detrimental effects offset the good. Reference 30.

Zinc Phosphate. Reference 27.

Common salt. Improved the tensile strength of mortars. Reference 25.

Tests with commercial and non-commercial admixtures

in percentages ranging from two to fifteen percent of the cement. Indicate that slightly hydraulic admixtures showed only small increase in strength and non-hydraulic admixtures showed reduction in strength up to about five percent for each one percent of admixture in terms of weight of cement. Colloidal clays and diatomaceous earth gave greatest reductions in strength and, in general, reduction was greater at one to three days than at greater ages. Reference 21.

The addition of arsenious oxide (As203). A thirty percent admixture gave forty percent increase in compressive strength at seven days and ninety days. The mixture has to be made at 320 to 374 degrees F. Reference 18.

Electric Heating of Concrete. This interesting article describes the experiments and experiences gained from the use of electric heating of concrete for cold weather construction. Reference 3.

### From the literature available we conclude:

- That fineness of grinding and the combined lime and silica present determine the aged strength.
- 2. A raised early strength and lowered late strength of mixtures may be the result of alumina containing compounds mixed with the calcium silicates in the absence of a retarder. The addition of tricalcium aluminate causing a reduction in

may be responsible for the increase in early strength. The development of tricalcium aluminate hydrate paste may influence a lowered late strength. The influence of the alumina containing compounds is attributed to the rapid development of a weak and open structure by the crystalline tricalcium aluminate hydrate; thus preventing optimum contacts of the hydrating calcium silicate grains.

- 3. The distribution of the water is as essential a factor as the total amount of water in combination. The structure and nature of the hydration of the colloidal calcium silicate hydrate appear to establish the rate of development of compressive strength, and not by hydration.
- 4. Tricalcium aluminate hydrate, a fluffy crystalline isotropic hydrated calcium aluminate, is formed rapidly when alumina compounds are treated with water.
- 5. The compounds chiefly responsible for high compressive strength in portland cement are tricalcium silicate and beta-dicalcium silicate, gauged with water to make a paste.
- 6. Tricalcium silicate reacts with water rapidly,

both by hydrolysis and hydration; beta-dicalcium silicate remains practically unhydralysed but combines with water to form fixed water of hydration, very slowly during the first month, but approaching the hydration value attained by tricalcium silicate in a year.

- 7. Crystalline calcium hydroxide and amorphous hydrated calcium silicate, of composition approaching the beta-dicalcium silicate hydrates to approximately the same amorphous hydrated silicate, are products of the reaction of water on tricalcium silicate. Traces of crystalline calcium nydroxide have been observed.
- 8. Crystalline tricalcium aluminate hydrate and an amorphous hydrate (the composition of which has not been definitely established) are products of the reaction of quadra calcium aluminate ferrate and water.
- 9. The quadra aluminate ferrate and the dicalcium ferrate, mixed with the calcium silicates in the absence of a retarder, lower the compressive strength of the mixtures. It is suggested that this may be due in part to the precipitation upon the grains of the hydrating calcium silicates of amorphous calcium ferrate hydrate, thus retarding the rate of impenetration of the water and reducing the bonding action.

- 10. The initial set is retarded by the use of gypsum. Gypsum reacts with the alumina that enters into solution with the formation of crystalline calcium sulphoaluminate; this occasions a delay in the development of the crystalline hydrated tricalcium aluminate.
- 11. Gypsum tends to counteract the influence of the alumina in lowering the compressive strengths of the calcium silicates. It is suggested that this favorable influence may be associated with a retardation in the development of the tricalcium aluminate hydrate due to effects of the prior formation of calcium sulfoaluminate permitting the structure of the set paste to be established by the hydrating tricalcium silicate.
- of the ferric-oxide containing compounds in lowering the compressive strengths of the calcium silicates. It is suggested that this favorable influence may be associated with the formation not only of calcium sulfoaluminate, as above, but also of the calcium sulfoferrite and the consequent reduction in the amount of the amorphous ferrite hydrate that can be deposited on the grains of the calcium silicates.
- 13. Dicalcium silicate produces nothing but gel;

so even if it had early strength it would be of no value. All-purpose cement cannot be made from any one compound. The compounds have an optimum proportion one to another.

- Amorphous silicate, upon crystallization, becomes agate quartz (SiO<sub>2</sub> hardness 7, specific gravity 2.65 and crystalline) or Calcite (CaCO<sub>3</sub> hardness 3, specific gravity 2.70 and crystalline) (limestone or marble).
- 15. Control in time of setting is facilitated by increasing silica and decreasing alumina.
- 16. Delayed development of full strength may be obtained by increasing silica at expense of lime.
- 17. Steam curing does not develop any strength but develops the strength in a shorter time.

References: 1,2,5,7,8,9,10, 11,13,26.

TABLE I

## TRUSILE STRENGTH OF CEMENT COMPOUNDS STORED IN WATER

## HBAT CEMENT

MIXTURE	1_	7.	28	91	182	365
(1) 20a0 810 <sub>2</sub> (2) 30a0 810 <sub>2</sub> (3) 30a0 Al <sub>2</sub> 0 <sub>3</sub>	440 100	698 135	98 7 <b>7</b> 2 125	538 683	616 657	718 642
81%(1) 19%(3) 81%(2) 19%(3) 50%(2) 50%(1) 40.5%(1) 40.5%(2) 19%(3)	98 267 166 307	105 665 362 432	192 908 527 671	576 562 670 681	380 551 576 671	614 603 778 803

## 1.3 MORTAR

MIXTURE		28	182	365
(1) 20a0 SiO <sub>2</sub> (2) 30a0 SiO <sub>2</sub>	<b>-</b> 2 <b>70</b>	23 407	194 371	225
(3) 30a0 Al <sub>2</sub> 6 <sub>3</sub>	•	•	•	. •
81%(1) 19%(3)	40	58	260	387
81\$(2) 19\$(3)	<b>330</b>	395	356	•
<b>50</b> \$(2) <b>50</b> \$(1)	119	220	320	•
40.5%(1) 40.5%(2) 19%(3)	198	247	335	•

Data after Bates 3% Plaster of Paris (1.65% 803) added except C3A 10% added.

TABLE II

CEMENTING MIX	1:3 MORTARS		WATER STORED	
	3ds	748	28 <b>ds</b>	
30a0 \$102	114	199	220	
30a0 SiO <sub>2</sub> (5% gypsum)	156	199	313	
75% 3Ca0 SiO <sub>2</sub> 25% 3Ca0 Al <sub>2</sub> O <sub>3</sub>	214	284	455	
3CaO Al203	•	•	43	
60% 30a0 SiO2 20% 30a0 Al203 20% gypsum	214	•	270	

Bates

TABLE III

MIXTURE	748	2848	9008	1804:	•
70% 30a0 SiO2 10% 20a0 SiO2	5000	6800	6800	6800	
30% 30a0 8i0 <sub>2</sub> 50% 20a0 8i0 <sub>2</sub>	1200	2800	5000	6200	

•

TABLE IV

	001	NCKETE	COMPRE	SSIVE	MORTAR TENSILE			LB	
Ho.	lds	348	7da	28ds	365da	lda	308	7ds	28ds
1	940	2330	3300	4650	<b>57</b> 50	319	492	579	644
2	950	3420	4580	5250	6840	378	525	610	666
3	1180	2220	3020	4520	5760	415	494	530	644
4	750	2700	4400	5350	6160	381	523	631	615
5	1530	3720	4790	5390	6000	455	552	628	626
6	530	1960	3480	5500	6070	349	501	607	632
7	855	2630	3780	4770	6490	351	473	527	610
ġ	625	2700	4450	5550	5830	425	512	526	588
9	870	3250	4690	5580	5870	424	563	604	668

1:2:4 mix, water cured and water cement ratio .60(wt.)

1:3 mortar water cured (both)

TABLE V

No.	Oa0	MeO	Al203	Fe <sub>2</sub> 03_	S10 <sub>2</sub>	T102	Na02	K20	<u>\$03</u>
7	65.6	.30	5.48	2.79	21.42	0.45	0.36	0.61	2.49
2	62.1	1.12	5.69	3.99	21.18	0.29	0.12	0.77	3.10
3	61.3	2.32	7.32	3.50	19.45	0.38	0.27	1.03	4.00
4	63.8	3.27	6.49	2.74	18.15	0.36	0.23	0.55	2.49
5	66.1	0.92	5.16	2.36	19.69	0.38	0.38	0.40	3.11
6	64.7	2.72	4.93	<b>3.3</b> 3	19.52	0.29	0.37	0.85	2.21
7	65.4	0.87	5.42	3.07	20.64	0.27	0.59	0.39	2.05
8	66.9	0.65	6.28	2.90	19.38	0.39	0.22	0.79	1.89
9	66.3	0.86	4.80	2.42	20.78	0.29	0.60	0.42	2.48

TABLE VI

Io.	CaQ	4020 Al205 Pe203	30a0 Al203	30a0 8102	20a0 S102	Flour
1	1.3	9	10	51	23	75
2	-5	12	8	56	11	77
3	1.1	11	14	31	<b>33</b>	69
4	2.0	8	13	55	12	79
5	3.7	7	10	57	13	83
6	1.9	10	7	63	9	65
7	3.5	9	ġ	48	23	75
8	2.9	9	12	62	ğ	69
9	1.7	7	9	62	13	77

## TABLE VII

1 2 3 4 5 6 7 8 9	2.78 2.96 2.64 3.05 3.01 3.02 2.80 3.04 2.93	0.94 1.00 0.90 1.04 1.01 0.98 1.04	0.94 1.00 0.90 1.03 1.02 1.02 0.98 1.03	0.92 0.99 0.88 1.00 0.98 0.98 0.92 0.99	0.91 0.98 0.86 1.00 0.99 0.99 0.94 1.00 0.96	1.03 1.07 0.96 1.11 1.10 1.10 1.06 1.12	0.95 1.02 0.95 1.12 1.07 1.07 0.99 1.05	2.59 1.88 1.60 1.97 2.36 2.43 2.11 2.88	1.96 1.42 2.09 2.37 1.48 1.48 1.76 2.16 1.98
B -	Ca0 / Ca0 /	(2 <b>.8</b> Si	0 <sub>2</sub> 1.		-	_			
D -	(Ca9-F Ca9 /	ree Ca	0) / ( S10 <sub>2</sub>	2.8510 1.65 <b>A</b> 1	2 1.1 2 <sup>0</sup> 3 .	8Al <sub>2</sub> 0 <sub>3</sub> 35Fe <sub>2</sub> 0	.65 <b>F</b> 3)	<sup>(e</sup> 2 <sup>0</sup> 3)	
G -	(CaO	1.4Mg0	) / (2	.83i0 <sub>2</sub>	1.14	1203	0.7Fe <sub>2</sub>	03)	

 $H - 8i0_2 / (Al_20_3 Fe_20_3)$ 

I - Al<sub>2</sub>0<sub>3</sub> / Fe<sub>2</sub>0<sub>3</sub>

TABLE VIII

3 days	3170	940	256
7 days	3940	1310	355
28 days	6000	1850	655

Grun

TABLE IX

1:3 SAND MORTARS AT 7 DAYS

## WATER CEMENT RATIO

Percent	Tensile strength
water	lbs. per sq. in.
8	410
9	400
10	340
ii	280
12	<b>230</b>

TABLE X

1:3 SAND MORTARS WATER CEMENT RATIO

## COMPRESSIVE STRENGTH

W/C	7 ds	28 ds	90 ds
0.52	725	1114	1407
0.56	777	1230	1866
0.60	1053	1484	2292
0.64	1084*	1640	2395#
0.68	1040	1682#	2310
0.72	784	1613	2 <b>290</b>
0.76	760	1425	2170

\* inconsistent

TABLE XI

## EFFECT OF CURING

	The state of the s	3ds.	7ds.	28ds.	<u>lyr.</u>
Test 1	air cured water cured	345 324	43 <b>4</b> 528	<b>419</b> <b>5</b> 62	671
Test 2	air cured water cured	458 501	517 607	554 632	62 <b>3</b>

TABLE XII

### EFFECT OF TEMPERATURE

	Temp."F"	l ds.	3 ds.	7 ds.	28 ds.
Normal cement	2 11 <b>17</b> 25 35	- 373 493 620	199 564 925 1096 1349	800 1158 1472 1586 1956	1916 2248 2625 2946 3036
High early strength coment	2 11 17 25 35	50 225 5 <b>75</b> 1106 1686	459 1443 1951 2221 2698	1961 2 <b>469</b> <b>2831</b> 2865 3004	3846 3604 <b>4076</b> 4263 4043

## TABLE XIII

STEAM CURING

STEAM AT 100 DEGREES FOR 16 HOURS
TEST THEN WATER AT 22 DEGREES OR MOIST AIR AT 22 DEGREES

No.	lds.	7 ds.	23 ds.	l ds.	7 ds.	_28_ds.
1	195	200	20 <b>0</b>	75	<b>34</b> 5	470
2	300	<b>305</b>	<b>35</b> 5	<b>7</b> 5	290	465
3	215	210	220	2 <b>00</b>	500	<b>520</b>
4	160	170	185	. 55	255	405

#3 was quick setting cement.

## TABLE XIV

# CONCRETE STORED IN MOIST AIR 24 HOURS BEFORE STEAM IS APPLIED

STEAM PRESSURE TIME OF CURING		68 •42hrs	103 .18hrs.		19 <b>7</b> .18hrs.		BT AIR B.7ds.		
1	2660	5480	5350	7140	6970	7600	5280	6340	
2	3330	5050	5120	5470	6000	7030	5480	6200	
3	3550	5470	4760	6670	5730	6790	2740	5080	
4	3460	5760	5580	7160	6110	6760	2560	5080	

TABLE XV

U <b>rmen</b> t	7 ds.	28 ds.		
Clinkered	358	<b>421</b>		
Melted	220	<b>3</b> 06		

## TABLE XVI

## CHEMICAL PROPORTIONING OF PORTIAND CEMENT WITHIN LIMITS

CaO	60 <b>- 7</b> 0	Fe <sub>2</sub> 0 <sub>3</sub>	0.5 - 6
SiO <sub>2</sub>	17 - 25	<b>F</b> e <sub>2</sub> 0 <sub>3</sub> <b>M</b> g0	0.1 - 5.5
Al <sub>2</sub> 6 <sub>3</sub>	3 <b>-</b> 8	Na <sub>2</sub> O	
Si0 Al <sub>2</sub> 0 SO <sub>3</sub>	1 - 3	Na <sub>2</sub> 0 plus K <sub>2</sub> 0	0.5 - 1.3

### PART II

#### PREFACE

The effect of chemical composition on the tensile strength or concrete has been quite thoroughly investigated: however, most of it has been on the basis of chemical logic, and we find that many of our most important developments have been illogical in approach or purely accidental. An example of the latter is the discovery of what is marketed today as air-entraining cement. A few years ago highway men were very much concerned over the fact that concrete highways were scaling off, except for a few highways in New York that had all been made of the same cement. Chemical analysis and the knowledge of the plant chemist could not determine why that cement was different from any other; the processes were all alike. The story unravels as follows: In the process of grinding the clinker to the required fineness we make use of a ball mill which is loaded with about six ton of steel balls from two to five inches in diameter. These balls must be cleaned once each twelve hour shift, a very tedious job. The men responsible for the equipment, being very human, were often discouraged with the difficult cleaning job; so one night one of them, having a particularly difficult time, threw a couple of shovels

of coal into the mill, hoping it would clean up the balls: and it did. Thereafter he used this method of cleaning the equipment, being very careful not to let anyone see him do it. In due time his partner on the other shift was informed of the method and cautioned to keep quiet about it. The two later discovered that a little grease and one shovel of coal would do the job very well. When the chemist had exhausted all of the chemical analysis he knew he began to question the men in the plant to get their opinion. The ball mill operators did not break down immediately: but later. observing that the chemist was greatly disturbed, they gained his confidence and told him what they had been doing, voicing the opinion that perhaps this was the reason for the different property of that particular cement. The results of tests were positive, and airentraining cements were on the market. We offer this story to appease any of the chemists who might think that in the work which follows we might have stepped out of bounds.

MIXING, SETTING, CURING AND TESTING PROCEDURE

It was decided to test a few chemicals as admixtures under the following conditions of setting and curing:

WAW.

Twelve specimens were made from the sample, placed in forms and left to set under the influence of electric, D. C., current for twenty-four hours. The specimens were then placed in water; at seven, fourteen, twenty-one and twenty-eight day ages respectively, 3 specimens were removed, one tested immediately and two left in air to be tested at about one hundred and forty days. The exact age is given with the data on each sample. The average breaking stress of the two test specimens is recorded.

Twelve specimens were made from the sample. All specimens were left in forms twenty-four hours and then placed in water. At seven, fourteen, twenty-one and twenty-eight day ages respectively, three specimens were removed from the water, one tested immediately and two left in air to be tested at about one hundred and forty days. The exact age is given with the data on each sample. The average breaking stress of the two test specimens is recorded.

MCM

Twelve specimens were made from the sample, placed in forms and left to set under the influence of

electrical, D. C. current for twenty-four hours.

Bight specimens were left in air until tested, one at seven, one at fourteen, one at twenty-one, one at twenty-eight and four at about one hundred and seventy-five days. The average of the four breaking stresses is recorded. Four specimens were left in air seven, fourteen, twenty-one and twenty-eight days; then twenty-one, fourteen, seven and seven days in water respectively; then in air until tested at about one hundred and seventy-five days. The exact age is given with the data on the sample.

Twelve specimens were made from the sample. All specimens were left in forms twenty-four hours, then removed and placed in air. Eight specimens were left in air until tested, one at seven, one at fourteen, one at twenty-one, one at twenty-eight and four at about one hundred and seventy-five days. The exact age is given with the data on each sample. The average breaking stress of the four test specimens is recorded. Four specimens were left in air seven, fourteen, twenty-one and twenty-eight days; then twenty-one, fourteen, seven and seven days in water respectively, then in air until tested at about one hundred and seventy-five days. The exact age is given with the data on the sample.

#### Test No. 1

Test number one was a control test to determine the current and voltage best suited for these particular test specimens.

It was found that a voltage of near forty with a current of three-tenths ampere was about right for the specimen. This volt-amperage value was small enough so that temperature rise was negligible, and the current was small enough not to cause excessive hydrolysis, but enough to effect a result.

Test No. 2

2,860 grams of cement. 988 c.c. of tap water.

Test procedure "A"

Test No. 3

Same mix as 2.

Test procedure "B"

Test No. 12

Same mix as 2.

Test procedure "A"

Test	No.	13		
			Same mix as 2.	
			Test procedure	"B"
			-	
Test	No.	20		
			Same mix as 2.	
			Test procedure	<b>"</b> O"
Test	Wa	01		
Test	HO.	21	<b>6</b> 0	
			Same mix as 2.	
			fest procedure	"D"
Test	No.	4		
			2,860 grams of	
			988 c.c. of 34 grams of	
				Urea.
			34 grams of	Urea.
Test	No.	6	34 grams of	Urea.
Test	No.	6	34 grams of	Urea.
Test	No.	6	34 grams of Test procedure	Urea. "B"
Test	No.	6	34 grams of Test procedure Same mix as 4.	WA"
Test			34 grams of Test procedure  Same mix as 4. Test procedure	WA"
			34 grams of Test procedure  Same mix as 4. Test procedure	WA"
			34 grams of Test procedure  Same mix as 4. Test procedure	Urea. "B"

Test No. 5 2,860 grams of cement.
916.5 c.c. of water.
71.5 c.c. 40% solution formaldahyde Test procedure "B" Test No. 15 Same mix as 5. Test procedure "A" Test No. 23 Same mix as 5. Test procedure "D" Test No. 7 2,860 grams of cement. 28.6 c.c. of phenol 959.4 c.c. of water Test procedure "B" Test No. 9 Same mix as 7. Test procedure "A" Test No. 25 Same mix as 7. Test procedure "D"

Test No. 8 2,860 grams of cement. 17 grams of urea.
35.7 c.c. of formaldahyde (40% sol.)
952.3 c.c. of water. Test procedure "B" Test No. 16 Same mix as 8. Test procedure "A" Test No. 24 Same mix as 8. Test procedure "D" Test No. 10 2,860 grams of cement.
35.7 c.c. of formaldahyde (40% sol.) 494 c.c. of carbolic sol. as in 7. 458 c.c. of water. Test procedure "B" Test No. 17 Same mix as 10. Test procedure "A" Test No. 26 Same mix as 10. Test procedure "D"

Test No. 11	
	2,860 grams of cement. 14.3 c.c. of phenol. 17 grams of urea. 973.7 c.c. of water.
	Test procedure "B"
Test No. 18	
	Same mix as 11.
	Test procedure "A"
	establishments entitletian-entit establishments
Test No. 27	
	Same mix as 11.
	Test procedure "D"
Test No. 14	
	2,860 grams of cement. 9.5 c.c. of phenol. 11.3 grams of urea. 23.8 c.c. of formaldahyde. 954.7 c.c. of water.
	Test procedure "B"
Test No. 19	
	Same mix as 14.
	Test procedure "A"
Test No. 28	
	Same mix as 14.
	Test procedure "D"

Test No. 41 953 grams of cement. 2,859 grams of sand (Ottawa 30-50). 582 c.c. of water. 9.5 grams of urea. Test procedure "B" Test No. 42 953 grams of cement. 2,859 grams of sand (Ottawa 30-50). 558 c.c. of water. 23.8 c.c. of formaldahyde. Test procedure "B" Test No. 36 953 grams of cement. 2,859 grams of sand. 582.5 c.c. of water. Test procedure "B" Test No. 37 Same mix as 36. Test procedure "D" Test No. 38 2,860 grams of cement.

988 c.c. of water. 85.8 grams of Z.

Test procedure "A"

Test	No.	39	
			Same mix as 38.
			Test procedure "B"
Test	No.	40	
			Same mix as 38.
			Test procedure "D"
Test	No.	43	
			2,860 grams of cement. 28.6 grams of magnesium sulphate. 988 c.c. of water.
			Test procedure "B"
Test	No.	49	
			Same mix as 43.
			Test procedure "D"
Test	No.	44	
			2,860 grams of cement dissolved in solution of \( \frac{1}{2} \) of 1\( \tilde{5} \) stearic acid solution in benzine; let dry and powder.  988 c.c. of water.
			Test procedure "B"
Test	¥a	45	
Test	nu .	<del>*</del> /	2,860 grams of cement. 28.6 grams of sodium oxylate. 988 c.c. of water.  Test procedure "B"

Test No. 47

Same mix as 45.

Test procedure "D"

Test No. 46

953 grams of cement.
2,859 grams of sand (Ottawa 30-50)
588 c.c. of phenol solution (same concentration as used before).

Test procedure "B"

Test No. 48

2,860 grams of cement.
28.6 grams of silicate of soda.
988 c.c. of water.

Test procedure "B"

TABLE I

# SPECIMENS 24 HOURS IN FORMS 6 DAYS IN WATER

Test No.	Chemical Admixture	Breaking stress psi.	Condition of setting
6	ŭ	1480	El.
3	Pl	1430	Pl.
3 4 7	U	1380	Pl.
	C	1300	Pl.
17	C.F.	12 <b>7</b> 5	El.
45	NaO2	1250	Pl.
12	Pl <sup>2</sup>	1240	El.
2	Pl	1230	El.
5	P	1155	Pl.
2 5 48	$NaSiO_2$	1110	Pl.
15	P -	1050	El.
13	Pl	1000	Pl.
18	U.C.	990	El.
10	F.C.	912	Pl.
11	U.C.	878	Pl.
43	ngs0₄	863	Pl.
8	U.F.	850	Pl.
14	C.U.F.	795	Pl.
16	U.F.	<b>753</b>	El.
44	Spec.	734	Pl.
<b>`9</b>	c c	660	El.
19	C.U.F.	572	El.
46	S.C.	433	P1.
36	\$	424	Pl.
42	S.P.	169	Pl.
41	s.v.	69	Pl.
38	Z	•	El.
<b>39</b>	Z	-	Pl.

U	Urea
Pl	Plain
C	Phenol
P	Formaldahyde
Spec.	Special
Z	Phosphorus Pentoxide
S	Sand
R)	Electric

TABLE II

SPECIMENS 24 HOURS IN FORMS 13 DAYS IN WATER

Test No.	Chemical Admixture	Breaking stress psi.	Condition of setting
6	Ŭ	1690	El.
13	P1	1580	Pl.
7	C	1495	Pl.
7 5 17	C F	1430	Pl.
17	F.C.	1355	El.
18	U.C.	1360	Kl.
8	U.r.	1350	Pl.
3	Pl	1300	Pl.
<b>3</b> 2	Pl	1285	El.
10	F.C.	1270	Pl.
15	F	1273	El.
4	σ	1230	Pl.
12	Pl	1285	El.
14	C.U.F.	1125	Pl.
īi	U.C.	1150	Pl.
45	MgS102	1125	Pl.
48	NaSO <sub>4</sub>	1125	Pl.
45	NaO <sub>2</sub>	1050	Pl.
16	$\mathtt{U}_{\bullet}\mathtt{F}_{\bullet}^{Z}$	734	El.
44	Spec.	840	Pl.
36	S	527	Pl.
42	S.F.	130	Pl.
9	C	-	El.
19	C.U.F.	-	El.
46	S.C.	-	Pl.
41	s.u.	•	Pl.
38	2	•	El.
39	Z	-	Pl.

Ú	Urea
Pl	Plain
C	Phenol
ř	Formaldahyde
Spec.	Special
Z	Phosphorus Pentoxide
Š	Sand
RI	Rlectric

TAPLE III

SPECIMENS 24 HOURS IN FORMS 20 DAYS IN WATER

Test	Chemical Admixture	Breaking stress psi.	Condition of setting
6	U	1580	퇴.
43	MgSO <sub>4</sub>	1485	Pl.
13	PĨ ,	1450	Pl.
15	F	1440	El.
45	<b>Na</b> O	1440	Pl.
18	U.C.	1425	El.
9	Ø	1400	El.
9 12	<b>P1</b>	1410	El.
10	F.C.	1230	Pl.
	U.F.	1210	Pl.
8 5 4	P	1200	Pl.
Á	Ū	1200	Pl.
48	NaSiO <sub>2</sub>	1140	Pl.
3	Pl	1120	Pl.
48 3 11	ชี	1115	Pl.
14	C.U.F.	1100	Pl.
44	Spe c.	870	Pl.
46	S.C.	650	Pl.
41	S.U.	540	· Pl.
36	S	178	Pl.
7 <b>0</b>	Č	-	Pl.
	F.C.	-	El.
17	Pl	-	El.
2 16		_	El.
10	U.F. C.U.F.	-	El.
19		_	Pl.
42	S.F.		El.
<b>38</b>	Z	<del>-</del>	Pl.
39	Z	-	- ·

U	Urea
Pl	Plain
C	Phenol
P	Formaldahyde
Spec.	Special
2	Phosphorus Pentoxide
ร	Sand
El	Electric

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TABLE IV

SPECIMENS 24 HOURS IN FORMS 27 DAYS IN WATER

Test No.	Chemical Admixture	Breaking stress psi.	Condition of setting
<b>3</b> 2	Pl.	1710	Pl.
2	Pl.	1640	El.
18	U.C.	1445	El.
10	F.C.	1360	P1.
17	F.C.	1275	El.
4	U	1250	Pl.
11	U.C.	1223	Pl.
19	C.U.F.	1125	El.
	P	1050	Pl.
5 6	ប	920	El.
12	Pl.	800	El.
8	U.F.	<b>7</b> 90	Pl.
36	S	760	Pl.
14	C.U.F.	536	Pl.
Ż	C	400	Pl.
13	Pl.	351	Pl.
45	NaO <sub>2</sub>	•	Pl.
48	WaSiO <sub>2</sub>	•	Pl.
15	F	•	Kl.
43	<b>M</b> g <b>S</b> 04	•	Pl.
16	U.F.	•	El.
44	Spec.	•	Pl.
77	C C	•	El.
9 46	S.C.	•	Pl.
40		_	Pl.
42	S.F.	-	Pl.
41	s.v.	_	El.
<b>38</b>	Z	-	Pl.
<b>39</b>	2	•	<b>.</b> •

ซ	Urea
Pl	Plain
C	Phenol
P	Formaldahyde
Spec.	Special
Z	Phosphorus Pentoxide
S	Sand
El	Electric

TABLE V

SPECIMENS 24 HOURS IN FORMS 6 DAYS IN LAB. AIR

Test No.	Chemical Admixture	Breaking stress psi.	Condition of setting
20	P1	553	El.
49	MgSO <sub>4</sub>	547	Pl.
26	F.C.	502	Pl.
24	U.F.	482	Pl.
23	P	472	Pl.
27	U.C.	464	Pl.
47	NaO	427	Pl.
25	C	389	Pl.
21	Pl	371	Pl.
37	S	338	Pl.
28	C.U.F.	320	Pl.
22	Ū	<b>317</b>	Pl.
40	3	•	Pl.

U	Urea
Pl	Plain
C	Phenol
P	Formaldahyde
Spe c.	Special
Z	Phosphorus Pentoxide
S	Sand
R1	Electric

TABLE VI

SPECIMENS 24 HOURS IN FORMS 13 DAYS IN LAB. AIR

Test No.	Chemical Admixture	Breaking stress psi.	Condition of setting
49	MgSO <sub>4</sub>	<b>75</b> 0	Pl.
20	Pl T	<b>75</b> 0	El.
22	ប	748	Pl.
21	P1	657	Pl.
27	v.c.	603	Pl.
25	C	550	Pl.
26	F.C.	373	Pl.
47	NaO	380	Pl.
23	F	376	Pl.
24	U.F.	319	Pl.
37	S.	315	P1.
28	C.U.F.	•	Pl.
40	<b>Z</b>	•	Pl.

U	Urea
Pl	Plain
C	Phenol
F	Formaldahyde
Spec.	Special
Z	Phosphorus Pentoxide
S	Sand
R1	Rlectric

TABLE VII

SPECIMENS 24 HOURS IN FORMS 20 DAYS IN LAB. AIR

Test No.	Chemical Admixture	Breaking stress psi.	Condition of setting
27	v.c.	1115	Pl.
20	Pl	790	El.
25	Č	<b>7</b> 55	Pl.
47	MaO	750	Pl.
<b>26</b>	F.C.	712	Pl.
21	P1	652	Pl.
22	Ū	562	Pl.
24	U.F.	550	Pl.
23	P	520	Pl.
37	Š	277	Pl.
49	Mg30	180	Pl.
28	C.U.F.	-	Pl.
40	<b>Z</b>	•	Pl.

U	urea
Pl	Plain
C	Phenol
P	Formaldahyde
Spec.	Special
2	Phosphorus Pentoxide
S	Sand
El	Electric

TABLE VIII

# SPECIMENS 24 HOURS IN FORMS 26 DAYS IN LAB. AIR

Test No.	Chemical Admixture	Breaking stress psi.	Condition of setting
27	U.C.	1375	Pl.
22	U	12 <b>7</b> 5	Pl.
21	P1	1050	Pl.
25	C	1030	Pl.
26	F.C.	1030	Pl.
24	U.P.	440	Pl.
23	F	350	Pl.
37	S	320	Pl.
20	Pl	-	El.
28	C.U.F.	-	Pl.
49	MgSO4	-	Pl.
47	NaO	-	Pl.
40	Z	•	Pl.

U	Urea
Pl	Plain
C	Phenol
P	Formaldahyde
Spec.	Special
2	Phosphorus Pentoxide
S	Sand
E1	Electric

TABLE IX

# SPECIMENS 24 HOURS IN FORMS 6 DAYS IN WATER THEN IN LAB. AIR UNTIL TESTED AT AGE SHOWN

Test	Chemical	Age	Breaking	Condition
No.	Admixture	days	stress psi.	of setting
12	Pl	129	870	El.
48	WaSiO <sub>2</sub>	175	818	Pl.
42	S.F.	175	795	Pl.
14	C.U.F.	149	775	Pl.
36	S	175	750	Pl.
11	U.C.	129	720	Pl.
7	σ	115	715	Pl.
44	Spec	175	690	Pl.
41	s.v.	175	680	Pl.
6	U	115	<b>650</b>	El.
19	C.U.F.	175	635	El.
46	S.C.	175	595	Pl.
18	U.C.	146	575	El.
10	F. C.	120	<b>55</b> 0	Pl.
13	P1	129	540	Pl.
3	<b>P</b> l	116	540	Pl.
15	P	148	540	El.
3 15 2 5 9 16	Pl	116	475	El.
5	P	116	460	Pl.
9	C	120	440	El.
	U.F.	147	400	El.
17	U.F.	147	385	El.
43	MgSO4	175	<b>380</b>	Pl.
<b>38</b>	2	175	<b>390</b>	El.
4	<b>U</b>	116	380	Pl.
45	NaO	175	370	Pl.
8	U.F.	121	370	Pl.
<b>39</b>	2	175	•	Pl.

U	Urea
Pl	Plain
C	Phenol
P	Formaldahyde
Spec.	Special
Z	Phosphorus Pentoxide
3	Sand
El	Electric

TABLE X

# SPECIMENS 24 HOURS IN FORMS 13 DAYS IN WATER THEN IN LAB. AIR UNTIL TESTED AT AGE SHOWN.

Test	Chemical Admixture	Age d <b>ays</b>	Breaking stress psi.	Condition of setting
48	NaSiO2	175	976	Pl.
13	Pl <sup>2</sup>	129	860	Pl.
42	S.P.	175	855	Pl.
36	S	175	815	Pl.
41	s.u.	175	<b>7</b> 60	Pl.
46	S.C.	175	720	Pl.
44	Spec	175	6 <b>5</b> 0	Pl.
2	PĪ	116	600	El.
12	Pl	129	600	El.
14	C.U.F.	149	550	Pl.
16	U.F.	147	5 <b>45</b>	El.
11	U.C.	129	520	Pl.
15	F	148	520	El.
5 8	P	116	490	Pl.
8	U.P.	121	470	Pl.
45	NaO	175	430	, <b>Pl.</b>
9	C	120	470	El.
10	F.C.	120	410	Pl.
3	Pl	116	<b>390</b>	Pl.
18	U.C.	146	375	El.
19	C.U.F.	175	<b>360</b>	El.
7	<u>C</u>	115	355	Pl.
4	Ŭ	116	<b>320</b>	Pl.
_6	<b>ט</b>	115	280	El.
38	<b>Z</b>	175	280	El.
17	F.C.	147	275	El.
43	MgSO <sub>4</sub>	175	185	Pl.
<b>39</b>	<b>Z</b>	175	•	Pl.

U	Urea
P1	Plain
C	Phenol
F	Formaldahyde
Spec.	Special
Z	Phosphorus Pentoxide
Š	Sand
El	Electric

TABLE XI

SPECIMENS 24 HOURS IN FORMS 20 DAYS IN WATER THEN IN LAB. AIR UNTIL TESTED AT AGE SHOWN.

Test No.	Chemical Admixture	Age days	Breaking stress psi.	Condition of setting
12	Pl	129	920	El.
3	Pl	116	<b>87</b> 0	Pl.
42	S.F.	175	860	Pl.
13	Pl	129	760	Pl.
41	S.U.	175	760	Pl.
<b>36</b>	S	175	<b>7</b> 25	Pl.
46	S.C.	175	705	Pl.
48	NaSiO <sub>2</sub>	175	690	Pl.
44	Spec	175	637	Pl.
15	F	148	5 <b>78</b>	Kl.
6	U	115	570	El.
14	C.U.F.	149	470	Pl.
7	C	115	455	Pl.
9	C	120	440	El.
10	P.C.	120	440	Pl.
11	U.C.	129	410	Pl.
18	U.C.	146	400	El.
2	Pl	116	400	El.
16	U.F.	147	<b>390</b>	El.
45	Hao	175	360	Pl.
	P	116	<b>33</b> 0	Pl.
5 4 8	U	116	310	Pl.
8	U.P.	121	270	Pl.
43	Mg30 <sub>4</sub>	175	2 <b>60</b>	Pl.
19	C.U.P.	175	240	El.
<b>38</b>	2	175	230	El.
17	F.C.	147	210	El.
39	Z	175	•	Pl.

ΰ	Urea
Pl	Plain
C	Phenol
P	Formaldahyde
Spec.	Special
Z	Phosphorus Pentoxide
S	Sand
TR7	Electric

TABLE XII

# SPECIMENS 24 HOURS IN FORMS 27 DAYS IN WATER THEN IN LAB. AIR UNTIL TESTED AT AGE SHOWN.

Test No.	Chemical Admixture	Age	Breaking stress psi.	Condition
<b>,,</b> 0	WOUNT X COTA	days	artess her.	or secorng
44	Spec	175	1000	Pl.
42	S.P.	175	880	Pl.
46	S.C.	175	815	Pl.
36	8	175	775	Pl.
48	NaSiO <sub>2</sub>	175	770	Pl.
3	Pl	116	740	Pl.
41	S.U.	175	715	Pl.
18	U.C.	146	545	El.
15	F	148	530	El.
19	C.U.F.	175	530	El.
12	Pl.	129	520	El.
9	C	120	470	El.
45	NaO	175	460	Pl.
13	Pl	129	430	Pl.
5 16	F	116	410	Pl.
16	U.F.	147	410	Kl.
<b>3</b> 8	2	175	410	El.
14	C.U.F.	149	<b>3</b> 80	Pl.
7	C .	115	<b>360</b>	Pl.
10	F.C.	120	<b>35</b> 0	Pl.
2	Pl	116	344	El.
4	U	116	<b>33</b> 0	Pl.
11	U.C.	129	<b>3</b> 20	Pl.
43	MgSO <sub>4</sub>	175	305	Pl.
8	U.F.	121	260	Pl.
17	F.C.	147	260	El.
6	U	115	250	El.
<b>39</b>	8	175	•	Pl.

U	Urea
Pl	Plain
C	Phenol
F	Formaldahyde
Spec.	Special
Z	Phosphorus Pentoxide
3	Sand
E)	Electric

## TABLE XIII

SPECIMENS 24 HOURS IN FORMS, 6 DAYS IN LAB. AIR, 21 DAYS IN WATER; THEN IN LAB. AIR UNTIL TESTED AT AGE SHOWN.

Test No.	Chemical Admixture	Age days	Breaking stress psi.	Condition of setting
21	Pl	175	765	Pl.
37	ន	175	750	Pl.
26	F.C.	175	590	Pl.
49	MgSO <sub>4</sub>	175	575	Pl.
28	C.U.P.	175	570	P1.
27	U.C.	175	450	P1.
47	NaO	175	430	Pl.
24	U.F.	175	373	Pl.
25	Ö	147	190	Pl.
20	Pl	175	180	El.
22	บั	175	177	Pl.
23	ř	175	•	Pl.
40	<b>3</b>	175	•	Pl.

U	Urea
Pl	Plain
C	Phenol
7	Formaldahyde
Spe c.	Special
Z	Phosphorus Pentoxide
S	Sand
163	Electric

TABLE XIV

SPECIMENS 24 HOURS IN FORMS, 13 DAYS IN LAB. AIR, 14 DAYS IN WATER; THEN IN LAB. AIR UNTIL TESTED AT AGE SHOWN.

Test No.	Chemical Admixture	Age days	Breaking stress psi.	Condition of setting
21	P1	175	890	Pl.
23	P	175	870	Pl.
49	MgSO <sub>4</sub>	175	708	Pl.
37	S	175	610	Pl.
47	NaO .	175	<b>59</b> 0	Pl.
22	ប	175	390	Pl.
24	U.F.	175	373	Pl.
28	C.U.F.	175	370	Pl.
27	U.C.	175	275	Pl.
26	F.C.	175	270	Pl.
20	Pl	175	270	El.
25	O	147	210	P1.
40	2	175	•	Pl.

U	Urea
Pl	Plain
C	Phenol
Ţ	Formaldahyde
Spec.	Special
Z	Phosphorus Pentoxide
8	Sand
El	Electric

TABLE XV

SPECIMENS 24 HOURS IN FORMS, 20 DAYS IN LAB. AIR, 7 DAYS IN WATER; THEN IN LAB. AIR UNTIL TESTED AT AGE SHOWN.

Test No.	Chemical Admixture	Age days	Breaking stress psi.	Condition of setting
24	U.F.	175	970	Pl.
49	Mg30₄	175	858	Pl.
<b>37</b>	ອັ <b>້</b>	175	530	Pl.
<b>27</b>	U.C.	175	530	Pl.
26	F.C.	175	520	Pl.
47	NaO	175	480	Pl.
22	Ū	175	450	P1.
21	Pl	175	410	Pl.
23	P	175	355	Pl.
20	Pl	175	300	El.
25	C	147	270	Pl.
28	C.U.F.	175	180	Pl.
40	2	175	•	Pl.

U	urea
Pl	Plain
C	Phenol
P	Formaldahyde
Spec.	Special
<b>Z</b>	Phosphorus Pentoxide
S	Sand
El	Electric

TABLE XVI

SPECIMENS 24 HOURS IN FORMS, 27 DAYS IN LAB. AIR, 7 DAYS IN WATER; THEN IN LAB. AIR UNTIL TESTED AT AGE SHOWN.

Test No.	Chemical Admixture	Age days	Breaking stress psi	Condition of setting
21	Pl	175	1660	Pl.
23	P	175	1180	Pl.
24	U.F.	175	1070	Pl.
27	c.v.	175	940	Pl.
26	P.C.	175	820	Pl.
47	NaO	175	620	Pl.
49	MgSO <sub>4</sub>	175	610	Pl.
28	C.U.F.	175	590	Pl.
22	Ŭ	175	560	Pl.
37	8	ī75	550	Pl.
ŹÒ	Pl	ī75	390	El.
25	Ċ	147	370	Pl.
40	\$	175	•	Pl.

U	Urea
Pl	Plain
C	Phenol
P	Pormaldahyde
Spec.	Special
Z	Phosphorus Pentoxide
S	Sand
Rl	Electric

## TABLE XVII

SPECIMENS 24 HOURS IN FORMS THEN IN LAB. AIR UNTIL TESTED AT AGE SHOWN

Test No.	Chemical Admixture	Age days	Breaking Stress psi.				Condition of setting		
2 <b>7</b>	C.U.	175	1260	1125	1200	1030	1155	Pl.	
21	Pl	175	1000	720	1160	940	960	Pl.	
26	F.C.	175	760	820	1100	870	890	P1.	
28	C.U.F.	175	610	•	•	1100	855	Pl.	
25	C	147	520	950	1080	860	850	Pl.	
22	U	175	640	930	1030	810	850	Pl.	
49	MgSO <sub>4</sub>	175	956	646	770	770	785	Pl.	
23	P T	175	340	710	1025	690	690	Pl.	
24	U.F.	175	870	1040	-	630	635	Pl.	
20	Pl	175	660	530	640	680	630	El.	
47	NaO	175	660	690	530	620	625	Pl.	
37	8	175	400	440	410	350	400	Pl.	
40	Ž	175	•	•	•	•	•	Pl.	

U	Urea
Pl	Plain
C	Phenol
F	Formaldahyde
Spec.	Special
Z	Phosphorus Pentoxide
S	Sand
El	Electric

#### DISCUSSION

The tests were, in the most part, made with neat cement. The improvement of the cement being attained, a study of the concrete with the improved cement would follow. The criticism of such a procedure is that it is not standard and that it gives irrational results. I concede that the results are not too good to compare with results obtained by others, but as it was not my objective to compare results with those of others, but to compare the results of various mixes and procedures as made by myself, I am satisfied that the results are sufficiently consistent to form an opinion.

Let me point out that although the standard tests on concrete and mortars make use of a test specimen that has been cured in water for the full period of aging, in the construction of buildings we seldom cure our concrete over three days. It has been my objective here to try to duplicate to some degree what happens in the field and to find out if under these conditions certain admixtures would increase the strength of my cement paste.

A study of the results shows that after seven days (one day in forms, six days in water) urea, phenol, formaldahyde, sodium oxide and sodium silicate did not lower the strength of the cement paste; that after fourteen days (one day in forms, thirteen days in water)

urea, formaldahyde and phenol, by themselves and in combination, show an average better than plain cement while sodium oxide, magnesium sulphate and sodium silicate are on a par with plain cement: that after twenty-one days (one day in forms, thirteen days in water) urea, formaldahyde, phenol, magnesium sulphate and sodium silicate and combinations of urea, phenol and formaldahyde are still on a par with plain cement: that after twenty-eight days (one day in forms, twentysix days in water) urea, formaldahyde, phenol and combinations thereof are still holding their own with plain cement. Looking now at the specimens that were withdrawn from the water and placed in laboratory air at periods of time parallel to above stated test times and tested at an average age of five months, we find for seven days water curing that urea, formaldahyde, phenol and combinations thereof and sedium silicate are still comparable with plain cement; for fourteen days curing tested at five months, no comparative changes; for twentyone days curing tested at five months, no comparative changes: and for twenty-eight days curing tested at five menths, no comparative changes. The samples of cement mortars with urea, formaldahyde and phenol admixtures treated as above show a general increase of about ten percent (10%) in strength over plain cement mortars throughout the testing period.

A study of the specimens left to cure in the laboratory air shows for seven day test an average increase in strength of about ten percent (10%) for urea. formaldahyde, phenol and combinations of these and magnesium sulphate over the average for plain cement. For fourteen day test these same admixtures are on a par or a little below that of plain cement. For twentyone day test. except for phenol-urea, these admixtures are on a par with plain cement, but phenol-urea shows a fifty percent (50%) increase over the plain cement average. For twenty-eight day test the urea admixture has gained twenty percent (20%) over the plain, the phenol-urea maintains about a thirty-five percent (35%) increase and the rest of the above mentioned admixtures are about on a par with the plain cement. At the end of six months urea, formaldahyde, phenol and combinations of these are all leading the plain cement, with phenolurea about fifty percent (50%) the better.

After a study of the specimens that were air-cured then soaked in water and again left in air until tested, we find that for seven days in laboratory air, twenty-one days in water, then in laboratory air until tested at six months, the average of the urea, formaldahyde, phenol, combinations of these and magnesium sulphate were on a par with the average of plain cement. For fourteen days in laboratory air, fourteen days in water, and then in

sulphate and formaldahyde are on a par with plain cement, but the rest have fallen below. For twenty-one days in laboratory air, seven days in water, and then in laboratory air until tested at six months, urea-formaldahyde and magnesium sulphate show one hundred percent (100%) gains over plain cement while the rest are on a par or a little better. For the twenty-eight days in laboratory air, seven days in water and then in laboratory air until tested at six months, urea-formaldahyde, urea-phenol and formaldahyde show good improvements over the average plain cement; the rest are on a par with the average, but one plain cement specimen is way ahead of the average of all (unexplainable except that the value could be in error).

#### CONCLUSIONS

From a study of the results as related in the discussion I conclude that: The admixture of urea, formaldahyde, phenol, combinations of these, magnesium sulphate, and sodium silicate show a tendency to improve the strength of the cement. The admixture of phenol-urea for concrete that is to be placed where present curing methods are not economical will give an increased ultimate strength. The admixture of urea, formaldahyde, phenol or combinations of these will give a more reliable cement paste; test results show that these admixtures give a more consistent result than plain cement paste. A glance at curves for test specimens eleven, eighteen and twenty-seven in comparison to others will confirm this.

#### RECOMMENDATIONS

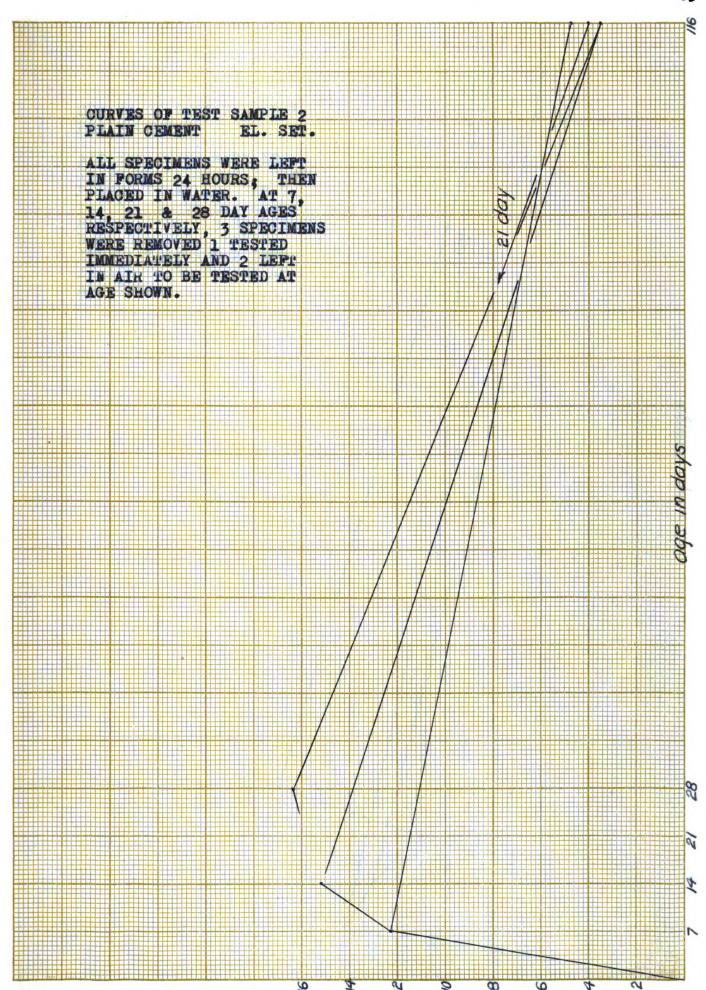
I would recommend that optimum data be obtained for admixtures of urea, formaldahyde, phenol and combinations of these. The procurement of such data will take a long time, but I feel that the preliminary investigations indicate that these admixtures will, if properly applied, prove beneficial to the concrete structure.

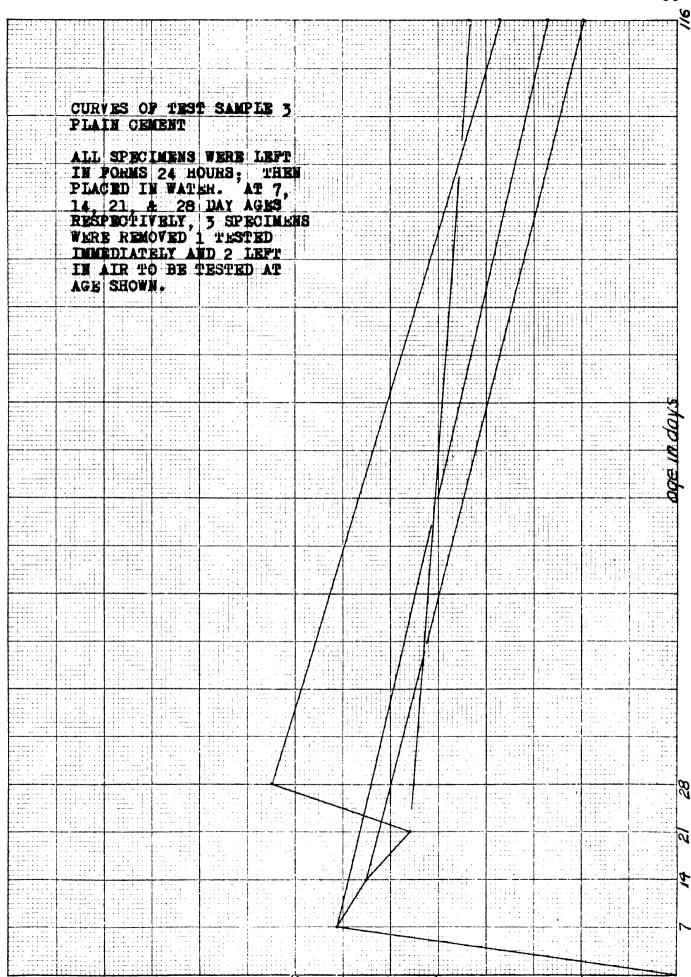
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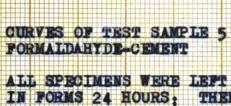




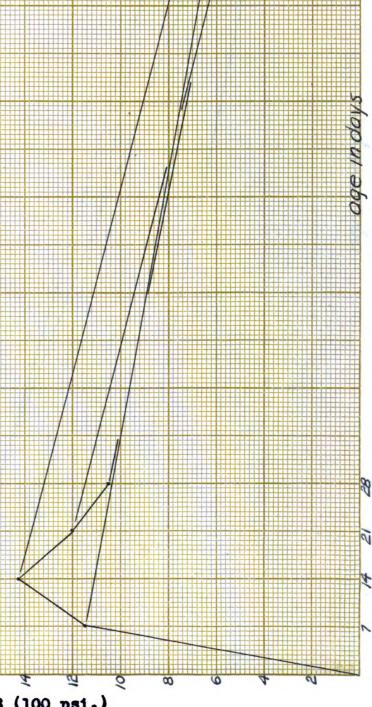
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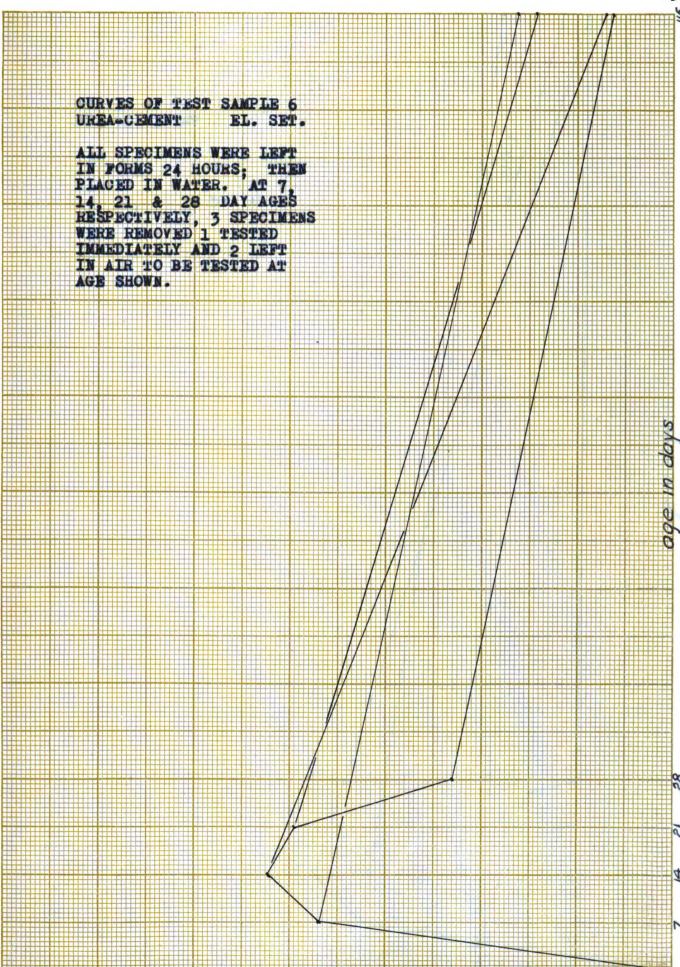
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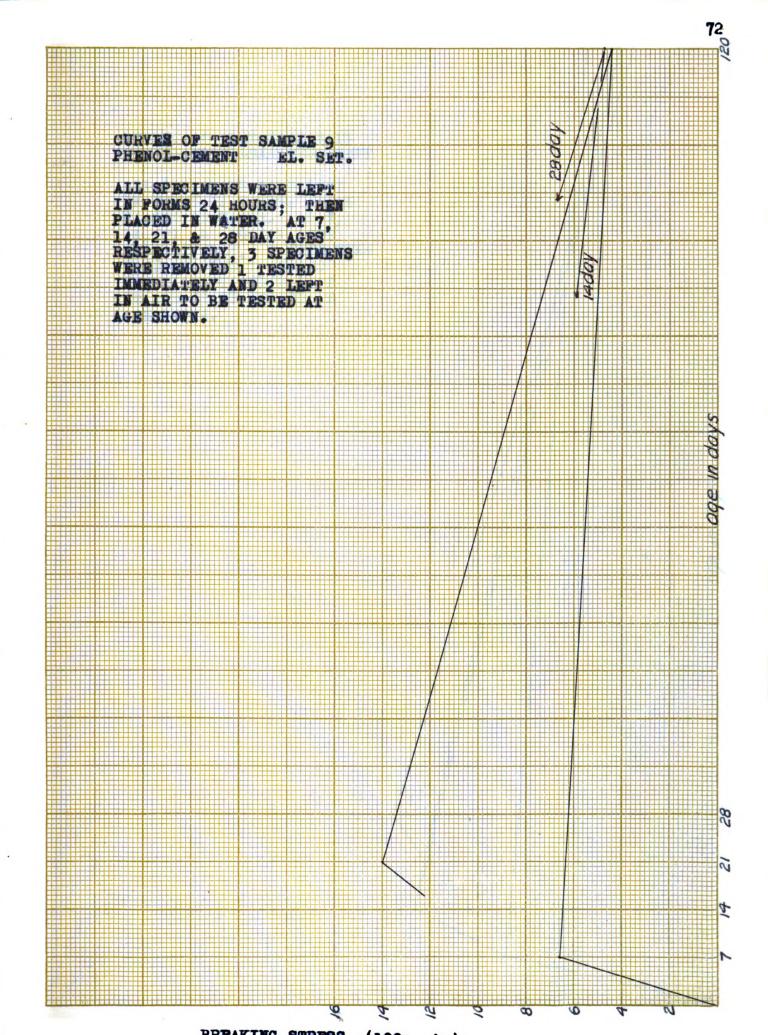
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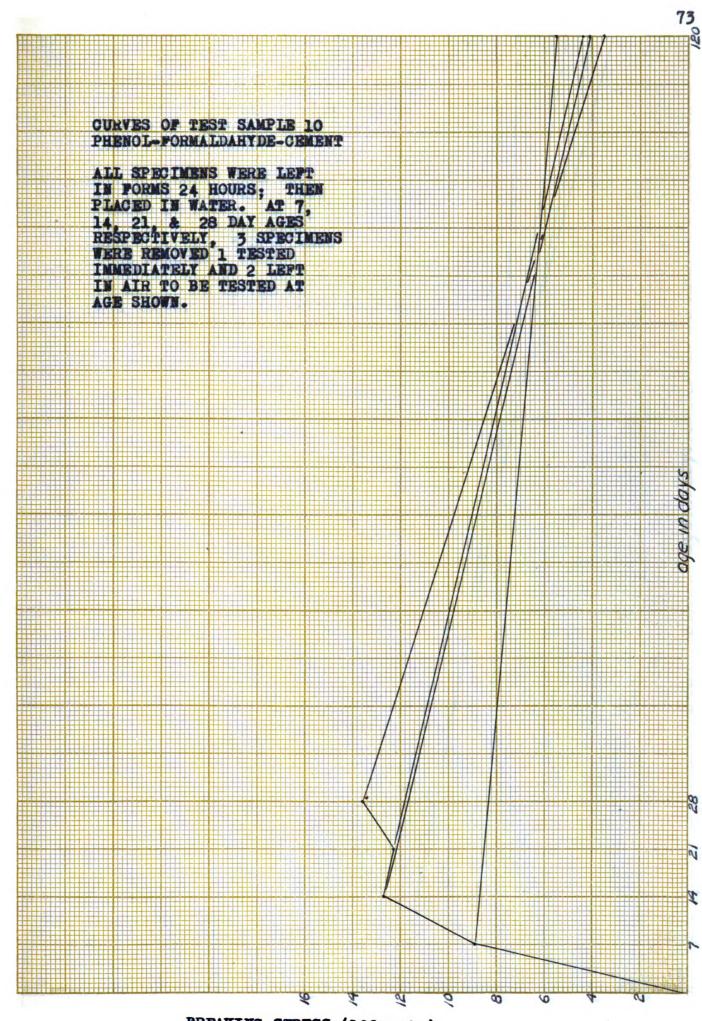
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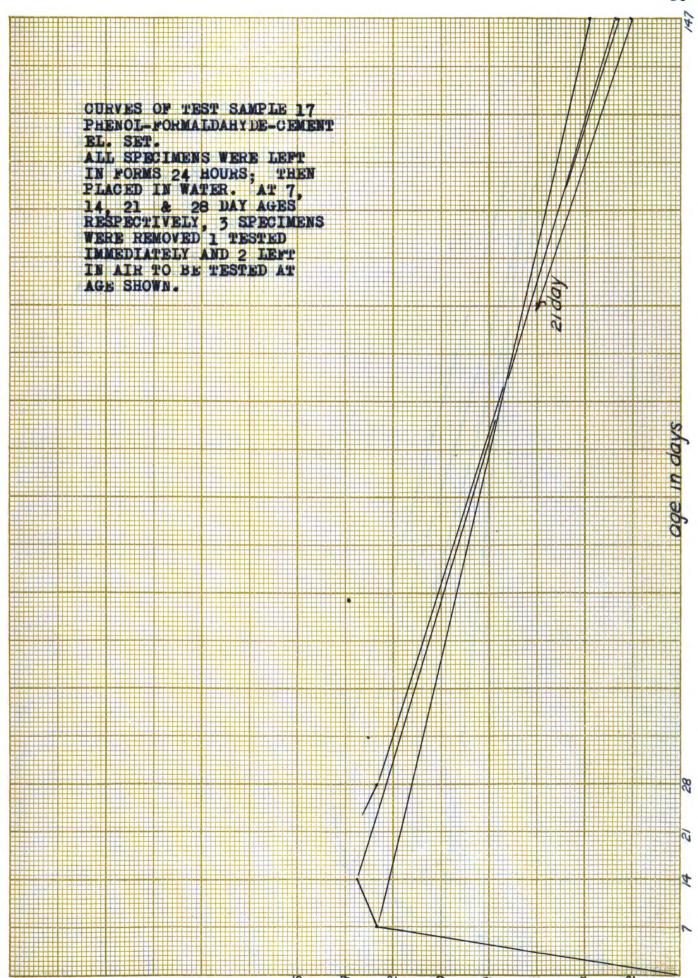
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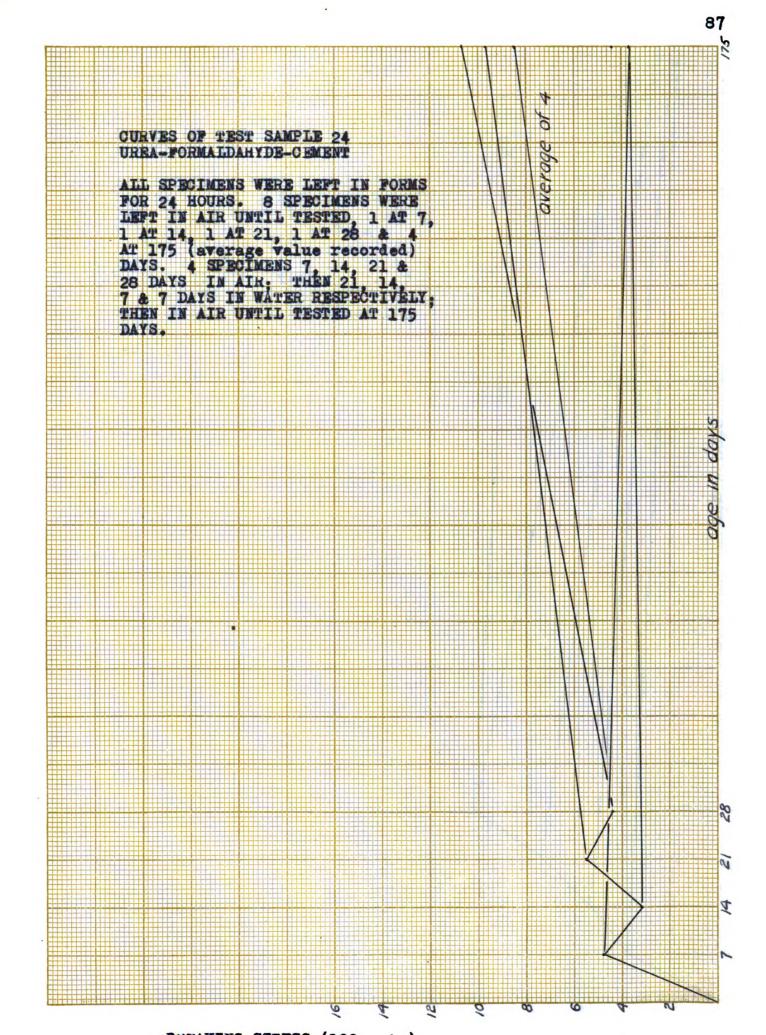
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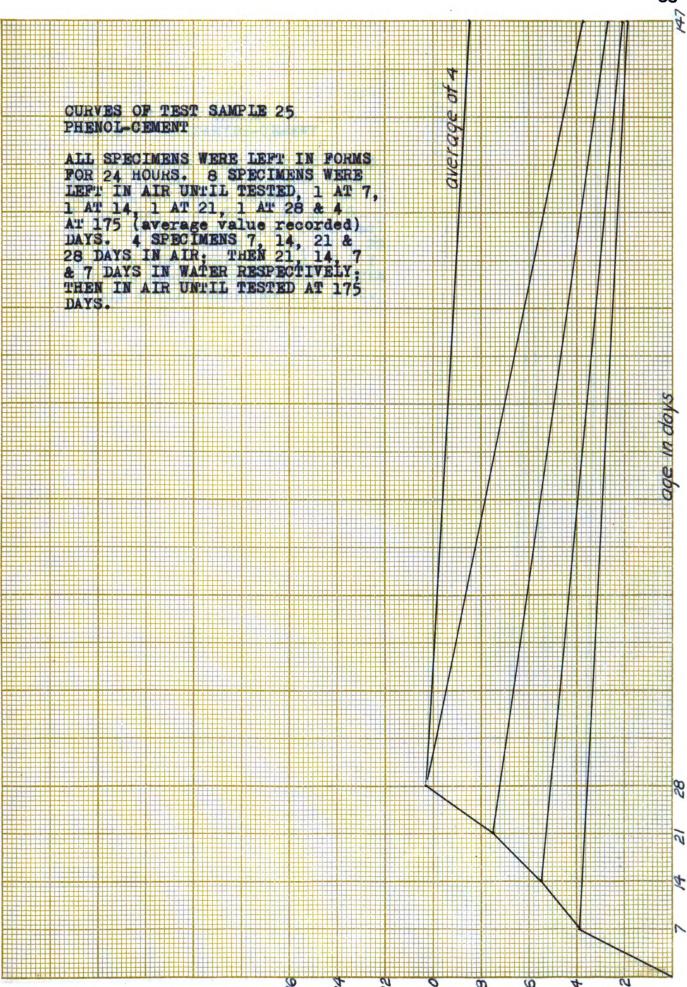
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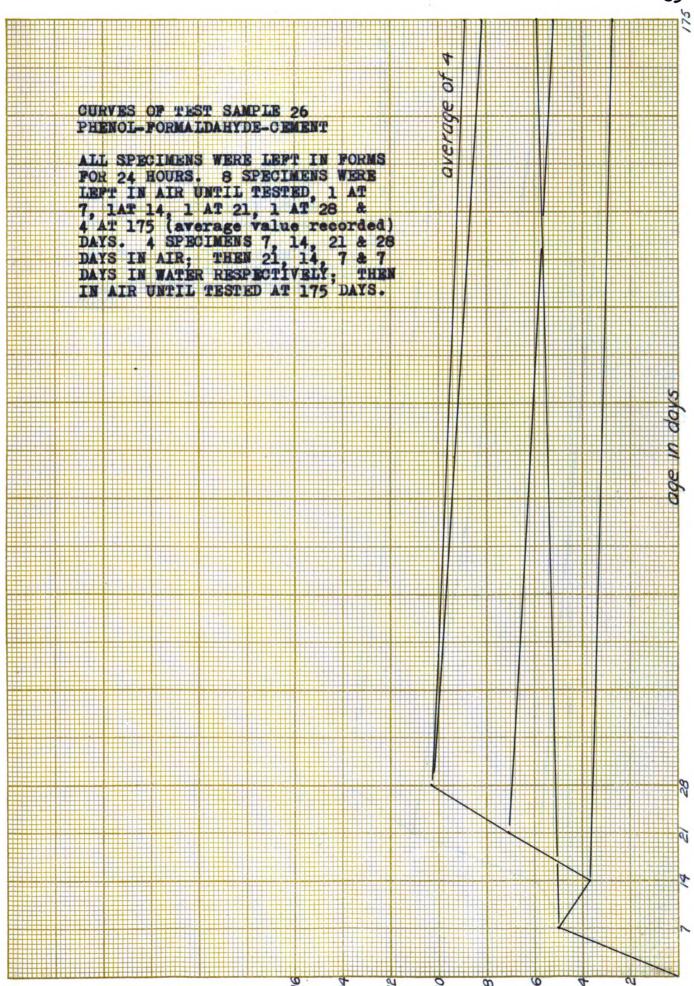


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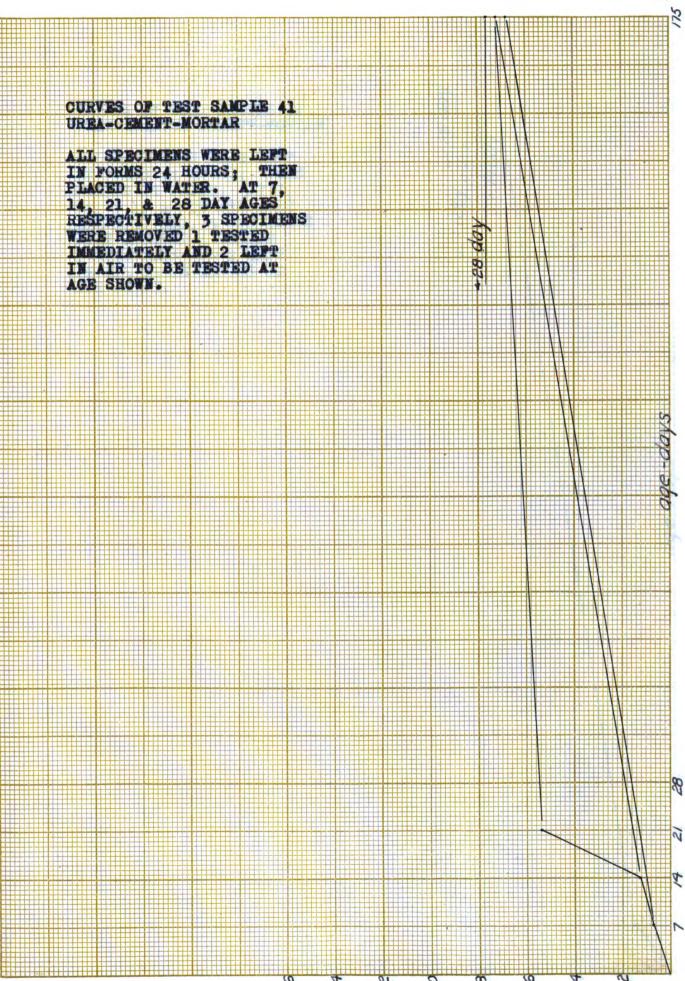
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## CURVES OF TEST SAMPLE 40 PHOSPHORUS-PENTOXIDE-CEMENT

ALL SPECIMENS WERE LEFT IN FORMS
FOR 24 HOURS. 8 SPECIMENS WERE
LEFT IN AIR UNTIL TESTED. 1 AT 7,
1 AT 14, 1 AT 21, 1 AT 28 & 4 AT
175 (average value recorded) DAYS.
4 SPECIMENS 7, 14, 21 & 28 DAYS
IN AIR: THEN 21, 14, 7 & 7 DAYS
IN WATER RESPECTIVELY: THEN IN
AIR UNTIL TESTED AT 175 DAYS.

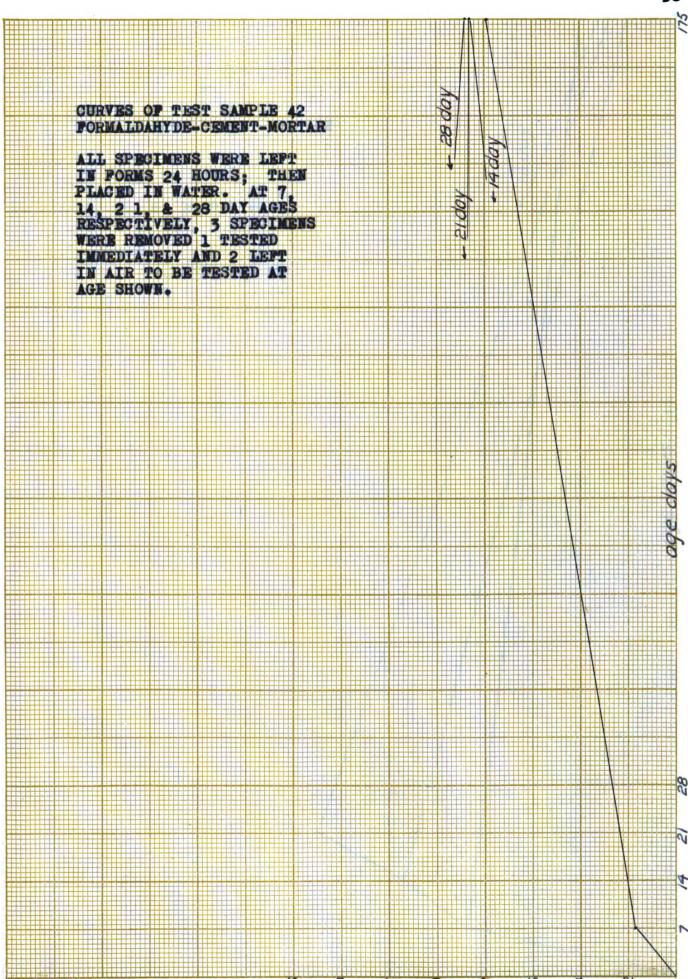
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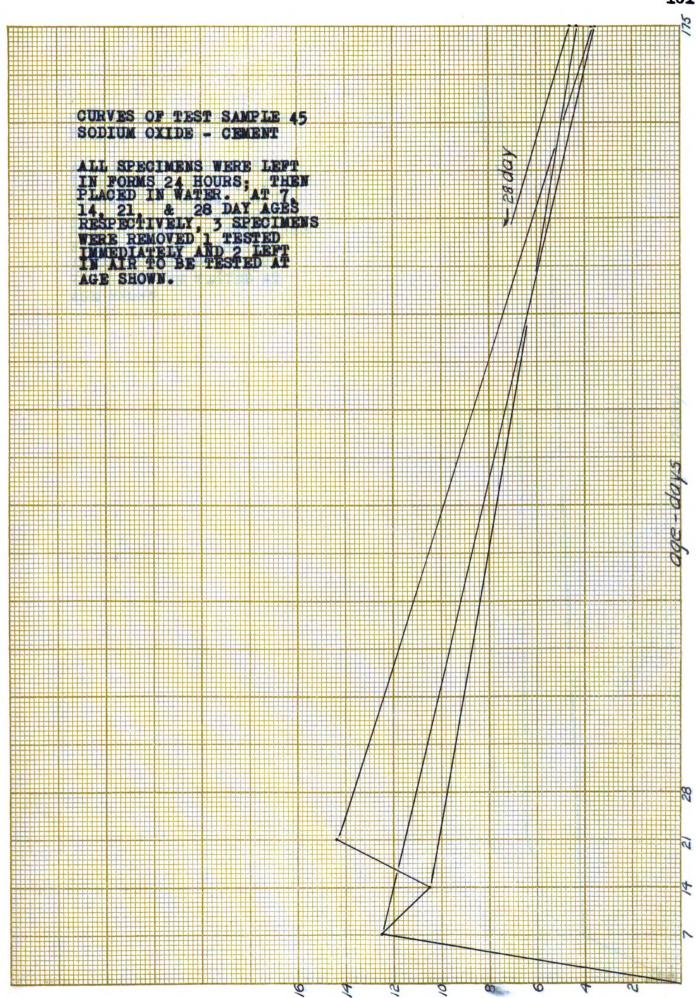
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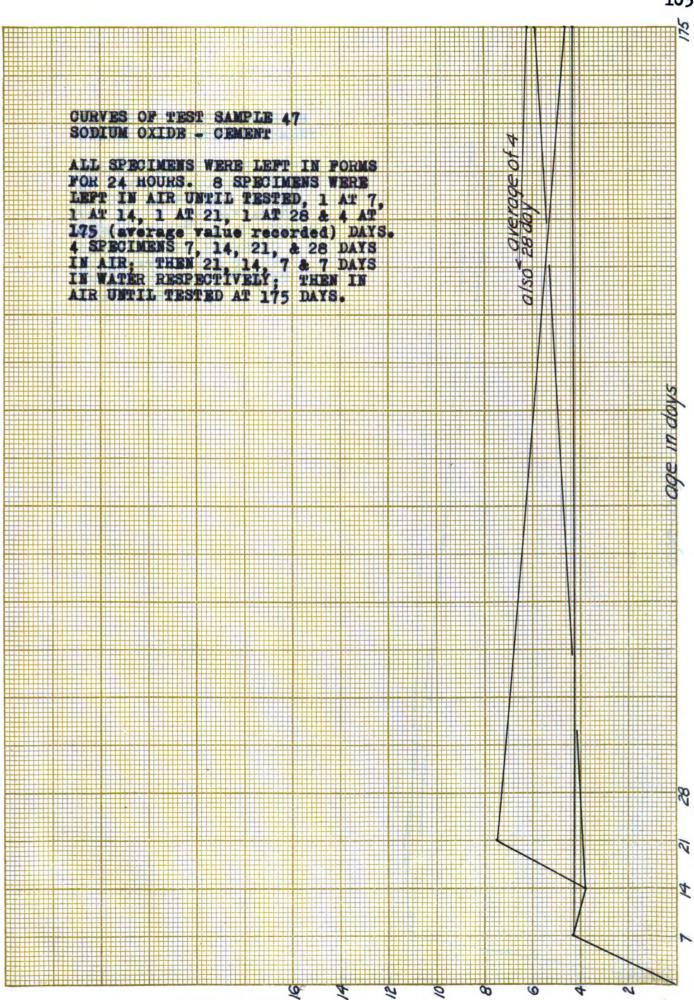
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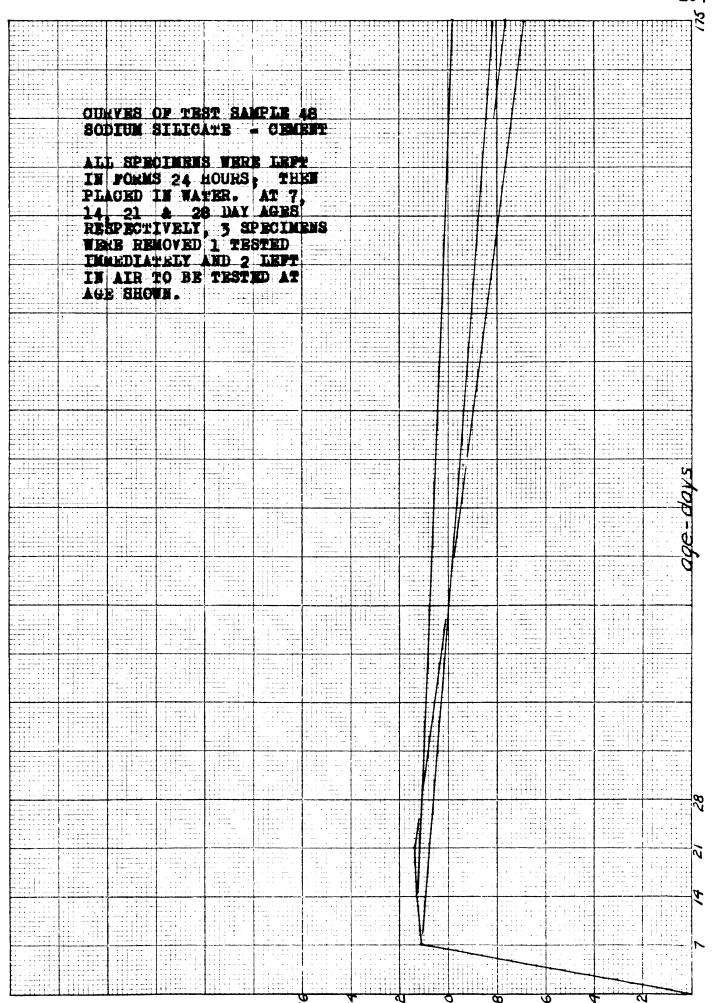
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BERAKING STRESS (100 pet.)

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## CURVES OF TEST SAMPLE 49 MAGNESIUM-SULPHATE - CEMENT

ALL SPECIMENS WERE LEFT IN FORMS
FOR 24 HOURS, 8 SPECIMENS WERE
LEFT IN AIR UNTIL TESTED, 1 AT 7,
1 AT 14, 1 AT 21, 1 AT 28 & 4
AT 175 (average value recorded)
DAYS. 4 SPECIMENS 7, 14, 21 &
28 DAYS IN AIR, THEN 21, 14, 7
& 7 DAYS IN WATER RESPECTIVELY;
THEN IN AIR UNTIL TESTED AT 175
DAYS.

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