

This is to certify that the
thesis entitled
OPTIMIZATION TECHNIQUES FOR
URBAN HIGHWAY TRAFFIC SIGNALIZATION
presented by

Sugeng Setyawan

has been accepted towards fulfillment
of the requirements for
Masters degree in Civil Engineering

William C. Taylor
Major professor

Date 4/2/86



RETURNING MATERIALS:
Place in book drop to
remove this checkout from
your record. FINES will
be charged if book is
returned after the date
stamped below.

JAN 11 1993
98024

**OPTIMIZATION TECHNIQUES
FOR
URBAN HIGHWAY TRAFFIC SIGNALIZATION**

**BY
SUGENG SETYAWAN**

A THESES

**SUBMITTED TO
MICHIGAN STATE UNIVERSITY**

*in partial fulfillment of the requirements
for the Degree of*

MASTER OF SCIENCE

Department of Civil and Environmental Engineering

1986

1211137

ABSTRACT

**OPTIMIZATION TECHNIQUES
FOR
URBAN HIGHWAY TRAFFIC SIGNALIZATION**

By

Sugeng Setyawan

Increasing construction costs which exceed revenues have forced traffic engineers to more closely fine tune the existing roadway facilities using various transportation system management strategies. Among these strategies, traffic signal coordination and optimization is recognized as one of the most cost effective and easiest to implement strategies.

Numerous computer packages are available to optimize and/or simulate existing or expected traffic flow conditions. However, few of these models have been introduced to the developing countries where the need for optimization is strong as financial constraints are one of their major problems.

Techniques for using two traffic signal models were introduced in this study and the consistency between the models tested. Furthermore, the impacts of signal timing optimization were evaluated. Delays, stops and

fuel consumptions were estimated for before and after optimization of signal timing plans, and the results were converted into highway user costs, so that the benefit of optimization could be determined.

This study also identified the level of saturation in the traffic flow which limits the value to be gained from the use of optimization models on a network.

TO MY PARENTS

ACKNOWLEDGMENT

I am deeply indebted to *Dr. William C. Taylor*, Professor and Chairperson of the Department of Civil and Environmental Engineering, for his immeasurable guidance and assistance during this study.

A sincere appreciation is expressed to the *Petra Christian University* in Indonesia which provided the possibility for my program of study.

Special thanks also to the other members of my committee, *Dr. Thomas L. Maleck*, and *Dr. Richard W. Lyles*, for their enthusiasm and assistance.

Finally, the support and understanding of my wife, *Ellyana*, and our children, *Renata* and *Karina*, in providing the impetus and strength to complete this work are admired.

Sugeng Setyawan

TABLE OF CONTENTS

	<i>Page</i>
LIST OF TABLES	<i>viii</i>
LIST OF FIGURES	<i>ix</i>

CHAPTER ONE : INTRODUCTION

I.1. Background	1
I.2. Traffic Signal Timing Optimization	2
I.3. Models Available	3
I.4. Study Objectives	11
I.5. Outline Of The Study	13

CHAPTER TWO : THE STATE-OF-THE-ART IN TRAFFIC - ENGINEERING

II.1. Transportation And Traffic Engineering - In The Developed Countries	14
II.1.1. U.S. Signal Timing Optimization - Project	17
II.2. Traffic Engineering In The Developing - Countries	20

CHAPTER THREE : MODEL OVERVIEW

III.1. TRANSYT-7F	25
III.1.1. Model Description	25
III.1.2. Input Required	26
III.1.3. The Operation	26
III.1.4. Computational Algorithms	27
III.1.5. Limitation and Other Features	33

III.2.	SOAP84	34
III.2.1.	Model Description	35
III.2.2.	Input Required	35
III.2.3.	The Operation	36
III.2.4.	Computational Algorithms	37
III.2.5.	Limitation and Other Features	43

CHAPTER FOUR : TRANSYT-7F AND SOAP84 CONSISTENCY - TEST OF THE MOB'S

IV.1.	Description of The Study Site	45
IV.2.	Study Approach	46
IV.3.	Results and Analysis of Performance - Measures	48
IV.4.	Consistency Test of The Performance - Measures	51

CHAPTER FIVE : EXAMINING THE IMPACTS OF SIGNAL - OPTIMIZATION

V.1.	Description of The Study Site	56
V.2.	Study Approach	58
V.3.	Results and Analysis	59
V.4.	Energy and Cost Evaluation	70

CHAPTER SIX : CONCLUSIONS

VI.1.	Summary	73
VI.2.	Future Development	75
VI.3.	Transferability of U.S. Experience to - Indonesia	76

APPENDICES

Appendix-1 TRANSYT-7F Optimization Run for Study -

Case-I, in Michigan Avenue, Lansing, Michigan	79
Appendix-2 SOAP84 Simulation Run of TRANSYT-7F - Optimal Timings for Study Case-I, in Michigan Avenue, Lansing, Michigan	99
Appendix-3 Regression Analysis - Program in Basic	111
Appendix-4 TRANSYT-7F Simulation Runs for The Study Case-II, in Cirebon, Indonesia	115
Appendix-5 TRANSYT-7F Optimization Runs for The Study Case-II, in Cirebon, Indonesia	126
REFERENCES	140

LIST OF TABLES

<i>Table</i>	<i>Page</i>
III.1. Reduction of Stops as a Function of Delay	32
IV.1. Relationship Between TRANSYT-7F and SOAP84 - Results	52
IV.2. The Effect of The Ratio of External to - Internal Link Volumes	54
V.1. Benefit Estimated for Volumes of .30S	71

LIST OF FIGURES

<i>Figure</i>	<i>Page</i>
III.1. TRANSYT-7F Estimate of Delay	32
III.2. Webster's Model Delay	39
IV.1. Section of Arterial Under Study	46
IV.2. Traffic Flow Diagram for The Case Study	47
IV.3. Comparison of Performance Measures	50
V.1. Network System of The Case Study	57
V.2. Link-Node Diagram for The Case Study	60
V.3. Network-wide Traffic Performance	61

CHAPTER ONE: INTRODUCTION

I.1. BACKGROUND

Currently, the emphasis in transportation planning has been shifted from long term, capital intensive - construction projects, to shorter term, relatively low cost projects, which are designed to optimize the use of existing facilities. Such a trend places heavy consideration on transportation system management [tsm] as a part of the planning process and as a prerequisite for improvements to increase the capacity and efficiency of transportation systems, in particular for urban traffic operations.

The provision of improved traffic signal systems for control at heavily trafficked intersections and corridors is by far the most common form of *tsm*. Inefficient use of the transportation system results when traffic signals are installed without sufficient study. While the U.S. Manual on Uniform Traffic Control Devices for Streets and Highways [25] discusses several warrants to justify traffic signal installations, the manual does not specify the operation of such signals.

The by-products of inefficient use of traffic signals include greater energy consumption, an increase in certain environmental impacts such as vehicle noise and emission levels, increased travel time and delay, higher

accident rates, and less reliable service.

The Federal Highway Administration forecasted that energy consumption could be conserved by 100,000 barrels of crude oil per day by optimizing the timing of the 130,000 coordinated, signalized intersections that currently exist in the U.S. urban streets. [21][24] Thus, it is obvious that a signal optimization program could be one of the most cost effective strategies that can be implemented under *tsm*.

1.2. TRAFFIC SIGNAL TIMING OPTIMIZATION

In general, signalized intersections can be classified into two major systems : (a) *isolated signals*, and (b) *coordinated signal systems* which consist of two or more intersections and streets that link those intersections. For the analysis of isolated signal timing, Webster's model represents one of the major advances in the development of optimizing signal timing. [6] [7] [8] Other analytical methods include the U.S. Highway Capacity Manual method [TRB Special-Report 209; ch.9], the Canadian Manual on Uniform Traffic Control Devices method, the Australian Road Capacity Guide method, the Average Loaded Phase Expanded [ALE] method, Bellis's method, Failure-rate method, and the use of certain British and Swedish models. For coordinated signals, various manual and computer aided progressive signal system designs have been

used with reasonable success on single arterial streets. In the case of signal network design, simplification was traditionally made by providing preferential treatment to one or more major arterials within the network, and after assigning favorable splits and offsets to these arterials, the remaining signals were forced to conform with the various restraints imposed by those major arterials.

The manual solutions are subject to more human error, and are often cumbersome and time consuming. With the availability of the computer as a versatile analytical tool, complex algorithms for signal timing design were developed, which represent significant progress in the current *state of the art*.

1.3. MODELS AVAILABLE

A number of computer based optimization models have been demonstrated to be effective and reliable. Some of those are :

SOAP

SOAP stands for the SIGNAL OPERATIONS and ANALYSIS PACKAGES, developed by the University of Florida, for the Florida Department of Transportation, and the Federal Highway Administration. SOAP is a macroscopic ana-

lysis program, which provides a method of evaluating and developing a wide-range of signal design alternatives for isolated intersections. SOAP is an optimization model which determines the solution for optimal cycle lengths, splits, phasing patterns, and left turn configurations for three or four legged intersections. Inputs include traffic flow per-approach, truck and bus composition, left turn data, signal related data, saturation flow rates, and progression related data. Basic outputs include delays, percent stops, percent saturation, maximum queue, left turn conflicts, and excess fuel consumption. The program is well written and easy to use.

MAXBAND

The MAXIMUM BANDWIDTH, is a bandwidth optimization program which calculates signal settings on arterials and triangular networks. This program can handle as many as twelve signals efficiently. Inputs include the range of cycle lengths, the network geometry, traffic flows and their capacities, left turn patterns, queue clearance times, and the allowable range of speeds. The output include a data summary report and the solution report of cycle time and bandwidths, selected phase sequencing splits, the offsets, travel time and speed on links.

TEXAS

The TRAFFIC EXPERIMENTAL and ANALYTICAL SIMULATION, was developed by the University of Texas Austin Center for Highway Research, for the Texas Department of Highways and Public Transportation. This program has the ability to evaluate existing and proposed intersection designs for both geometric and traffic operations. TEXAS is a simulation model which provides quantifiable measures of changes in traffic operations from roadway geometry, driver and vehicle characteristics, intersection and lane control, flow conditions, and signal timing plans.

PASSER

PROGRESSION ANALYSIS and SIGNAL SYSTEM EVALUATION ROUTINE, was developed by the Texas A and M University's Texas Transportation Institute for the Texas Department of Highways and Public Transportation. PASSER provides a valuable tool for determining optimal splits, phases, and offsets, primarily for coordinating traffic signals along arterial highways. Basic inputs include the range of the cycle lengths, movement flows, saturation flows, left turn patterns, queue clearance times, the desired speeds, minimum green time, allocation of bandwidth by direction, cross street phase sequences, and link distances. Outputs include cycle length, bandwidths, band

speeds, a time-space diagram, delay, probability of queue clearances, offsets, splits, phase sequences, and volume over capacity ratios.

SUB

SIMULATION OF URBAN BUSES, is a special purpose model which has been developed by the Federal Highway Administration for evaluating the benefits of alternative bus stop location [near side, farside or midblock], and physical characteristics such as protected or unprotected lanes. This model was developed to provide transit operators with a tool for evaluating bus operations along an arterial as well as the effect of various bus stop strategies on their performances.

TRANSYT

The ***TRAFFIC NETWORK STUDY TOOL***. The original TRANSYT was developed by Mr. Dennis I. Robertson of the ***Transport and Road Research Laboratory [TRRL]***, Crawthorne in the United Kingdom. TRANSYT is a major tool available to traffic engineers for analysis and evaluation of alternative traffic control strategies. It is primarily used for optimizing the signalization on arterial and grid networks. The model has a hill climb optimization algorithm that varies the offsets and phase-lengths at

each signal to locate the particular set of signal timings that minimize a Performance Index, which is a weighted linear combination of stops and delays. Basic inputs include cycle lengths, phasing, performance index weights, lost time value, link lengths, either link travel times or speeds, link flows, turning movements and saturation flows. Basic link outputs include percent saturation, total travel, travel times, delays, rate of stops, maximum queue length, offsets and splits, plus the value of the performance index. Since the original model was introduced in 1968, numerous improvements have been made and a number of versions issued, some examples are :

- ***TRANSYT-6C***

TRANSYT-6C includes fuel consumption, the estimation of emissions, demand response, a provision for priority links, and a more comprehensive measure of effectiveness. Users may determine the basis of optimization by weighting various variables.

- ***TRANSYT-7F***

This is one of the recent versions available to day with the North American no-

menclature on input and output. It also produces a time space diagram and the estimation of fuel consumption. A new revision will optimize the cycle length, and identify potential intersection blockages.

- **TRANSYT-8**

TRANSYT-8, the newest version which includes gap acceptance functions for left turns and a cycle search routine. The British government charges a fee, limiting the use of the program.

SIGOP

SIGOP stands for the traffic SIGNAL OPTIMIZATION-model. It is a powerful analysis and design tool. Conditions, such as the existing conditions, might be analyzed in terms of a number of useful traffic engineering measures. The signal timing may be optimized for cycle length, splits and offsets to minimize the 'disutility' function. Comparison of the results of several alternatives enables the traffic engineer to evaluate the relative effectiveness of the designs. The current version of SIGOP was developed by KLD Assoc., Inc. for the Office of Research, Federal Highway Ad-

ministration. It can handle up to 150 intersections, multi-phase signals can be modeled as well.

Input includes arrival flows and saturation flows, the minimum green times, yellow times, special phase times, passenger car equivalent factors for trucks, buses, and turning vehicles, while the output include time-space diagrams along selected arterials and links.

NETSIM

NETSIM is an abbreviation of the name of a model for traffic NETWORK SIMULATION. It was developed by KLD and Associates for the Federal Highway Administration. NETSIM was designed primarily to provide a powerful analysis tool to test complex network problems. It is particularly useful for the analysis of dynamically controlled traffic signal systems with the highly variable nature of their operation. This simulation model can also evaluate several alternatives which are being considered and provides a basis for a comprehensive analysis and identification of potential problems which could occur that would not show up in other models. The inputs include : the network geometry, arrival flows, saturation flows, turning counts, traffic composition, and signal setting. Outputs include a variety of measures such as delay and stops, cycle failures for pretimed signals, fuel consumption and level of emissions. NETSIM

simulates the performance of a network by using a microscopic, probabilistic model which incorporates car following, queue discharge, and lane switching algorithms. It has the capacity to handle 99 intersections, 160 links, and 1600 vehicles at a time.

PRIFRE

PRIFRE is a reverse acronym for the *FREEWAY-PRIORITY LANE* model. This model was developed by the University of California Institute of Transportation Studies at Berkeley, to improve the efficiency of freeway systems. The physical system considered by PRIFRE is a directional freeway with a priority lane reserved for high occupancy vehicles [HOV's] and the on and off ramps to the freeway. PRIFRE can be used to evaluate various types of priority treatment of high occupancy vehicles, but its primary use is for the evaluation of freeway rather than signalized intersections.

FREQ3P

FREQ3P is an acronym for the *FREEWAY OPTIMIZATION with QUEUING, version 3 control and PRIORITY treatment.* This model has been used frequently to evaluate alternative priority entry control strategies at any or all entrance ramps to optimize freeway operations. It can

be used to maximize an objective function such as passenger input or miles of travel. The physical system considered by this model is a directional, urban freeway section and the associated ramps. The freeway section is described as a series of contiguous sections which are internally operationally homogenous. It was developed by the University of California Institute of Transportation Studies at Berkeley, and it is not proposed for signalized intersections.

OTHER MODELS

Recently, a number of advanced computer models for analyzing transportation corridors have been introduced. Models like ; ***FREQ6PL, FREQ6PE, FRESCOT, TRANSYT-6C,*** and ***SINTOL*** provide the capability for investigating demand, supply and control interaction for transportation corridors. Such models are still in the process of development, testing and refinement, and won't be considered more in this study.

1.4. STUDY OBJECTIVES

These computer models have been developed and calibrated according to the conditions of the developed countries. Many of these factors are significantly different in developing countries. Thus, the models can

not readily be used by practicing engineers for solving many of the transportation management problems in the developing countries. The lack of use in these countries is also caused by many other reasons, for example :

- lack of computer hardware to run the models,
- difficulties of obtaining the software program and model documentation,
- non-familiarity with the existing models and their uses,
- perception that use of models requires an expertise in traffic flow and higher mathematical theory,
- a belief that computer models will not give practical results,

Considering the beneficial use of such models in the U.S. and other developed countries, as well as remembering the 'optimization' policy introduced by the Indonesian Direktorat General of Highway, in 1979, this study was performed to determine the feasibility of using such models in Indonesia. Hence, the purpose of this study is primarily to demonstrate optimization techniques and analyze the possible impacts of using signal optimization models in Indonesia.

1.5. OUTLINE OF THE STUDY

Two models were selected for more detailed study, TRANSYT-7F and SOAP84 versions, and their consistency was tested on an arterial street in the city of Lansing, Michigan. The performance measures of the models were compared statistically to examine the differences, if any. Furthermore, the effectiveness of TRANSYT-7F was tested in an Indonesian network. The existing network was loaded with volumes equal to $.10S$, $.20S$, $.30S$ and $.40S$, where S is the street saturation flow. Then the improvements of signal timing optimization were measured, so that the benefits of the optimization could be quantified.

CHAPTER TWO: THE STATE-OF-THE-ART IN TRAFFIC ENGINEERING

A brief insight of traffic engineering experiences in the developed countries and the developing countries are presented in this chapter. The greater population density that is commonly found in the developing countries, especially in urban areas, is associated with a lower level of traffic engineering service than in the developed countries. Many other factors also contribute to differences in traffic conditions, including nature, culture, government regulations, technology of public transportation systems, communication systems, alternate transportation facilities, pedestrian behaviors, etc. This chapter concludes briefly by examining what benefits are transferable, and what 'lessons' could be learned from the success experiences of the developed countries.

II.1. TRANSPORTATION AND TRAFFIC ENGINEERING PRACTICES IN THE DEVELOPED COUNTRIES

"The inherent need of highly developed and industrialized nations is a sophisticated and widespread transportation system". [12]

This statement is certainly true for the developed countries, which is evidenced with their intricate

urban, regional, and national transportation systems. The adequate mobility broadens the perspective of the individual as well as the nation. Mobility links the activities from different locations and facilitates the movement of ideas, people, and goods. A strong transportation system is the principal component of economic, social, cultural, and political structures of modern societies, which is an important factor in the national development. Nations that have strong and stable economics are generally those with more than adequate transportation systems.

Although Transportation Engineering has been practiced for many centuries, it was not until the last two decades that this study received recognition as a professional subdiscipline within civil engineering. For the developed countries, transportation today has benefited from centuries of experiences accumulated from across the globe, and has recently been crystalized as an interdisciplinary study involving :

- engineering;
- operation research; and
- the social sciences.

It can be defined as the science and art of planning, designing, implementing, operating, monitoring, and maintaining any medium for facilitating the movement

of people and goods.

The planning stage involves a deliberate investigative study for identifying and evaluating needs and options. It is in this stage where the tools of system engineering and the social sciences receive great emphasis. The policy or a standard system of transportation planning for urban, state, regional or rural settings is always based on scientific, analytical design techniques such as *trip generation*, *trip distribution*, *modal split* and *traffic assignment*.

The designing of transportation facilities involves adherence to engineering science, design standards and an assortment of specifications. The science of engineering has a predominant role in facility design.

Implementation refers to construction and the final opening of transportation facilities to the public. The implementation process is one of the key successes of developed countries, especially for projects which are designed to encourage a particular human behavior. It is very uncommon to find the opening of modern facilities without parallel activities in the area of public education such that proper use is encouraged and negative impacts will be minimized.

Operation is usually the direct function of government, though the operation by quasi-governmental and private agencies is not uncommon. Good management is a key component of an effective operation.

Monitoring functions are keyed to the evaluation of the primary transportation objectives and the performance of the physical components. Monitoring is a data collection and analysis mechanism which generates an information base useful for future designs, service improvements, and detecting new needs. An example of monitoring activity is the maintenance of accident record systems, which is important for improvements to design or other policies on vehicle operation.

Maintenance is just emerging as an activity which must be deliberately planned. In the U.S., thousands of miles of interstate highway have been built since 1956. However, neglecting maintenance in the planning process caused the maintenance cost to be significantly higher than what was estimated.

II.1.1. U.S. SIGNAL TIMING OPTIMIZATION PROJECT

As stated before, one of the current *tsm* targets is to provide systematic control to the traffic operation over the existing highway network so as to achieve a

comprehensive set of mobility, safety, energy, environmental and efficiency objectives.

The National Signal Timing Optimization Project was initiated by the Federal Highway Administration as a fuel conservation effort in response to the high cost of imported oil. About 35 percent of the U.S.' total daily oil consumption is consumed by vehicles traveling in urban areas throughout signalized intersections. [10] A significant portion of this is wasted due to poor traffic signal timing. In the networks with poorly timed traffic signals, the fuel consumed by vehicles stopping and idling at traffic signals accounts for approximately 40 percent of the network-wide vehicular fuel consumption. By improving traffic signal timing, the quality of traffic flow will be improved, and driving will be made faster and easier for all traffic using the street system.

There are an estimated 240,000 signalized intersections in the U.S.; of these, an estimated 130,000 are coordinated. [21][24] Just optimizing the signal timing at those intersections nationwide, it has been estimated, could conserve about two million gallons of gasoline per day. Therefore the FHWA's main target is the development of optimal signal timing plans for all of the coordinated signals and most of the non-coordinated signals within the U.S. As an initial part of the overall effort, the National Signal Timing Optimiza-

tion Project was intended to satisfy the following objectives :

- establishing credible data on the effectiveness of signal timing optimization;
- making signal timing optimization projects easy to accomplish;
- defining cost, level of staff, computer, and other resources necessary to undertake a signal timing optimization project, so that such an optimization project can be effectively budgeted.

To accomplish these objectives, the TRANSYT-7F program was first developed, then applied to programs in 11 cities nationwide to evaluate the effectiveness of the model. The 11 cities participating in the project were : Charleston, SC ; Denver, CO ; Des Moines, IA ; Fort Wayne, IN ; Gainesville, FL ; Milwaukee, WI ; Nashville, TN ; Portland, OR ; Pawtucket, RI ; San-Francisco, CA ; and Syracuse, NY. By evaluating the results, it was concluded that a signal timing optimization program could significantly reduce vehicle delays, stops, and particularly fuel consumption which is a valid national objective that can be realized through the actions of state and local governments. TRANSYT-7F has proven to be a very valuable tool for signal timing

optimization projects. Its success for design depends largely on the accuracy of the data used in the model.

II.2. TRAFFIC ENGINEERING IN THE DEVELOPING COUNTRIES

Most of the third world countries are experiencing rapid growth in motorization, as their urban centers are growing much faster than rural areas. However, such rapid developing motorized societies exist without adequately expanding the roadway infra-structure or transportation policies. As a result, urban centers are experiencing a major growth in the need for personal mobility, and consequently in increased traffic volumes and congestion. Traffic control practices are not keeping pace with this rapid growth. Economic resources for investment have become increasingly scarce, and this trend seems likely to continue. Traffic engineering problems are usually approached by managerial solutions rather than capital intensive ones to meet the rising mobility demands in urban centers. There are vast variations in traffic engineering characteristics, standards, and management techniques among the countries.

A study was performed by the *ITB Technical Council Committee 4A - 20*, in order to understand the specific types of traffic engineering practices in the developing

countries. (20) The study included :

DATA COLLECTION AND INVENTORIES

The study was designed to assess in a comprehensive manner the collection techniques and their application in design, as well as to provide some insight into the *state of the art* in traffic engineering practices. The information gathered, can be summarized as follows :

- most of the countries have some method of traffic volume counting;
- accident reports are not regularly recorded;
- the study of pedestrian behavior, particularly at intersection locations, are not routinely conducted; and
- parking studies are not done regularly, especially with respect to duration and off-street parking facilities.

STANDARDS AND PRACTICES

Many countries have some form of highway capacity manual. Some of them use the U.S. Highway Capacity Manual (22) as well as their own manuals which were developed to take account of the different traffic behavior. Some specific differences in traffic behavior

are a large percentage of heavy trucks, traffic with slow moving vehicles, mixed traffic problems, etc.

TRAFFIC CONTROL

Most of the countries use their own standards, which have been developed based on local conditions and represent modifications of U.S. or European manuals. Local languages are used for signs, and some warrants for the installation of traffic signals are modified. Certain countries, especially in Latin America, use the U.S. manual markings as well as their own markings. A lot of the countries do not incorporate the pedestrian phasing in traffic signal designs. With regard to having an inventory system for signs and markings, most countries maintain the drawings of the location record, and quite a few are using computerized maintenance systems.

TRAFFIC CONTROL PRACTICES ON SPECIAL EVENTS

Developing countries face serious traffic problems, particularly during special events due to the concentration of automobiles on the road networks within the urban centers. Few countries do trip-generation analysis, and parking demand studies. However, nearly all of the countries practice some form to control for the added traffic during special events. Those countries implement

one way systems, detour by signs, and manned detours. In case of deficiency and or incidents on the roadway, usually no restrictions are provided, which could create a hazard to traffic.

TRAFFIC ENGINEERING PROBLEMS

In the surveys used to identify typical problems in the developing countries, most of those countries addressed nearly similar problems, which generally include the following :

- limited financial resources;
- poor geometric design of the roadway;
- deficiency of traffic control devices; especially signs and signals;
- insufficient roadway lighting;
- inability to manage parking;
- inadequate traffic law enforcement;
- difficulties in safely accomodating slow moving vehicles, and mixed traffic problems;
- inefficient traffic police forces; and
- the operation of unsafe vehicles.

Recognizing such problems, particularly the financial constraint, it would be reasonable if a signal timing optimization program which became the *state of the art* in

transportation engineering in the developed countries, could be considered in setting policy in the developing countries. The benefits of such programs are obvious, and have been proven in the developed countries. However, care must be taken in implementing the models to include such specific behavior as previously mentioned. The differences cannot be neglected and good judgement is always necessary.

CHAPTER THREE: MODEL OVERVIEW

As previously described, the models selected in this study were TRANSYT-7F and SOAP84, for the reasons that they are easy to use and their use in the U.S. has demonstrated their reliability. Another consideration is that both models place the emphasis on various impact evaluations for their objective functions.

III.1. TRANSYT-7F

TRANSYT-7F stands for the TRAFFIC NETWORK STUDY TOOL, version 7F, where 'F' refers to the fact that this is the FHWA's version of TRANSYT-7, the TRRL's model. The newest version, TRANSYT-8, is now available under a licensing agreement.

III.1.1. MODEL DESCRIPTION

The TRANSYT model is a macroscopic, deterministic simulation and optimization program with independent time scan. The original program was written in machine language for use on the MARCONI MYRIAD Computer, and later rewritten in Fortran IV for more universal use. The model has been executed successfully on the IBM-360, 370, and 3033, AMDAHL-470, CDC-7700, digital VAX-11/780, BURROUGHS-7700, and HONEYWELL-6220. It requires 288k bytes

of cpu addressable memory in unoverlaid format or 172k bytes if the program is overlaid. The physical characteristics of a system considered by TRANSYT-7F is coordinated signalization on arterials and networks of up to 50 intersections with about 250 links.

III.1.2. INPUT REQUIRED

The TRANSYT program requires as input, the description of : traffic volumes and flow patterns, signal timing parameters [cycle lengths, phasing, minimum pedestrian timing if any, and the offsets], network geometry [link sections and lengths, intersections, the saturation flows, and average free speeds], and control data which indicates what the user desires to take into account, e.g. ; simulate or optimize, read English or metric unit.

III.1.3. THE OPERATION

TRANSYT's optimization procedure uses a 'hillclimbing' process that begins with the calculation of the performance index [PI] of the network for an individual set of signal timings. The next stage is to alter the offset of one of the signals by the number of time units in the specified step size, and recalculate the network's PI. If the index is reduced, then the

offset is altered successively in the same direction until a minimum value of the PI is obtained. If the first alteration increased the PI, the search is made in the opposite direction. Each signal is adjusted in a similar manner in a specified order until the minimum value of PI is reached. The process of optimizing the offset of each signal in turn is repeated to obtain the final signal settings. TRANSYT-7F also optimizes splits by altering the start of each phase and recalculating the PI as before. The current version has the capability to determine the optimal cycle length for a given range specified by the user. A number of executions will be done for every step size increment and the cycle with the lowest PI will then be selected. Therefore, optimization no longer requires multiple runs by varying input for cycle lengths as the earlier versions. Other features of the current version are the capability to provide a time-space diagram, the estimation of fuel consumption, and reduced control inputs by internally generating certain parameters.

III.1.4. COMPUTATIONAL ALGORITHMS

The major algorithms in TRANSYT-7F are the calculation of traffic characteristics and minimizing the objective function which is called the performance index [PI]. The index is a linear combination of delay and

stops, and is expressed as :

$$PI = \sum_i^n [W(D)_i \cdot d_i + k W(S)_i \cdot s_i],$$

where :

- W = the weighting factors for delay (D), and stops (S) respectively, for every link i,
 d_i = delay on the i^{th} link of the network, in [veh-hr/hr],
 s_i = average number of stops per-second on link i,
 k = a user input coefficient to express the importance of stops relative to delay.

TRANSYT simulates traffic flow macroscopically, by considering vehicles as platoons rather than individually. There are three traffic flow patterns ; the 'IN', 'GO', and 'OUT' patterns considered.

The 'IN' pattern is the arrival pattern and is expressed mathematically as :

$$IN_{it} = \sum_j^n F_{ij} [P_{ij} * OUT_{jt}]$$

where :

IN_{it} = the 'IN' pattern on link i for step t [the cycle is divided into time increments called steps],

F_{ij} = a smoothing factor for flow from link j to link i ,

P_{ij} = the proportion of OUT_{jt} that feeds link i ,

OUT_{jt} = the out pattern of link j for step t , and

n = the number of links j that feed link i .

The smoothing function [F_{ij}] represents the dispersion of vehicles, which is expressed as :

$$F_{ij} = 1/[1 + (s_p * c_e * t)]$$

where :

s_p = the smoothing parameter [usually assumed to be .35, but it might be varied],

c_e = a coefficient which 'shifts' the effective travel time [set to .8],

t = link travel time.

The 'GO' pattern is the flow rate at each step that would leave the stopline if there were enough traffic to saturate the green.

The 'OUT' pattern is the periodic traffic flow

rate leaving a link, which would be equal to the 'GO' pattern as long as there is a queue, and after the queue dissipates, it would be equal to the 'IN' pattern for the duration of the effective green. To determine the 'OUT' pattern, the number of vehicles held at the stopline during time interval t , must first be determined by :

$$m_t = \max \{ [m_{t-1} + q_t - s_t]; 0 \}$$

where :

m_t = the number of vehicles in the queue in time interval t , on a given link [similarly for m_{t-1}],

q_t = number of vehicles arriving in interval t , given by 'IN' pattern,

s_t = number of vehicles allowed to leave in interval t , given by the 'OUT' pattern.

Thus, the 'OUT' pattern given for link i during time interval t is :

$$OUT_{it} = m_{i,t-1} + q_{it} - m_{it}$$

TRANSYT estimates a delay as composed of a uniform element, d_u ; a random element, d_r , and the delay due to oversaturation, d_s . The uniform delay is determined by

averaging the queue length over the cycle, or :

$$d_u = [C/(3600/N^2)] * \sum_{t=1}^N m_t$$

where :

- d_u = uniform delay in [veh-hr/hr],
- C = cycle length in seconds,
- m_t = queue length during step t ,
- N = number of steps in the cycle.

TRANSYT computes the combined effect of random delay and saturation delay [d_{rs}] using an algorithm as follows :

$$d_{rs} = [(B_n/B_d)^2 + (x^2/B_d)]^{0.5} - [B_n/B_d]$$

where :

- d_{rs} = random, and saturation delay,
- B_n = $2 * [1-x] + [x * z]$,
- B_d = $4 * z - z^2$,
- z = $[2x/v] * [60/T]$,
- x = the degree of saturation,
- v = volume of the link,
- T = simulation time, normally 60 minutes.

The total delay, then is computed as $D = [d_u + d_{rs}]$.

<i>SECONDS OF DELAY :</i>	1	2	3	4	5	6	7	8	9	10	>10
<i>PERCENT OF STOPS :</i>	20	50	65	76	83	88	93	95	97	99	100

Table III.1. Reduction of stops as a function of delay.

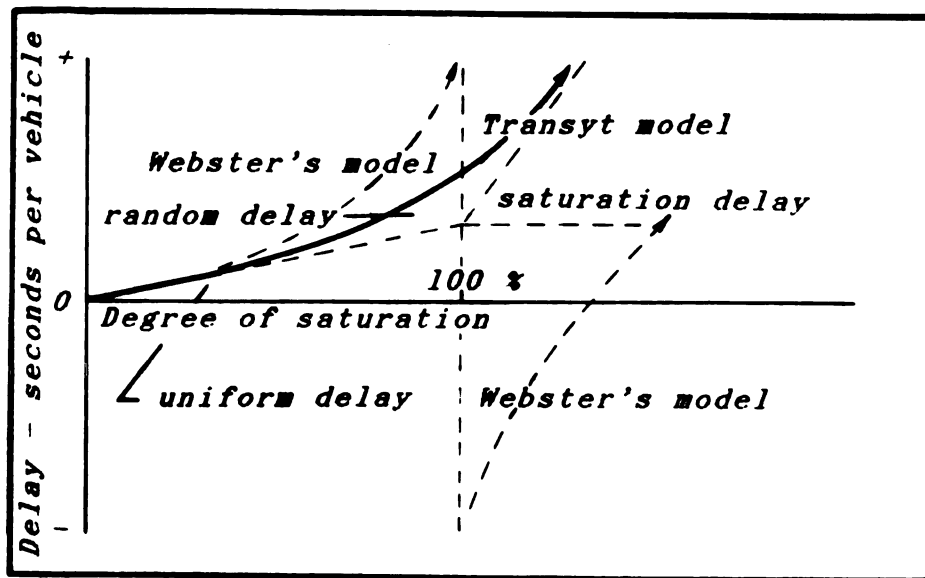


Figure III.1. TRANSYT-7F estimate of delay

The number of vehicles stopped is simply equal to the number of vehicles delayed. However, if the delay to such vehicles is small, only partial stops are counted.

The recommended adjustment of these stops, based on an empirical study suggested by TRRL are shown in Table III.1.

III.1.5. LIMITATIONS AND OTHER FEATURES

TRANSYT-7F is sufficiently realistic, and extremely useful in designing the optimal signalization of many network configurations. However, care should be taken in order to use this model. TRANSYT-7F assumes all major intersections in the network are signalized, even though mid-block bottlenecks or other sign controlled intersections can be modelled. TRANSYT does not adequately model unprotected left turning movements. The fact that vehicles must wait for an acceptable gap in traffic in opposing link must be considered since this could reduce the rates of saturation flow, and cause increasing - delays and stops in the traffic streams. Tanner's curve and the approach introduced in the user's manual (29) do not adequately represent the real world. TRANSYT-7F calculates average speed in the network by excluding links having zero distance [e.g. external links] and links that have been assigned zero delay and stop - weights [e.g. non-vehicular links]. This could of course significantly influence the results of optimization, since average speed is an important factor in estimating total travel time which is an independent variable in the estimation of fuel consumption. The entering traffic

for any network, the dispersion, volumes, and the proportion of turns are considered to be uniform and constant, which are not realistic over the entire period of analysis.

Despite those several limitations, TRANSYT-7F is an extremely powerful tool for the practicing traffic engineer. It can be used to design larger networks by subdividing those into sections handled by the present program. It also uses a fairly realistic flow model without requiring outrageous run times.

III.2. SOAP84

SOAP84, released by the Federal Highway Administration in June 1985, is one of several versions of the *Signal Operation Analysis Package*. It develops signal control plans for any three or four legged isolated intersection. This program can evaluate a wide range of control alternatives including pretimed or multiphase actual control. SOAP determines the optimal cycle length and phasing, and provides several measures of effectiveness including delay, stops, fuel consumption, and volume over capacity ratio. Left turn configurations might also be programmed with separate capacity calculation made for protected phases, unprotected phases and the clearance intervals.

III.2.1. MODEL DESCRIPTION

The SOAP model, designed and written by the Univ. of Florida Transportation Research Center, is a deterministic, macroscopic, optimization program. It was written in Fortran IV on an IBM 370/165 computer system. The program requires 202 k bytes of computer memory. During its development phase, the program had been run using IBM Fortran G, H-extended and WATF IV computers. The current version is ready to run on most IBM systems with some changes required for other systems. A version is also available for the BORROUGH computer. For the microcomputer version, the computational methodology is similar, but not identical to the methodology used in the original IBM version. SOAP84 reconciles only one set of design volumes, saturation flows, phase sequences, etc. This version deals with a maximum of 48 contiguous time periods, each with its own characteristics. Phasing optimization is accomplished by a multiple run, trial and error procedure. The complete phase sequence must be specified for SOAP84.

III.2.2. INPUT REQUIRED

The input data required include the traffic volumes, which are adjusted to reflect trucks and buses if any,

saturation flows per hour of green time given to each movement, the headway for each movement; the existing signal timing which includes minimum green time, cycle length, phasing, green time by movement, and all red time; the loss time due to starting and stopping of every movement; the growth factor to project future conditions at the intersection; left turns per cycle on clearance; peak-hour factor; speed; type of controller, pretimed or actuated control.

III.2.3. THE OPERATION

SOAP has the capability for design, analysis and evaluation. All of which can be accomplished by providing the input data previously described. To design signal timing, it is necessary to configure the intersection and input the appropriate data. SOAP then produces all legitimate phasing patterns. It internally analyzes each pattern and selects the one which can be executed using the minimum amount of green time. Cycle length is the most difficult design element to be determined, particularly when several control periods are being considered. However, SOAP determines this quickly based on the traffic volumes, capacity and other parameters. A trial and error optimization procedure is done to find the cycle length which produces the minimum total delay, subject to constraints which govern the tolerable length

of the queue. To analyze the effects of various control strategies, a number of measures of effectiveness [MOE], are computed, including delay, stops, excess fuel consumption, degree of saturation, and left turn conflicts. Evaluation is done by comparing several alternative schemes. Comparisons can be produced automatically by SOAP or manually by the user.

III.2.4. COMPUTATIONAL ALGORITHMS

The algorithms used by SOAP for the calculation of the values of the MOE's identified above are :

DELAY

To estimate delay, SOAP utilizes Webster's delay model for unsaturated flow under fixed time operations. The Webster's model has three components, which are expressed as the delay due to uniform arrivals, D_1 ; random arrival, D_2 ; and an adjustment factor, D_3 . The delay due to uniform vehicle arrivals, is :

$$D_1 = C [1 - L]^2 / 2 [1 - L * X]$$

where :

C = cycle length [sec.],

L = the proportion of green time given to

the movement [effective green time/cycle length],

X = the degree of saturation of the movement
or : approach flow [veh./sec.]/saturation-
flow [veh./sec.]

The component due to random arrivals, is :

$$D_2 = [X^2]/[2*q]/[1-X]$$

where :

q = approach flow [veh./sec.], and the other notations are the same as previously described.

Then the adjustment factor is :

$$D_3 = [- .65][C/q^2]^{1/3} * X^{(2+5L)}$$

which was derived empirically to provide a better mathematical fit to the field data. Webster's delay increases infinitely as the degree of saturation X , approach 100 %, and reenters the finite realm from the negative side at values above 100 %. This is due to the $[1-X]$ factor in the denominator of the second term. The upper limit on the useful range of X is generally in the neighborhood of 97.50 %. For the

region where the saturation is between 97.50 % to 100 %, delay is assumed to be constant at the value for $X = 97.50 \%$; or 2.0 min./veh., whichever value is less.

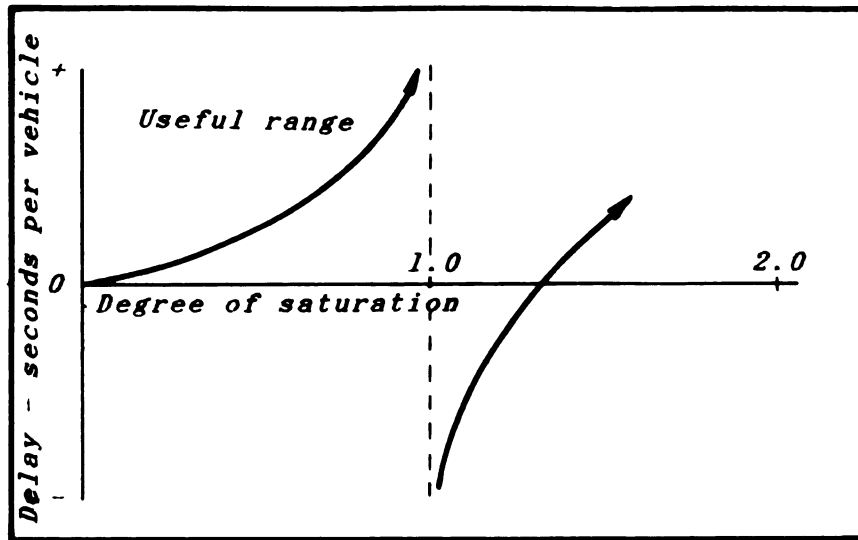


Figure III.2. Webster's model delay

Great care must be taken in applying Webster's equation under conditions of near or oversaturation. Webster's delay model is plotted in Figure III.2. For saturation in excess of capacity, the following formula is used :

$$Q_r = T[q - L * S]$$

where :

Q_r = the number of vehicles not accomodated

during the green,
 T = time period [sec.],
 S = saturation flow [veh./sec.], while the
 queue length at the end of the phase,
 Q_e , is calculated as the sum of Q_b ,
 the queue length at the beginning of
 the period, and Q_r .

Therefore with these values, the total delay is :

$$D = T/2/[Q_b + Q_r].$$

In the case of actuated control, the problem is quite complex, and no reliable delay model exists. The approximation used in SOAP simulates a 'well timed' actuated controller. The actuated control strategy is assumed to :

- distribute green time available in proportion to the demand on the critical approach;
- minimize 'wasted' time by terminating each green interval soon after the queue has been served.

THE CYCLE LENGTH

Cycle length is calculated by using Webster's -

method as well. For fixed timed operation, the optimal cycle length is :

$$C_0 = [(1.5*I) + 5] / (1 - y)$$

where :

I = total lost time due to starting and stopping critical movements, and
 y = the overall degree of saturation [i.e. the proportion of green time required for the traffic movement].

For actuated control, the cycle calculated is the average cycle length, which is determined as :

$$C_a = [1.1*I] / [1 - y]$$

In the low to moderate demand range, this value will always be smaller than C_0 . As the intersection approaches saturation, actuated control approaches fixed time control.

EXCESS FUEL CONSUMPTION

Fuel consumption is determined from the total of the percentages of stops and delay i.e :

$$E = E_s + E_d$$

The excess fuel consumption due to stops is calculated as :

$$E_s = f_s * q * P_s$$

where :

- E_s = gallons of fuel consumed due to stops,
- f_s = fuel consumption rate [gal/stop],
- q = traffic volume [veh/hr],
- P_s = percent of stops, which is equal to the number of vehicles joining the queue while it is still discharging, all divided by the number of arrivals per-cycle, or :

$$P_s = [r*S]/c/[S - v],$$

where :

- r = red time [sec],
- S = saturation flow during green [veh/sec],

Then, the excess fuel consumption associated with delay is :

$$E_d = f_i * q * d / 3600$$

where :

- f_i = fuel consumption rate per veh-hr of idling,
 d = average vehicle delay [sec/veh].

LEFT TURN CONFLICTS

A conflict can occur when left turns are permissive, or not exclusively protected. The measure of effectiveness is described as the number of left turns which cannot be accommodated safely. The turning vehicles may cross traffic, if sufficient gaps in the oncoming traffic exist. Tanner's model is used to calculate the effective left turning saturation flow. This model relates the opposing flow to left turn traffic. Given the opposing flow, the left turn saturation flow is taken from a curve and compared to the left turn demand. Excess demand is the number of left turn conflicts.

III.2.5. LIMITATIONS AND OTHER FEATURES

As described before, SOAP has the capability of design and analysis for any traffic control strategy, either pretimed or actuated operations. However, several limitations warrant notice. The optimization and analysis are obviously based on mathematical models which often

cannot take human behavior into consideration. The SOAP program does not exactly simulate the traffic in real world events. For instance, the combining of right turns with thru traffic presents a problem with the accurate estimation of the capacity. Some other problems include : the incapability of analyzing closed loops, lack of wide-spread testing, and lack of validation of the platoon dispersion algorithm.

CHAPTER FOUR: TRANSYT-7F & SOAP84 CONSISTENCY TEST OF THE MOE'S

TRANSYT-7F and SOAP84 are well accepted as producing realistic results with nearly similar computational bases. Both are deterministic, macroscopic, time scan, simulation and optimization models based on simple sets of equations. They offer practical methods of signal timing which are easy to use. TRANSYT-7F is generally used for the traffic signals along an arterial or network of up to 50 nodes with approximately 250 directional links, while SOAP84 provides a method of developing signal control plans at isolated intersections. This would of course raise an intriguing question: can TRANSYT-7F and SOAP84 produce consistent results under the same traffic condition?. A positive answer could reflect favorably on both models. Therefore, it is the objective of this study to reveal the answer to this question.

IV.1. DESCRIPTION OF THE STUDY SITE

In the downtown area of Lansing, the capitol of the state of MICHIGAN, a section of Michigan Avenue was selected as the case study. Michigan Avenue carries two way traffic, two lanes in each direction, 12 ft width each, with a left turning lane between directions.

Parking is permitted on both sides of the road.

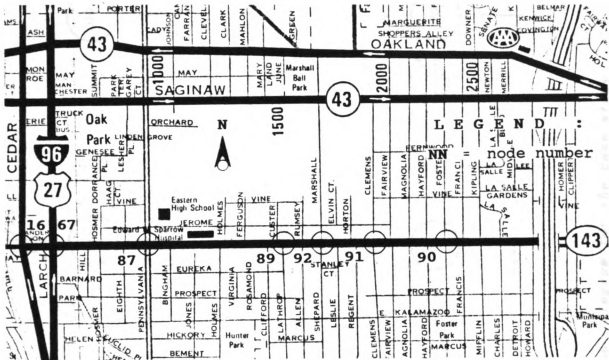
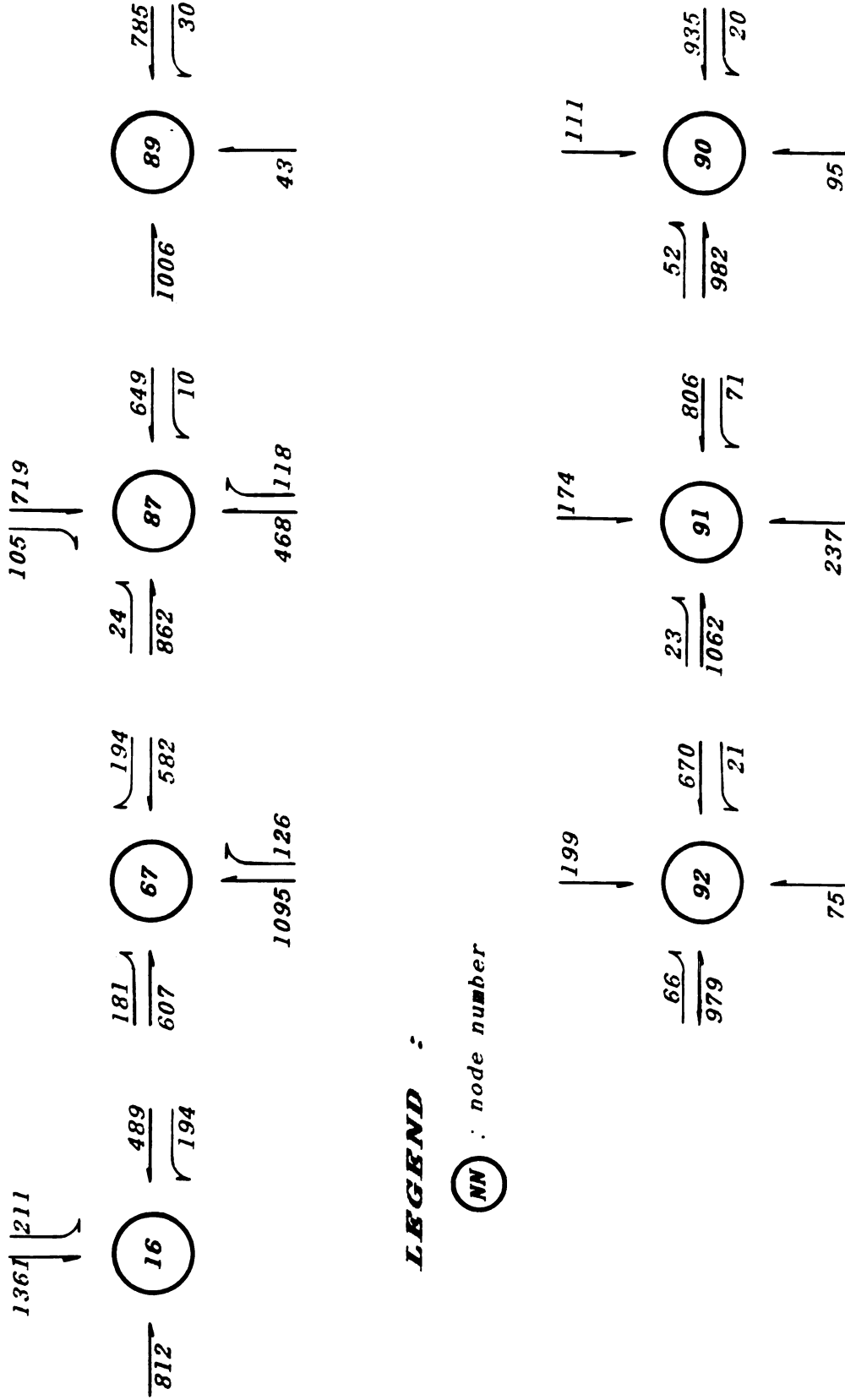


Figure IV.1. Section of arterial under study

The section under study consists of seven coordinated signalized intersections with 60 second cycle length, all are pretimed and operated in two phases.

IV.2. STUDY APPROACH

The TRANSYT-7F program was first run to determine the optimal cycle length, splits, and systemwide performance with optimal settings corresponding to the



LEGEND :

NN : node number

Figure IV.2. Traffic flow diagram for the case study

traffic flows at the observation period of 16.00 - 17.00 hours. The optimal setting times resulting from the TRANSYT-7F run then were specified as inputs for the SOAP84 evaluation. The outputs generated by TRANSYT-7F and SOAP84 program are provided in the Appendix-1, and Appendix-2 respectively.

Total delays, stops and fuel consumptions predicted by both models were selected as performance measures for testing of their consistency. Scatter plots and linear regression equations were established to determine the relationship patterns of performance measures estimated by both models.

IV.3. RESULTS AND ANALYSIS OF PERFORMANCE MEASURES

Delay and stops are well recognized as very useful measures of effectiveness in a traffic control system.

DELAY

SOAP utilizes Webster's model to estimate the average delay per-vehicle. SOAP defines delay as the difference in average travel time through the intersection and the travel time for a vehicle which is not stopped or slowed down by a signal. Webster's delay model was

modified by Mr. Robertson for use in the TRANSYT program to give more reasonable results, particularly for traffic volumes beyond the saturation point. TRANSYT and SOAP consider delay mostly as a waiting time in the queue caused by the red phase. Total delay is then defined by both models as the product of the average delays and the total intersection volumes.

STOPS

The proportion of vehicles stopped by the signal is also important. Stops are very significant factors in the estimation of both fuel consumption and potential accidents at a traffic signal. SOAP describes the proportion of vehicles stopped at a signal as equal to the number of vehicles in the queue at the beginning of the green time plus the number of vehicles which join the queue while the queue is still discharging, divided by the average number of arrivals per cycle. TRANSYT assumes that vehicles which are delayed are also stopped, though this is not always the case in the actual traffic flow. The problem of modelling this arises when vehicles do not actually stop. TRRL considers that the short periods of delay can be expressed as a fraction of stops for vehicles affected. [see table III.1.] The total delay, stops, and fuel consumption performances predicted by TRANSYT-7F and SOAP84

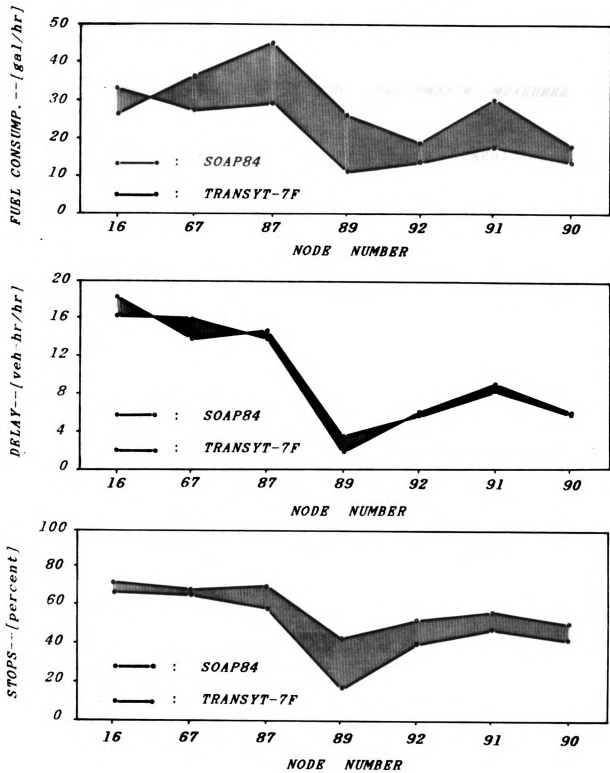


Figure IV.3. Comparison of performance measures

programs are shown in Figure IV.3.

IV.4. CONSISTENCY TEST OF THE PERFORMANCE MEASURES

As stated earlier, TRANSYT-7F and SOAP84 were developed for different purposes. TRANSYT-7F is primarily used for analyzing traffic signals along an arterial or a network, while SOAP84 is a model for analyzing isolated intersections. Therefore, it would be reasonable if in some cases their estimation of the performance measures would be different. However, their reliability will be enhanced if they generate consistent results under similar traffic conditions.

In this case, the regression equations were first established to determine the relationship patterns for the performance measures generated by both models under similar traffic conditions. The linear regression equations presented in Table IV.1. indicate that the results for total delay and stops generated by both models were strongly correlated.

Within the conditions investigated in this study, TRANSYT-7F and SOAP84 produced compatible results in terms of the estimation of delay and stops. However, if we consider the fuel consumption performances predicted in this study, they seem significantly different. The bases of computing fuel consumption are obviously different between these models.

<i>M.O.E. :</i>	<i>REGRESSION EQUATION</i>	<i>CORREL. COEF., [R].</i>	<i>S.S.E.</i>
<i>Tot. del.</i>	<i>STDEL = .69640 + .95 TTDEL</i>	<i>.97</i>	<i>1.52</i>
<i>Stops</i>	<i>SSTOP = 30.66364 + .59 TSTOP</i>	<i>.96</i>	<i>3.64</i>
<i>Fuel cons.</i>	<i>SFCON = 3.69637 + .60 TFCON</i>	<i>.62</i>	<i>7.70</i>

Table IV.1. Relationship between TRANSYT-7F and SOAP84 results

NOTE :

STDEL = SOAP84 total delay.

TTDEL = TRANSYT-7F total delay.

SSTOP = SOAP84 total uniform stops.

TSTOP = TRANSYT-7F total uniform stops.

SFCON = SOAP84 total fuel consumption.

TFCON = TRANSYT-7F total fuel consumption.

TRANSYT-7F generates the total gallons of fuel consumed by all vehicles based on experimental studies. A stepwise multiple regression analysis was used which

resulted in the following equation : [4][29]

$$F = k_1 * TT + k_2 * TD + k_3 * TS$$

where ;

F = gallons of fuel consumption per-hour.

TT = total travel per-hour.

TD = total delay.

TS = total stops.

k1, k2, k3, are coefficients of regression.

SOAP84 on the other hand, estimates only the total fuel consumption due to idling delays and accelerations from stopped positions. Therefore, it would be reasonable if the estimation from SOAP84 is lower than that estimated by TRANSYT-7F. However, this will not always be the case. At node # 16 for example, [see Figure IV.3.], because of a high level of saturation, SOAP84 produces a high value of the estimation of fuel consumption. TRANSYT-7F also poses a problem in the estimation of fuel consumption. The calculation of the average speed which excludes the contribution of external links will give different results for the determination of total travel for intersections with various ratios of external to internal link volumes but the same total volume. The effect of the ratio of external to internal link volumes on fuel consumption estimation

are shown in Table IV.2.

<i>LINK VOLUME</i>				<i>FUEL CONS. [GAL/HR]</i>		
<i>NN</i>	<i>EXTERNAL</i>	<i>INTERNAL</i>	<i>RATIO</i>	<i>TRANSYT-7F</i>	<i>SOAP84</i>	<i>RATIO</i>
89	43	1,821	.02	26.71	10.16	.38
90	206	1,989	.10	19.10	14.57	.76
92	274	1,736	.16	19.64	14.11	.72
91	411	1,962	.21	30.12	18.45	.61
67	1,221	1,564	.78	36.42	27.50	.76
87	1,410	1,545	.91	45.06	29.66	.66
16	2,384	683	3.49	26.45	33.23	1.26

Table IV.2. The effect of the ratio of external to internal link volumes.

CHAPTER FIVE : EXAMINING THE IMPACTS OF SIGNAL OPTIMIZATION

The optimization of traffic signal settings for a particular network are determined by a given set of roadway geometrics [supply variables], and the traffic patterns [demand conditions]. If either the supply or demand changes significantly, the existing signal timings may not be optimal, and retiming signals might be desired. The most important issues addressed in the maintenance of signal timing are :

- level of congestion under existing traffic conditions; and
- the magnitude of traffic growth in the future.

In a heavily trafficked network, small increases in traffic volume might offset the benefits of signal optimization. On the other hand, a network consisting of light traffic might be able to accommodate the increasing traffic volume without significant effect. This chapter addresses the magnitude of the traffic impacts likely to be caused by the increasing traffic, and to evaluate alternative policies for managing it whenever needed.

V.1. DESCRIPTION OF THE STUDY SITE

A network in a suburban area of the city of Cirebon, Indonesia, was selected as the case study for the following reasons :

- the availability of traffic signal timing data;
- the possibility of facing serious traffic signal problems caused by the increasing traffic volume in the future under the existing signal timings; and
- learning the beneficial impacts of signal timing optimization, by applying TRANSYT-7F directly to an Indonesian Network.

This network is located near Cirebon harbor which gives a specific meaning to its traffic. The study area includes four signalized intersections, all of which are pretimed and operated in two phases. The street system operates two way traffic, which consists of one or two 10 ft width lanes in each direction. No parking is permitted on the street. The business activities in the downtown area, and the potential of new developments in the city is expected to increase the traffic volumes, which will affect the traffic pattern throughout the network, and could create a serious

level of congestion.

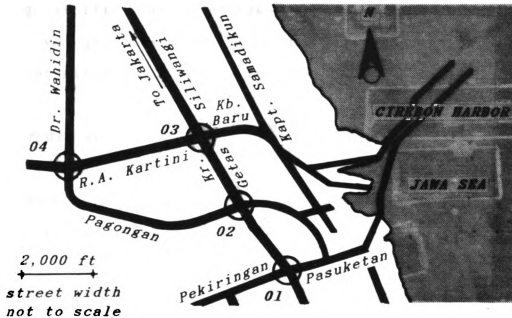


Figure V.1. Network system of the case study

For this particular network, two issues will be addressed :

- how sensitive is the existing signal timing to the increasing traffic volumes throughout the network; and
- what is the maximum level of traffic that can be accepted with signal timing optimization, before major design changes, and

other TSM measures would be considered.

A simple way to examine the network sensitivity to increasing traffic is to assume incremental networkwide traffic volume, then measure the performances before and after optimization, by using the TRANSYT model.

V.2. STUDY APPROACH

TRANSYT-7F runs in this study were performed for two kinds of submodels :

- a submodel that simulates, at macro level the performance with existing signal - timings; and
- an optimization submodel that seeks the - 'best' signal settings.

The '*best*' signal setting, as previously described, was chosen based on a performance index - a weighted sum of stops and delays. TRANSYT input data characterize network geometry [road sections, intersections, and the saturation flows], signal timing parameters [cycle lengths, phasings, and minimum pedestrian timings if any], and the traffic volumes.

For the purpose of this study, traffic volumes were varied sequentially as a proportion of the sa-

turation flow, S , to the values of : $.10S$, $.20S$, $.30S$, and $.40S$, for both simulation and optimization runs. The turning traffic was assumed to be 20 percent of the total volumes, for either left or right turn flows. The direct outputs of TRANSYT include delay and stops along with the optimized signal settings. The values of the saturation flows were determined by considering the street geometries, percentage of trucks and the turning volumes. [22] To measure the impacts of these changes, TRANSYT system performance was converted into highway user costs and direct energy consumption. While the assumption that volumes increase uniformly throughout the network might not be realistic for design purposes, it nevertheless provides the first cut estimation of the impacts at a sketch planning level. An alternative way to determine the frequency of retiming could be by monitoring traffic growth through systematic traffic counts, observe the operation of the intersections, identify the problems, and then apply the TRANSYT model to evaluate the need for installation of new timings.

V.3. RESULTS AND ANALYSIS

To code the data for TRANSYT-7F runs, an illustration of the link-node diagram is presented in Fig. V.2. TRANSYT-7F simulation runs were first performed

for the existing signal timings. Traffic flows were varied in the order of : .10S, .20S, .30S, and .40S

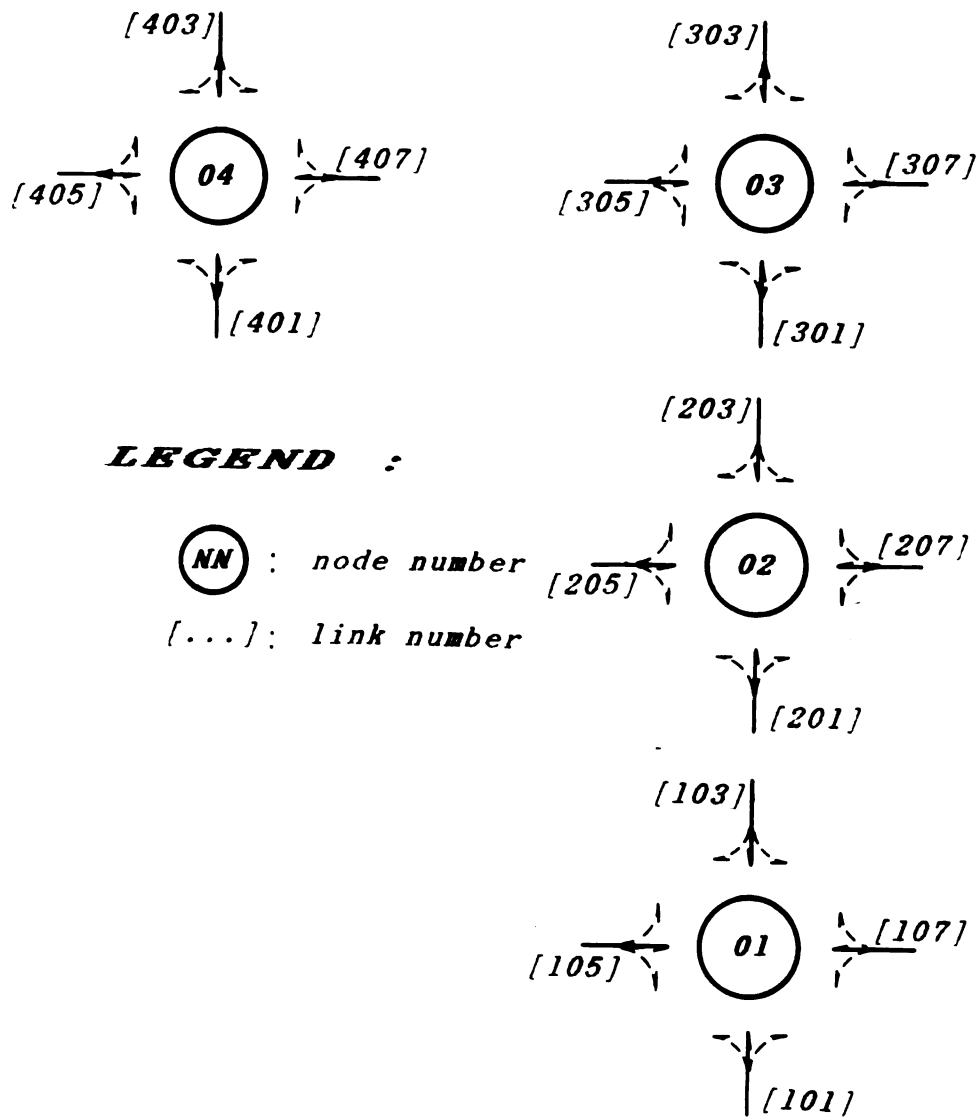
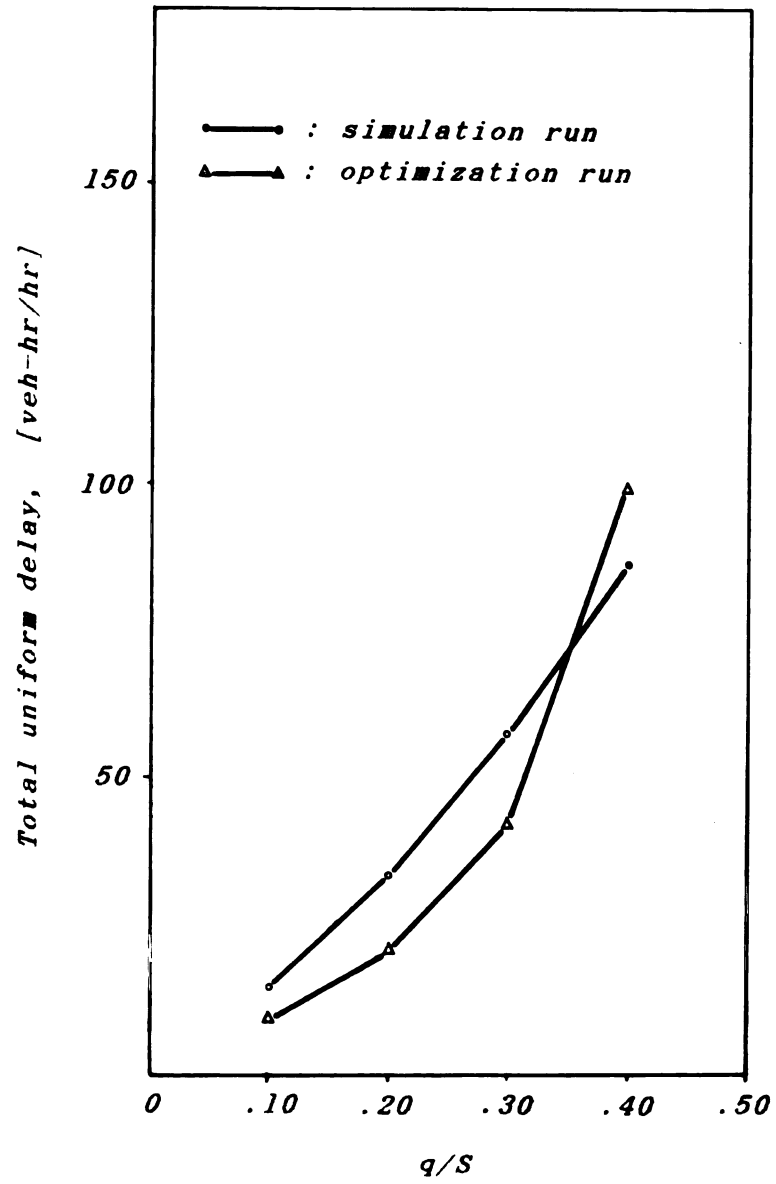
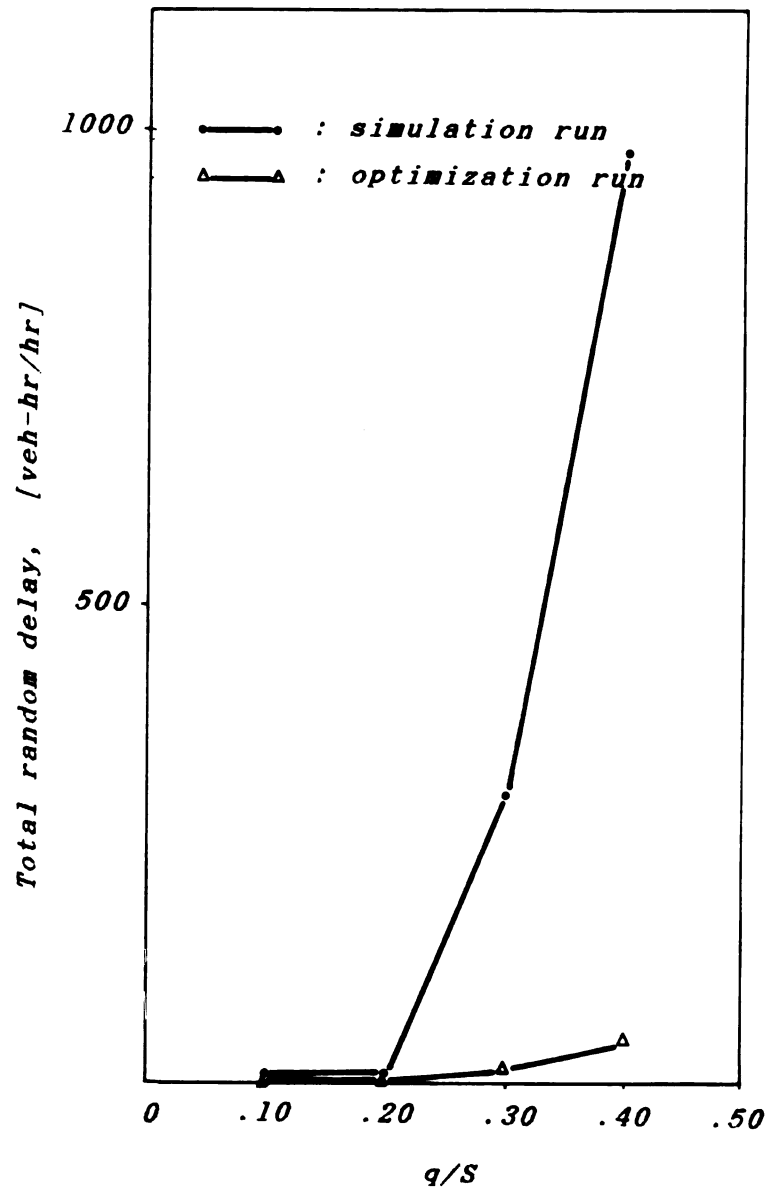


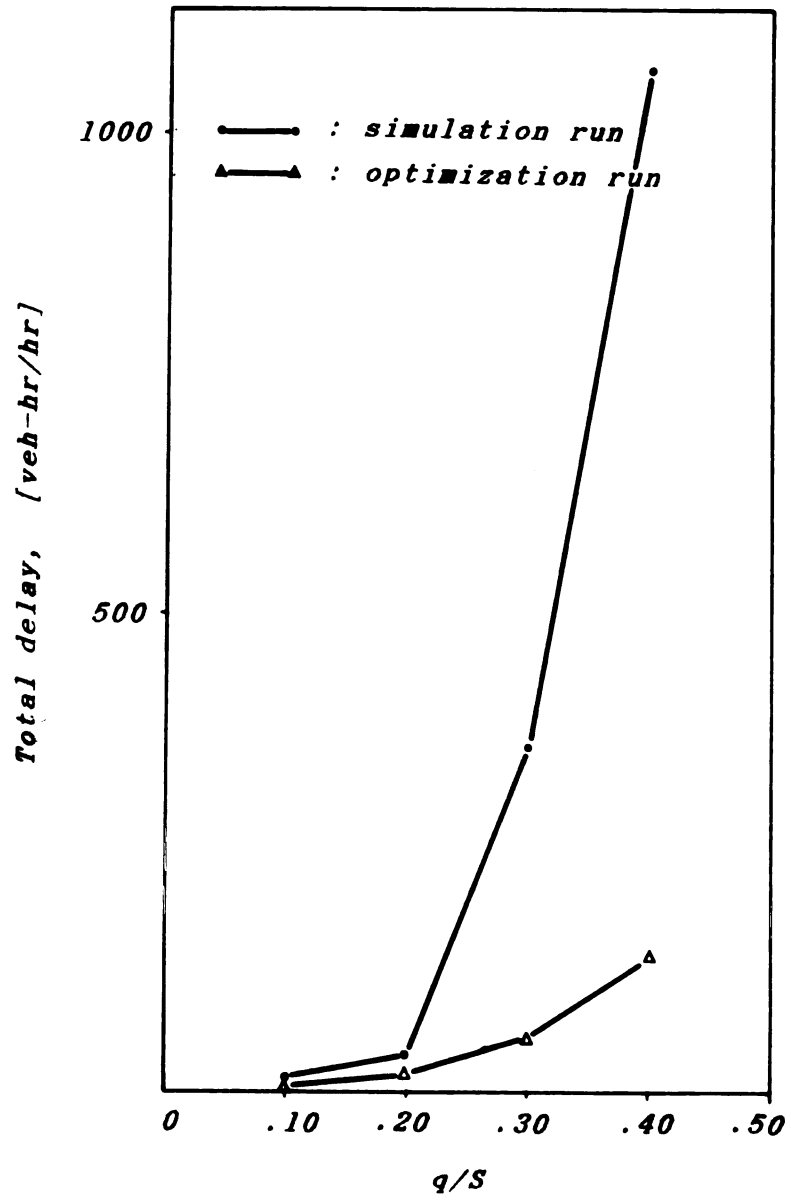
Figure V.2. Link-node diagram for the case study

Figure V.3. Network-wide traffic performance

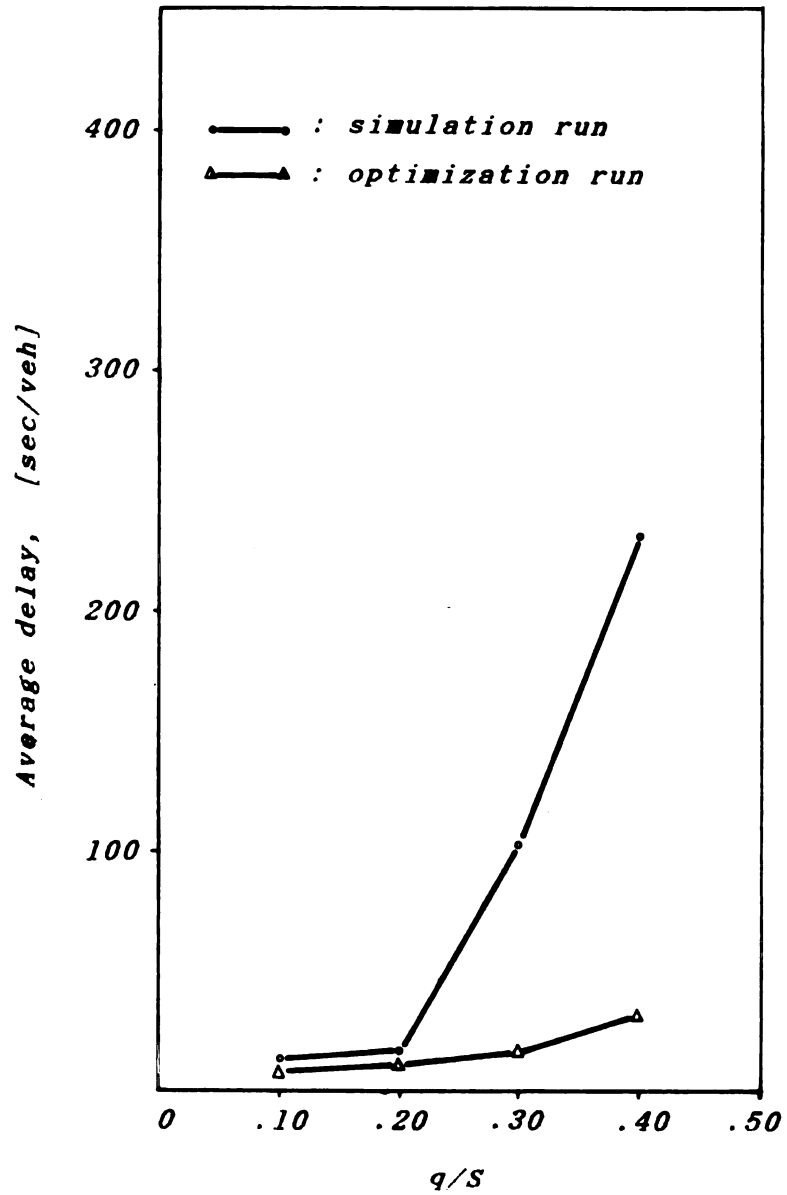
[a] total uniform delay vs q/S



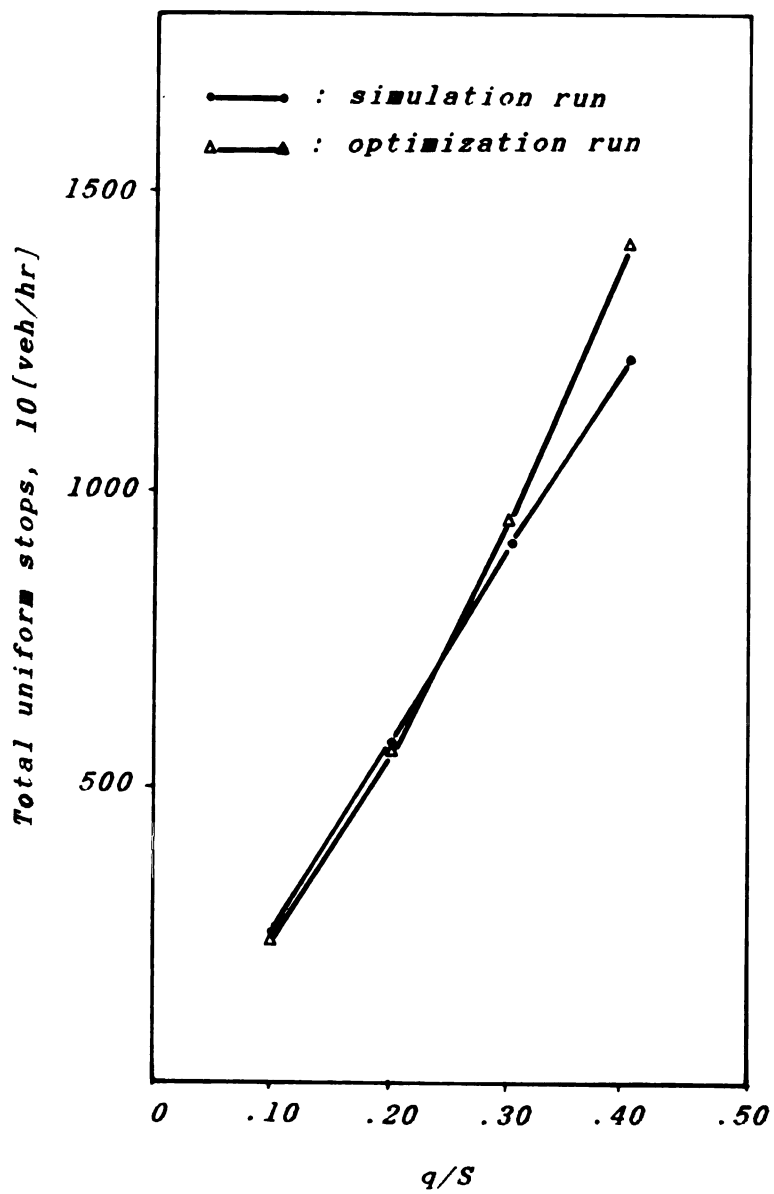
[b] Total random delay vs q/S



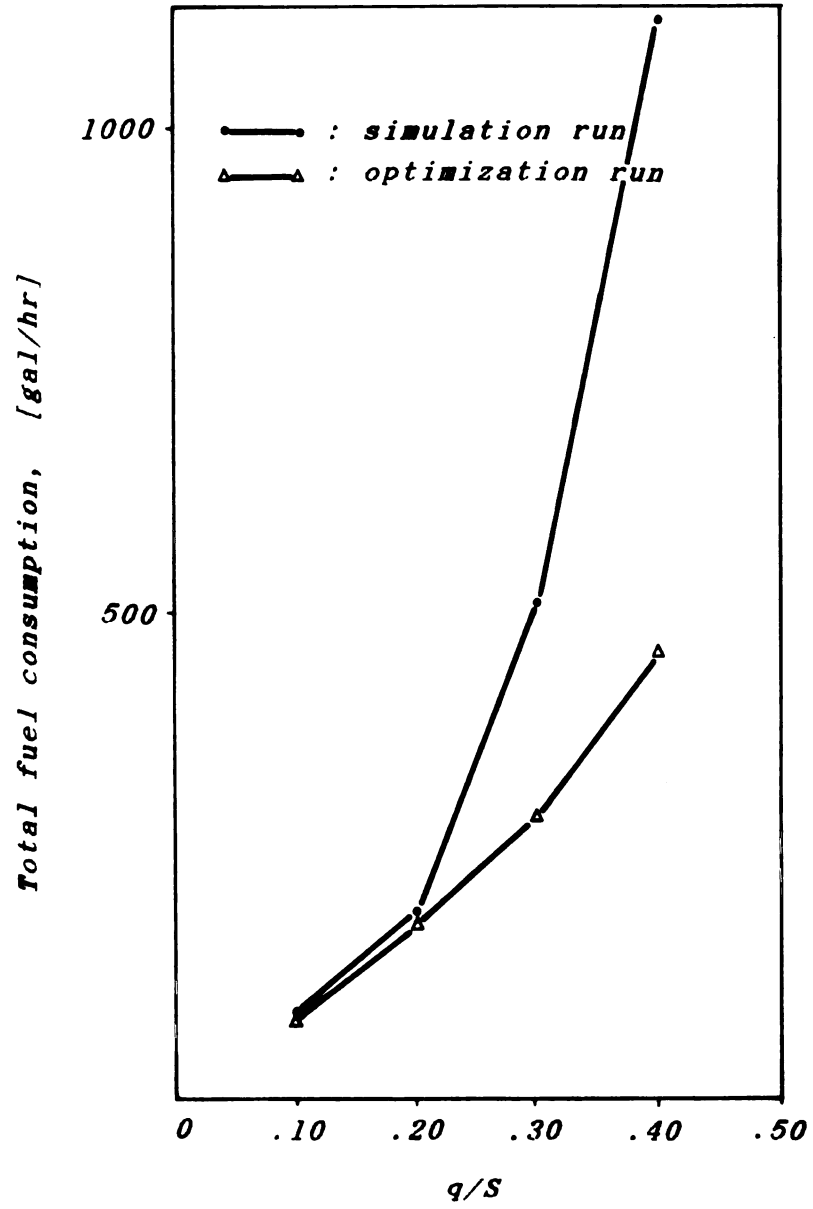
[c] Total delay vs q/S



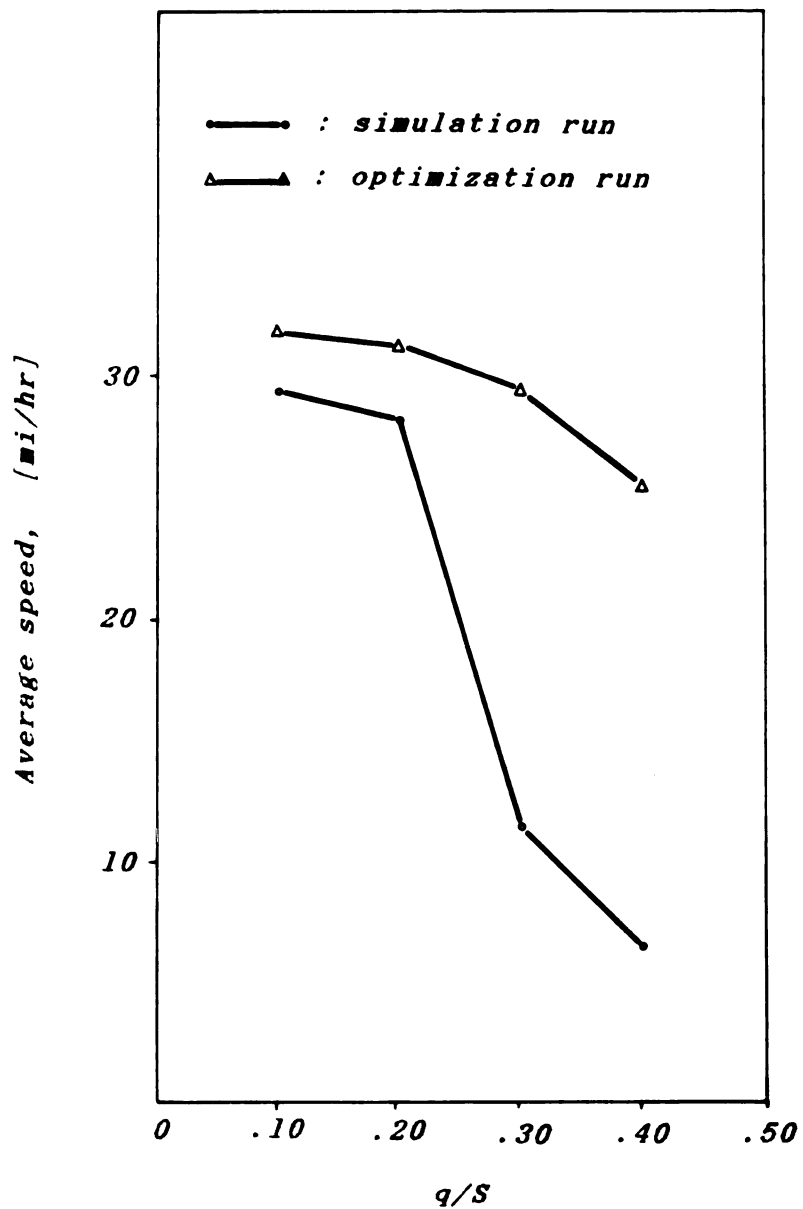
[d] Average delay vs q/S



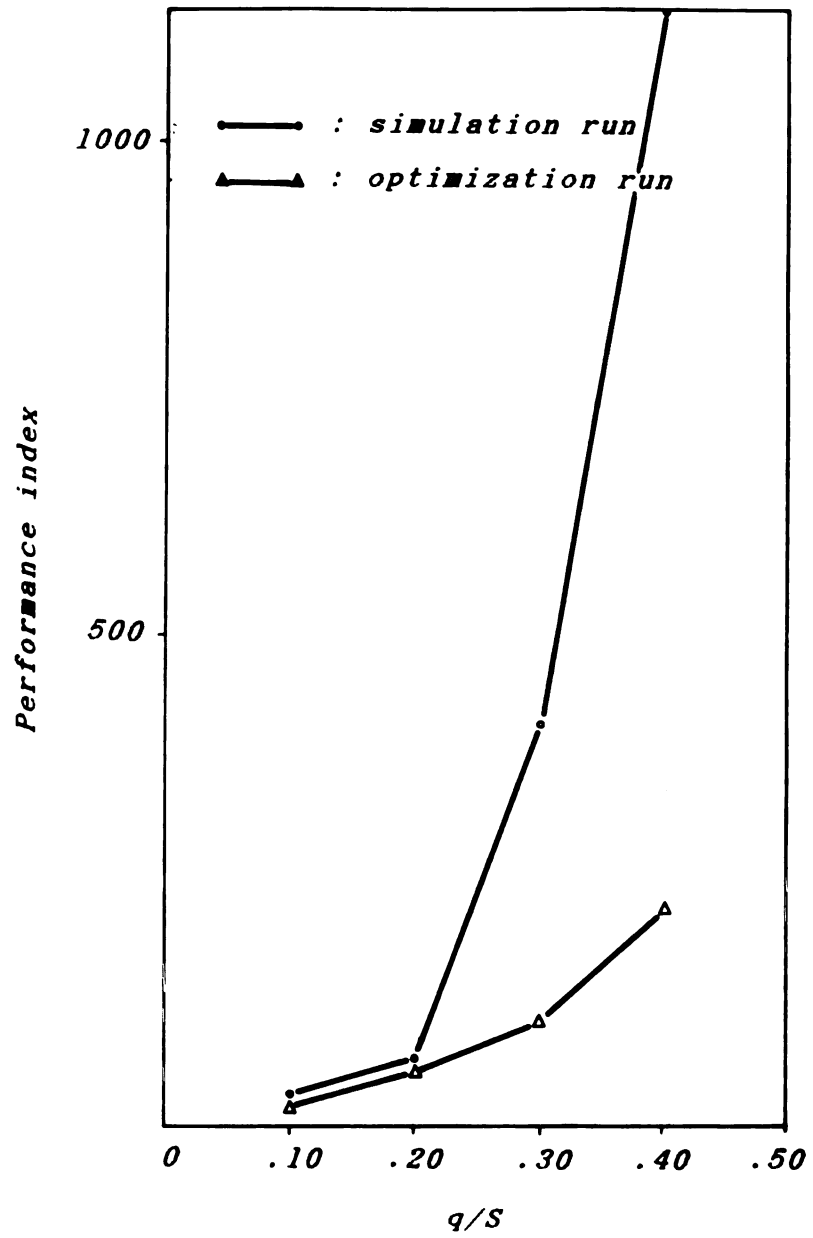
[e] Total uniform stops vs q/S



[f] Total fuel consumption vs q/S



[g] Average speed vs q/S



[h] Performance index vs q/S

where S is the street saturation flow. The optimization runs then were conducted to seek the optimal signal settings for the same variation of traffic flows. The complete outputs generated by both submodels are presented in APPENDIX-4. Figure V.3. a, b, c, d, e, f, g, and h, illustrate the systemwide performances predicted by both submodels. The optimization run for volume variations of $.10S$ and $.20S$, does not significantly affect the traffic performance. However, as volumes increase to $.30S$, the network experiences congestion problems. Some links begin to be oversaturated. For the traffic volumes of $.40S$, the number of oversaturated links are dramatically increased and the network becomes sensitive to any additional traffic volumes. For example, an additional traffic volume from $.10S$ to $.20S$, would result in an increased average delay of $[(16.83 - 13.42)/(13.42) * 100 \%] = 25.41 \%$, but as the volume increases to $.30S$, the average delay increases to $[(103.28 - 16.83)/(16.83) * 100 \%] = 513.67 \%$, a significant increase in terms of average delay !.

The intersection performance for the existing signal timing at traffic volumes greater than or equal to $.40S$ deteriorates rapidly, as the number of critical intersections increases rapidly. For traffic volumes of $.40S$ all of the intersections become oversaturated. Signal timing optimization could considerably improve the

intersection performance as illustrated in Figure V.3. However, the benefit of optimization diminishes when the overall traffic volume increases to more than .40S. Though improvements exist compared to the existing condition performance under similar traffic volumes, the level of service at the intersection drastically decreases. Total stops also increase, and the optimal cycle length is relatively long. The degree of saturation remains above 85 %, which means the intersections would still be critical !. [3] Thus, unless such traffic growth could be avoided throughout the system, or other changes in travel behavior occurred, another TSM strategy would be needed.

V.4. ENERGY AND COST EVALUATION

As was illustrated in Figure V.3., a signal timing optimization would improve the network performance for increasing traffic volumes under certain values of traffic flow. If we consider the case where traffic volumes increase to .30S, the delay would be reduced by $[(358.03 - 54.75)/(358.03) * 100 \%] = 84.71 \%$, Total stops would be increased by $[(9104.40 - 9439.80)/(9104.40) * 100 \%] = 3.68 \%$, while total travel time could be reduced by $[(498.51 - 195.24)/(498.51) * 100 \%] = 60.84 \%$. Fuel consumption would be conserved by $[(512.22 - 292.01)/(512.22) * 100 \%] = 42.99 \%$, and the average speed would

be increased by $[(11.47 - 29.31)/(11.47) * 100 \%] = 155.54 \%$. Furthermore if we assume that this condition occurs only in the afternoon peak period, a simple estimation of benefits could be illustrated in Table V.1. [1][3]

By assuming that this condition would occur for two hours per day, and 300 work-days per year, then the total annual benefit would be U.S. \$ 535,380.

<i>CONSERVATION :</i>	<i>[\$] UNIT COST :</i>	<i>[\$] COST :</i>
- delay saved [hrs] : 303.28	2.00	606.56
- stops saved [vph] : -335.40	.0016	- .54
- fuel saved [glns] : 220.21	1.30	286.27
- <i>[\$] TOTAL COST :</i>		892.30/pk-hr

Table V.1. Benefit estimated for volumes of .30S

The contribution from fuel consumption alone would account for U.S. \$ 171,800. Furthermore, if we assume that the saving during the morning peak period are 75 percent of the afternoon peak, and the savings in the midday are 50 percent of those in the afternoon peak,

then the total annual benefit from fuel saving only could be U.S. \$ 386,500.; an attractive alternative to be considered!. Similarly, for the traffic flows of .40S, the annual benefit from savings of fuel consumption alone would be U.S. \$ 644,500.

CHAPTER SIX: CONCLUSIONS

VI.1. SUMMARY

Total delay, stops and fuel consumption forecasted by TRANSYT-7F were compared with the results generated by SOAP84 in this study. The results for delays and stops were similar and consistent with what was expected. The linear regression equations presented in table IV.1. indicate that the results were strongly correlated. In the case of fuel consumption, TRANSYT-7F produced some-what higher values than those forecasted by SOAP84. This can be explained by the difference in basic computational procedure for fuel consumption in both models. TRANSYT-7F is used mainly for analyzing the signalization along an arterial street or a network. Therefore it includes the traffic flow between the nodes. SOAP84 only considers the fuel consumption at isolated intersections. One exception to these results is shown at node 16, where the total fuel consumption predicted by SOAP84 is higher than that predicted by TRANSYT-7F. This can be explained by the formulation of delay for both models, since fuel consumption is a linear function of delay. SOAP84 utilizes the Webster's model, which overestimates delay for high values of the degree of saturation. TRANSYT-7F uses a modified Webster's equation, and does not increase as

rapidly as the level of saturation increases. After we recognize the limitation of Webster's model, the results from node 16 are easily understood.

As the case study of the CIREBON Network illustrated, TRANSYT-7F can be used for assessing the impacts caused by increasing traffic flows. Such evaluations are useful and can provide an indication of when traffic signal retiming would be warranted. Signal optimization is a low cost alternative for expediting traffic flow, reducing highway user costs and energy consumption. TRANSYT-7F offers the analyst the advantage of assessing impacts networkwide, instead of intersection by intersection. Several other positive features of the TRANSYT-7F program include :

- The traffic flow model traces flow patterns from link to link and incorporates a platoon model to allow one to study the effects of vehicular dispersion. This is particularly important where there is mixed vehicle traffic.
- The versatility of the optimization process in which both offsets and splits can be manipulated sequentially, and
- The consideration of both uniform and random delay to traffic in a signal network, thus considering both system coordination and the

capacity effects.

VI.2. FUTURE DEVELOPMENT

Based on the experiences gained in this study, some particular problems will be addressed in using computer models for traffic operation analysis in future model enhancements and developments. Those are:

STANDARIZATION OF THE INPUT DATA

Both models use similar input data, however each model uses different coding schemes which must be mastered by the user. This may confuse the unfamiliar users. Unnecessary effort and confusion could be reduced substantially by the development of a common input data coding scheme which can be shared by several traffic signal models.

PERSONEL TRAINING

More training is obviously required in the preparation of inputs and the mechanics of execution of various programs. Understanding how to interpret the output is also required to ensure the potential benefits of

the models.

MAINTENANCE

It is natural to expect the improvement of the capability of any models as time and resources permit.

VI.3. TRANSFERABILITY OF U.S. EXPERIENCE TO INDONESIA

The recent progress achieved by the U.S. in the development of computer models for traffic operation analysis has been proven to be one of the most cost beneficial alternatives to be considered. For Indonesia, or any other third world country, where money constraints strongly influence policy, this could be an attractive conclusion. However, in order to adopt such advanced developments, current patterns and trends must be considered. A great difference exists in traffic patterns, resulting from some different environmental factors, which may limit the direct transferability of results.

In Indonesia, motorization has grown rapidly since the beginning of the 1970's, especially in the urban areas. Even-though Indonesia is an *OPEC* country, the wasting of energy will increase the government subsidy greatly. Particularly in these recent years, where the world market price of oil has dropped drastically, the

conservation of domestic consumption is important.

The operation of mixed-traffic in the roadways, which is uncommon in the U.S., needs specific consideration as well. Vehicles operate in various speeds which effects the capacity of the roadway, the smoothness of flow, and the pattern of traffic flows.

Driver behavior, which is different from that in the U.S., could result in different traffic characteristic which need to be carefully studied. The factors effected might be ; the average speed, the value of headway, the conversion factors into passenger car units, etc.

Pedestrian behavior, government regulations, technology of public transportation, communication systems, and some other factors could significantly effect the traffic patterns which need to be carefully studied. By considering these kind of factors, and adjusting them wisely, the models may still be useful, and the benefit of using them can be achieved.

APPENDICES

APPENDIX-1

***TRANSYT-7F OPTIMIZATION RUN FOR STUDY CASE-I,
IN MICHIGAN AVENUE, LANSING, MICHIGAN***

0-----
INTERSECTION 16

IRUN TITLE: RUN---[SS/MI.AVE/TTF/CH99]---OPTIMIZATION RUN FOR THE ARTERIAL SIGNAL TIMINGS---

FIELDS: 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16

LINE NO.	CARD TYPE	MODE NO.	OFFSET/ YLD.PT.	REF INT	CONTROLLER TIMING DATA												DOUBLE CYCLE	
					INT1	INT2	INT3	INT4	INT5	INT6	INT7	INT8	INT9	INT10	INT11	INT12		
5)	12	16	52	1	18	4	34	4	0	0	0	0	0	0	0	0	0	0

LINE NO.	CARD TYPE	MODE NO.	START INTVL	VARIAB. INTVL	YELLOW	RED	MINIM. SECS.	LINKS MOVING IN THIS PHASE												CONT. FLAG
								1	2	3	4	5	6	7	8	9	10	11	12	
6)	21	16	1	1	2	0	22	1605	1607	1608	0	0	0	0	0	0	0	0	0	
7)	22	16	3	3	4	0	38	1603	1604	0	0	0	0	0	0	0	0	0	0	

LINE NO.	CARD TYPE	LINK NO.	LINK LENGTH	SAT. FLOW	TOTAL VOL.	MID-BLK. VOL.	SECOND INPUT LINK.....												THIRD INPUT LINK.....	QUEUE CAP.
							NO.	SPD/TT	NO.	SPD/TT	NO.	SPD/TT	NO.	SPD/TT	NO.	SPD/TT	NO.	SPD/TT		
8)	28	1603	0	3700	1361	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
9)	28	1604	0	1200	211	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
10)	28	1605	0	2400	812	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
11)	28	1607	500	2400	489	0	6701	86	35	6707	403	35	0	0	0	0	0	0	0	
12)	28	1608	500	1200	194	0	6701	34	35	6707	160	35	0	0	0	0	0	0	0	

0-----
INTERSECTION 67

IRUN TITLE: RUN---[SS/MI.AVE/TTF/CH99]---OPTIMIZATION RUN FOR THE ARTERIAL SIGNAL TIMINGS---

LINE NO.	CARD TYPE	MODE NO.	OFFSET/ YLD.PT.	REF INT	CONTROLLER TIMING DATA												DOUBLE CYCLE		
					INT1	INT2	INT3	INT4	INT5	INT6	INT7	INT8	INT9	INT10	INT11	INT12			
13)	12	67	52	1	32	4	20	4	0	0	0	0	0	0	0	0	0	0	0

LINE NO.	CARD TYPE	MODE NO.	START INTVL	VARIAB. INTVL	YELLOW	ALL-RED	MINIM. SECS.	LINKS MOVING IN THIS PHASE												CONT. FLAG
								1	2	3	4	5	6	7	8	9	10	11	12	
14)	21	67	1	1	2	0	36	6705	6706	6707	6712	0	0	0	0	0	0	0	0	
15)	22	67	3	3	4	0	24	6701	6709	0	0	0	0	0	0	0	0	0	0	

LINE NO.	CARD TYPE	LINK NO.	LINK LENGTH	SAT. FLOW	TOTAL VOL.	MID-BLK. VOL.	SECOND INPUT LINK.....												THIRD INPUT LINK.....	QUEUE CAP.
							NO.	SPD/TT	NO.	SPD/TT	NO.	SPD/TT	NO.	SPD/TT	NO.	SPD/TT	NO.	SPD/TT		
16)	28	6701	0	3700	1095	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
17)	28	6705	500	2400	607	0	1604	155	35	1605	452	35	0	0	0	0	0	0	0	
18)	28	6706	500	1200	181	0	1604	46	35	1605	135	35	0	0	0	0	0	0	0	
19)	28	6707	1750	1200	582	0	8701	10	35	8707	447	35	8710	78	35	0	0	0	0	
20)	28	6709	0	1200	126	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
21)	28	6712	1750	1200	194	0	8707	149	35	8710	27	35	0	0	0	0	0	0	0	

0-----
 INTERSECTION 92

IRUN TITLE: RUN--([SS/MI.AVE/T7F/CH99])---OPTIMIZATION RUN FOR THE ARTERIAL SIGNAL TIMINGS---

FIELDS: 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16

LINE NO.	CARD TYPE	NODE NO.	OFFSET/ YLD.PT.	REF INT	INTERVAL DURATIONS (SECS. OR PERCENT)				CONTROLLER TIMING DATA				DOUBLE CYCLE		
					INT1	INT2	INT3	INT4	INT5	INT6	INT7	INT8		INT9	INT10
40)	12	92	55	1	32	4	20	4	0	0	0	0	0	0	0
0									PHASE TIMING DATA						
									LINKS MOVING IN THIS PHASE						
41)	21	92	1	1	2	0	36	9205	9206	9207	9208	0	0	0	0
42)	22	92	3	3	4	0	24	9201	9203	0	0	0	0	0	0
0								LINK DATA							
								SECOND INPUT LINK.....							
								THIRD INPUT LINK.....							
								QUEUE							
								SPD/TT							
								CAP.							
43)	28	9201	0	1400	75	0	0	0	0	0	0	0	0	0	0
44)	28	9203	0	1400	199	0	0	0	0	0	0	0	0	0	0
45)	28	9205	600	2400	979	0	8901	20	35	8905	940	0	0	0	0
46)	28	9206	600	1200	66	0	8901	10	35	8905	56	35	0	0	0
47)	28	9207	1000	2400	670	0	9101	60	35	9103	10	35	9107	600	35
48)	28	9208	1000	1200	21	0	9107	21	35	0	0	0	0	0	0

CONT. FLAG

0-----
 INTERSECTION 91

IRUN TITLE: RUN--([SS/MI.AVE/T7F/CH99])---OPTIMIZATION RUN FOR THE ARTERIAL SIGNAL TIMINGS---

FIELDS: 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16

LINE NO.	CARD TYPE	NODE NO.	OFFSET/ YLD.PT.	REF INT	INTERVAL DURATIONS (SECS. OR PERCENT)				CONTROLLER TIMING DATA				DOUBLE CYCLE		
					INT1	INT2	INT3	INT4	INT5	INT6	INT7	INT8		INT9	INT10
49)	12	91	26	1	31	4	21	4	0	0	0	0	0	0	0
0									PHASE TIMING DATA						
									LINKS MOVING IN THIS PHASE						
50)	21	91	1	1	2	0	35	9105	9106	9107	9108	0	0	0	0
51)	22	91	3	3	4	0	25	9101	9103	0	0	0	0	0	0
0								LINK DATA							
								SECOND INPUT LINK.....							
								THIRD INPUT LINK.....							
								QUEUE							
								SPD/TT							
								CAP.							
52)	28	9101	0	1400	237	0	0	0	0	0	0	0	0	0	0
53)	28	9103	0	1400	174	0	0	0	0	0	0	0	0	0	0
54)	28	9105	1000	2400	1052	0	9201	14	35	9203	40	35	9205	948	35
55)	28	9106	1000	1200	23	0	9203	10	35	9205	13	35	0	0	0
56)	28	9107	1270	2400	806	0	9007	800	35	0	0	0	0	0	0
57)	28	9108	1270	1200	71	0	9007	70	35	0	0	0	0	0	0

CONT. FLAG

0 CYCLE EVALUATION SUMMARY PERFORMANCE

0	CYCLE LENGTH (SEC)	STEP SIZE (STEPS)	AVERAGE DELAY (SEC/VEH)	PERCENT STOPS (%)	FUEL CONSUMPTION (GAL/HR)	PERFORMANCE INDEX	NUMBER SATURATED LINKS
0	60	30	27.15	55	254.3	196.3	1
0	62	31	21.47	56	235.3	170.0	1
0	64	32	16.66	57	219.2	147.8	1
0	66	33	14.17	56	209.6	135.0	1
0	68	34	13.63	56	207.6	132.3	0
0	70	35	13.37	55	205.5	129.9	0
0	72	36	13.61	54	205.2	130.0	0
0	74	37	13.59	53	204.0	128.9	0
0	76	38	13.47	52	202.5	127.4	0
0	78	39	13.62	52	202.7	127.8	0
0	80	40	13.68	52	202.7	128.0	0
0	82	41	13.79	52	202.3	127.8	0
0	84	42	13.47	50	199.5	124.6	0
0	86	43	13.52	50	199.0	124.2	0
0	88	44	13.61	50	199.5	124.7	0
0	90	45	13.90	49	200.1	125.8	0
0	92	46	13.87	49	199.9	125.5	0
0	94	47	13.93	49	200.3	125.8	0
0	96	48	14.77	49	202.2	129.1	0
0	98	49	14.93	48	202.1	129.4	0
0	100	50	15.34	48	203.3	131.3	0

0--- PROGRAM NOTE --- TRANSYT-7F OPTIMIZES THE SYSTEM USING THE BEST CYCLE LENGTH AND HILL-CLIMB STEP SIZES AS INDICATED BY CARD TYPE 52.

TRANSYT-7F: RUN---[SS/MI.AVE/TTF/CH99]---OPTIMIZATION RUN FOR THE ARTERIAL SIGNAL TIMINGS--- 86 SECOND CYCLE 43 STEPS

0<PERFORMANCE WITH OPTIMAL SETTINGS>

0	NODE LINK NO	FLOW (VEH/H)	SAT DEGREE (%)	FLOW OF SAT (VEH-MI/H)	TOTAL TRAVEL TIME (VEH-H/H)	UNIFORM RANDOM DELAY (VEH-H/H)	DELAY TOTAL (SEC/VEH)	AVERAGE DELAY (SEC/VEH)	UNIFORM STOPS (VEH/H;%)	MAX BACK OF QUEUE (VEH/LK)	QUEUE CAPACITY (VEH/LK)	FUEL CONSUM (GA/H)	PHASE LENGTH (SEC)	LINK NO			
16	1603	1361	3700	81	.00	8.22	7.36	.86	8.22	21.8	1081.8(79%)	.27	>	0	12.00	44	1603
16	1604	211	1200	39	.00	.94	.88	.06	.94	16.0	127.8(61%)	.3	>	0	1.39	44	1604
16	1605	812	2400	79	.00	5.28	4.57	.72	5.28	23.4	644.8(79%)	16	>	0	7.43	42	1605
16	1607	489	2400	47	46.19	2.64	1.33	.11	1.33	9.8	170.1(35%)	5		20	4.04	42	1607
16	1608	194	1200	38	18.32	1.05	.48	.06	.54	10.0	64.4(33%)	2		20	1.58	42	1608
0	16 :	3067	MAX = 81	81	64.51	18.14	14.51	1.80	16.32	19.2	2089.0(68%)				26.45	PI =	30.8

67	6701	1095	3700	88	.00	9.41	7.88	1.53	9.41	30.9	961.7(88%)	24	>	0	12.21	34	6701
67	6705	607	2400	46	57.33	3.04	1.32	.10	1.42	8.4	195.5(32%)	5	>	20	4.72	52	6705
67	6706	181	1200	28	17.10	.88	2.03	.03	.39	22.8	56.8(31%)	1	>	20	1.37	52	6706
67	6707	582	1200	89	192.75	9.13	2.03	1.66	3.68	7.9	487.2(84%)	12	>	70	13.76	52	6707
67	6709	126	1200	31	.00	.75	.71	.04	.75	21.4	87.6(70%)	2	>	0	1.03	34	6709
67	6712	194	1200	30	64.25	2.15	.31	.33	.34	6.3	83.4(43%)	2	>	70	3.33	52	6712
0	67 :	2785	MAX =	89	331.43	25.36	12.62	3.38	16.00	20.7	1872.4(67%)	2	>	0	36.42	PI =	29.0
87	8701	468	2800	48	.00	2.85	2.74	.11	2.85	21.9	338.9(72%)	9	>	0	3.96	35	8701
87	8703	719	2800	74	.00	5.23	4.72	.51	5.23	26.2	584.6(81%)	15	>	0	7.06	35	8703
87	8705	862	2400	67	285.49	10.42	2.02	.34	2.36	9.9	400.3(46%)	11	>	70	15.63	51	8705
87	8706	24	1200	4	7.95	.29	.06	.00	.06	9.7	10.0(41%)	0	>	70	4.43	51	8706
87	8707	649	2400	51	331.89	11.31	1.81	.13	1.94	10.8	260.3(40%)	7	>	108	16.00	51	8707
87	8708	10	1200	2	5.11	.17	.02	.00	.02	8.1	3.0(30%)	0	>	108	.23	51	8708
87	8709	118	1200	28	.00	.57	.64	.03	.67	20.3	79.8(68%)	2	>	0	.93	35	8709
87	8710	105	1200	25	.58	.58	.56	.03	.58	20.0	70.1(67%)	2	>	0	.81	35	8710
0	87 :	2955	MAX =	74	630.44	31.52	12.58	1.14	13.72	16.7	1746.9(59%)	2	>	0	45.06	PI =	25.9
89	8901	43	1400	16	.00	.34	.33	.01	.34	28.4	33.9(79%)	1	>	0	.44	22	8901
89	8905	1006	2400	61	514.46	15.50	.74	.24	.98	3.5	147.9(15%)	4	>	108	21.35	64	8905
89	8907	785	2400	48	89.26	3.05	.42	.11	.53	2.4	108.8(14%)	3	>	24	4.61	64	8907
89	8908	30	1200	4	3.41	.12	.02	.00	.02	3.0	8.2(27%)	0	>	24	.32	64	8908
0	89 :	1864	MAX =	61	607.13	19.01	1.51	.36	1.87	3.6	298.8(16%)	0	>	0	26.71	PI =	3.9
90	9001	95	1400	31	.00	.75	.72	.03	.75	28.5	75.3(79%)	2	>	0	.97	24	9001
90	9003	111	1400	36	.00	.90	.85	.05	.90	29.2	88.0(79%)	2	>	0	1.15	24	9003
90	9005	982	2400	62	236.14	8.41	1.49	.25	1.74	6.4	261.9(27%)	7	>	51	12.24	62	9005
90	9006	52	1200	7	12.50	.42	.07	.00	.07	4.9	15.4(30%)	0	>	51	.64	62	9006
90	9007	935	2400	59	.00	2.15	1.94	.21	2.15	8.3	447.3(48%)	11	>	0	4.05	62	9007
90	9008	20	1200	3	.00	.03	.03	.00	.03	4.9	6.1(30%)	0	>	0	.05	62	9008
0	90 :	2195	MAX =	62	248.65	12.66	5.10	.54	5.64	9.3	894.0(41%)	0	>	0	19.10	PI =	11.8
91	9101	237	1400	69	.00	2.33	1.95	.39	2.33	35.4	206.1(87%)	5	>	0	2.85	26	9101
91	9103	174	1400	51	.00	1.49	1.35	.13	1.49	30.7	143.6(83%)	4	>	0	1.88	26	9103
91	9105	1062	2400	69	201.27	8.30	2.23	.39	2.62	8.9	324.9(31%)	8	>	40	12.05	60	9105
91	9106	23	1200	3	4.36	.16	.04	.00	.04	6.0	9.9(43%)	0	>	40	.27	60	9106
91	9107	806	2400	53	193.82	7.81	2.20	.15	2.34	10.5	395.7(49%)	10	>	51	12.11	60	9107
91	9108	71	1200	9	17.07	.62	.14	.00	.14	7.2	28.2(40%)	1	>	51	.97	60	9108
0	91 :	2373	MAX =	69	416.52	20.72	7.90	1.05	8.96	13.6	1108.3(47%)	1	>	0	30.12	PI =	16.7
92	9201	75	1400	23	.00	.56	.54	.02	.56	26.8	57.7(77%)	1	>	0	.73	25	9201
92	9203	199	1400	61	.00	1.82	1.58	.24	1.82	32.9	167.0(84%)	4	>	0	2.26	25	9203
92	9205	979	2400	63	111.32	4.98	1.57	.26	1.84	6.8	230.3(24%)	6	>	24	7.34	61	9205
92	9206	66	1200	8	7.50	.31	.10	.00	.31	5.5	20.2(31%)	0	>	24	.51	61	9206
92	9207	670	2400	43	126.98	5.28	1.62	.08	1.70	9.1	323.7(48%)	8	>	40	8.55	61	9207
92	9208	21	1200	3	3.98	.15	.04	.00	.04	7.3	9.3(44%)	0	>	40	.25	61	9208
0	92 :	2010	MAX =	63	249.79	13.11	5.45	.60	6.05	10.8	808.2(40%)	0	>	0	19.64	PI =	11.7

<SYSTEM WIDE TOTALS INCLUDING ALL LINKS>

0	TOTAL DISTANCE TRAVELED (VEH-MI/H)	TOTAL TRAVEL TIME (VEH-H/H)	TOTAL UNIFORM DELAY (VEH-R/H)	TOTAL RANDOM DELAY (VEH-H/H)	AVERAGE DELAY (SEC/VEH)	TOTAL DELAY (VEH-H/H)	TOTAL UNIFORM STOPS (VEH/H-*)	TOTAL FUEL CONSUM (GA/H)	PERFORMANCE INDEX	SPEED (MI/H)
0	2548.46	140.51	59.68	8.88	14.31	8817.5 (51%)	203.50	129.79	26.49	<TOTALS>
1	TRANSYT-7F: RUN---[SS/MI.AVE/T7F/CH99]---OPTIMIZATION RUN FOR THE ARTERIAL SIGNAL TIMINGS---									
0	TRANSYT-7F SIGNAL CONTROLLER SETTINGS									

NETWORK-WIDE SIGNAL TIMING DATA

OSYSTEM CYCLE LENGTH = 86 SECONDS
 OMASTER OFFSET REFERENCE LOCATION = INTERSECTION NO. 16
 OALL OFFSETS ARE REFERENCED TO THE START OF INTERVAL NO. 1 AT THIS SIGNAL.

INTERSECTION CONTROLLER SETTINGS

INTERSECTION	16
OINTERVAL NUMBER :	1 2 3 4
OINTVL LENGTH(SEC):	38 4 40 4
OINTVL LENGTH (%) :	44 5 46 5
OPIN SETTINGS (%) :	100/0 44 49 95
O PHASE START (PH#):	1 2
OINTERVAL TYPE :	V Y V Y
OLINKS MOVING :	1605 1603
	1607 1604
	1608

OOFFSET = 0 SEC. 0 %
 OTHIS IS THE MASTER CONTROLLER.

O+++ 193 +++ WARNING + THE OFFSET FALLS WITHIN 1% OF AN INTERVAL CHANGE POINT AT THE START OF INTERVAL NO 1.
 1TRANSYT-7F: RUN---[SS/MI.AVE/T7F/CH99]---OPTIMIZATION RUN FOR THE ARTERIAL SIGNAL TIMINGS---- 86 SECOND CYCLE 43 STEPS

```

----- INTERSECTION 67 -----
OINTERVAL NUMBER :      1  2  3  4
OINTVL LENGTH(SEC):    48  4 30  4
OINTVL LENGTH (%) :    55  5 35  5
OPIN SETTINGS (%) :   100/0 55 60 95
OPHASE START (PH#) :      1  2
OINTERVAL TYPE :      V  Y  V  Y
OLINKS MOVING :      6705 6701
                       6706 6709
                       6707
                       6712
OFFSET = 76 SEC.  88 %

```

```

----- INTERSECTION 87 -----
OINTERVAL NUMBER :      1  2  3  4
OINTVL LENGTH(SEC):    47  4 31  4
OINTVL LENGTH (%) :    54  5 36  5
OPIN SETTINGS (%) :   100/0 54 59 95
OPHASE START (PH#) :      1  2
OINTERVAL TYPE :      V  Y  V  Y
OLINKS MOVING :      8705 8701
                       8706 8703
                       8707 8709
                       8708 8710
OFFSET = 32 SEC.  37 %
I TRANSYT-7F: RUN---[SS/MI.AVE/T7F/CH99]---OPTIMIZATION RUN FOR THE ARTERIAL SIGNAL. TIMINGS--- 86 SECOND CYCLE  43 STEPS

```

```

----- INTERSECTION 89 -----
OINTERVAL NUMBER :      1  2  3  4
OINTVL LENGTH(SEC):    60  4 18  4
OINTVL LENGTH (%) :    69  5 21  5
OPIN SETTINGS (%) :   100/0 69 74 95
OPHASE START (PH#) :      1  2
OINTERVAL TYPE :      V  Y  V  Y
OLINKS MOVING :      8905 8901
                       8907
                       8908
OFFSET = 70 SEC.  81 %

```



```

----- INTERSECTION 90 -----
INTERVAL NUMBER :      1      2      3      4
OINTVL LENGTH(SEC) :    58    40  20  4
OINTVL LENGTH (%) :    67    5  23  5
OPIN SETTINGS (%) :   100/0  67  72  95
OPHASE START (PH#) :      1      2      2      2
OINTERVAL TYPE :      V      Y      V      V
OLINKS MOVING :      9005  9001
                   9006  9003
                   9007
                   9008
OFFSET = 2 SEC. 2 %
1TRANSYT-7F: RUN---[SS/MI.AVE/T7F/CH99]---OPTIMIZATION RUN FOR THE ARTERIAL SIGNAL TIMINGS--- 86 SECOND CYCLE 43 STEPS

```

```

----- INTERSECTION 91 -----
INTERVAL NUMBER :      1      2      3      4
OINTVL LENGTH(SEC) :    56    4  22  4
OINTVL LENGTH (%) :    64    5  26  5
OPIN SETTINGS (%) :   100/0  64  69  95
OPHASE START (PH#) :      1      2      2      2
OINTERVAL TYPE :      V      Y      V      V
OLINKS MOVING :      9105  9101
                   9106  9103
                   9107
                   9108
OFFSET = 77 SEC. 90 %

```

```

----- INTERSECTION 92 -----
INTERVAL NUMBER :      1      2      3      4
OINTVL LENGTH(SEC) :    57    4  21  4
OINTVL LENGTH (%) :    66    5  24  5
OPIN SETTINGS (%) :   100/0  66  71  95
OPHASE START (PH#) :      1      2      2      2
OINTERVAL TYPE :      V      Y      V      V
OLINKS MOVING :      9205  9201
                   9206  9203
                   9207
                   9208
OFFSET = 70 SEC. 81 %
1TRANSYT-7F: RUN---[SS/MI.AVE/T7F/CH99]---OPTIMIZATION RUN FOR THE ARTERIAL SIGNAL TIMINGS--- 86 SECOND CYCLE 43 STEPS

```


1TRANSYT-7F: RUN---[SS/MI.AVE/T7F/CH99]---OPTIMIZATION RUN FOR THE ARTERIAL SIGNAL TIMINGS--- 86 SECOND CYCLE 43 STEPS

-KEY- I : ARRIVALS THAT QUEUE (NORMALLY ON RED).
S : DEPARTURES FROM QUEUE (NORMALLY AT THE SATURATION FLOW RATE).
O : ARRIVALS AND DEPARTURES ON GREEN.
-,+ : DELINEATORS ("+" MARKS EVERY TENTH STEP).

N : THE NUMBERS ACROSS THE BOTTOM ARE A TIME SCALE IN UNITS OF STEPS.
NOTE: THE FLOW PROFILE DIAGRAM SHOWS EFFECTIVE GREEN AND RED, NOT ACTUAL.
FUTHERMORE, THE "OFFSET" TO THE LINK EFFECTIVE GREEN HAS NOT BEEN
ADJUSTED TO THE MASTER CONTROLLER, IF THERE IS ONE.

1TRANSYT-7F: RUN---[SS/MI.AVE/T7F/CH99]---OPTIMIZATION RUN FOR THE ARTERIAL SIGNAL TIMINGS--- 86 SECOND CYCLE 43 STEPS
0---- PROGRAM NOTE --- THIS IS THE INPUT DATA REPORT FOR TIME-SPACE DIAGRAM NO. 1

TIME-SPACE DIAGRAM DATA

LINE NO.	CARD NO.	NO. NODES	TIME FLAG	TIME SCALE	DIST. SCALE	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP					
70)	60	7	0	0	300	0	0	0	0	0					
0 LINE NO.	TITLE PLOT NODES : 16, 67, 87, 89, 92, 91, 90. PLOT TITLE CARD														
0 71)	LINK PAIRS ALTERNATING BY DIRECTION PLOT LINK STREAM CARD														
LINE NO.	TYPE	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP					
72)	61	1605	1607	6705	6707	8705	8707	8905	8907	9205	9207	9105	9107	9005	9007
1TRANSYT-7F: RUN---[SS/MI.AVE/T7F/CH99]---OPTIMIZATION RUN FOR THE ARTERIAL SIGNAL TIMINGS---	86 SECOND CYCLE 43 STEPS														

APPENDIX-2

***SOAP84 SIMULATION RUN OF TRANSYT-7F OPTIMAL TIMINGS
FOR STUDY CASE-I, IN MICHIGAN AVENUE, LANSING, MICHIGAN***

VERSION: 84.03

RELEASE: JUNE, 1985

SIGNAL OPERATIONS ANALYSIS PACKAGE

OFFICE OF IMPLEMENTATION ... FEDERAL HIGHWAY ADMINISTRATION

TECHNICAL SUPPORT MESSAGE CENTER: (904) 392-0378

SOAP INPUT ECHO

NO.	CARD ID	A	B	NBT	NBL	SBT	SBL	EBT	EBL	WBT	WBL	COMMENT
3	BEGIN			1600	1700	60		25				MI AVE. AT NN16
4	CONTROL	60	1600	1	86	86	0					
5	HEADWAY	4.0	2.2	2.5	2.5	2.2	2.5	2.2	2.5	2.2	2.5	
6	VOLUME	60	1600			1361	211	812		489	194	
7	CAPACITY	60	1600			3	1	2		2	1	
8	LEFT										2	
9	TIMING	60	1600	44	42							
10	SEQUENCE											T T
11	RUN											NN16 SOAP84 RUN

*** WARNING: THE FOLLOWING MOVEMENTS ARE ASSUMED NOT TO EXIST:

- MOVEMENT NO. 1 NORTHBOUND THRU.
- MOVEMENT NO. 2 NORTHBOUND LEFT.
- MOVEMENT NO. 6 EASTBOUND LEFT.

*** NOTE ... CYCLE LENGTH COMPUTED FROM TIMING CARD.

LEFT TURN CHECK (PER 60 MINUTE PERIOD)

PERIOD	NORTHBOUND	SOUTHBOUND	EASTBOUND	WESTBOUND
NO. TIME	VOL	VOL	VOL	VOL
	CAP	CAP	CAP	CAP
1	1600	0.	211.	820.
		0.	0.	0.
			194.	205.

< SIGNAL OPERATIONS ANALYSIS PACKAGE >

LEFT TURN CHECK (PER 60 MINUTE PERIOD)

PERIOD NO. TIME	NORTHBOUND VOL CAP	SOUTHBOUND VOL CAP	EASTBOUND VOL CAP	WESTBOUND VOL CAP
1 1600	0. 0.	0. 0.	181. 292.	0. 0.

INTERSECTION NAME	RUN NO. AND TITLE	CONTROLLER TYPE	DIAL	SEQUENCE N/S	E/W	PHASES	LOST TIME /PH TOTAL
MI AVE. AT N67	1: N67 SOAP84 RUN PRETIMED	1	T	T	2	2	4.0 8.0

MEASURES OF EFFECTIVENESS

MOVEMENTS:	DELAY (VEH-HRS)	STOPS (%)	EXC FUEL (GAL)	EXC LEFT (VEH)	MAXIMUM QUEUE	V/C RATIO	LEFT TURN TREATMENT	PROTECTION VEH/CYC	PH 1	PH 2	PH 3	PH 4	PH 5	PH 6
NB THRU	7.76	80.0	14.43		23.3	.53			XXXX					
EB THRU	1.78	54.2	4.36		7.9	.33			XXXX					
LEFT	1.82	91.2	2.74	.0	3.9	.62	NONE	2.0	XXXX					
WB THRU	2.45	57.9	5.96		10.7	.42			XXXX					

MEASURES OF EFFECTIVENESS

ANALYSIS PERIOD:	DELAY (VEH-HRS)	STOPS (%)	EX. FUEL (GAL)	EX. LEFT (VEH)	MAXIMUM QUEUE	V/C RATIO	ALL RED (SEC)	DIAL CYCLE NO. (SEC)	PH 1 (%)	PH 2 (%)	PH 3 (%)	PH 4 (%)	PH 5 (%)	PH 6 (%)
1600-1700:	13.81	69.0	27.50	.0	23.3	.62	.0	1 86.0:	39.5	60.6				

SUMMARY : 13.81 69.0 27.50 .0 23.3 .62 TIMINGS WERE INPUT BY THE USER.

< SIGNAL OPERATIONS ANALYSIS PACKAGE >

SOAP INPUT ECHO

NO.	CARD ID	A	B	NBT	NBL	SBT	SBL	EBT	EBL	WBT	WBL	COMMENT
21	BEGIN			1600	1700	60		25				MI AVE. AT NNB7
22	CONTROL	60	1600	1	86	86	0					
23	HEADWAY	4.0	2.2	2.5	2.2	2.5	2.2	2.5	2.2	2.5	2.5	
24	VOLUME	60	1600	586	824	824	862	24	649	10		
25	CAPACITY	60	1600	2	2	2	2	1	2	1	2	
26	LEFT											
27	TIMING	60	1600	35	51							
28	SEQUENCE											
29	RUN	1										NNB7 SOAPB4 RUN

*** WARNING: THE FOLLOWING MOVEMENTS ARE ASSUMED NOT TO EXIST:

- MOVEMENT NO. 2 NORTHBOUND LEFT.
- MOVEMENT NO. 4 SOUTHBOUND LEFT.

*** NOTE ... CYCLE LENGTH COMPUTED FROM TIMING CARD.

LEFT TURN CHECK (PER 60 MINUTE PERIOD)

PERIOD NO.	TIME	NORTHBOUND VOL	SOUTHBOUND CAP	EASTBOUND VOL	CAP	WESTBOUND VOL	CAP
1	1600	0.	0.	0.	24.	341.	10. 252.

NO.	CARD ID A	B	NBT	NBL	SBT	SBL	EBT	EBL	WBT	WBL	COMMENT
30	BEGIN		1600	1700	60		25				MI AVE. AT NN89
31	CONTROL	60	1	86	86	0					
32	HEADWAY	4.0	2.2	2.5	2.2	2.5	2.2	2.5	2.2	2.5	
33	VOLUME	60	43		1006		785	30			
34	CAPACITY	60	1		2		2	1			
35	LEFT										
36	TIMING	60	22	64							
37	SEQUENCE								T	T	
38	RUN	1									NN89 SOAP84 RUN:

NO. CARD ID A B NBT NBL SBT SBL EBT EBL WBT WBL COMMENT

*** WARNING: THE FOLLOWING MOVEMENTS ARE ASSUMED NOT TO EXIST:

- MOVEMENT NO. 2 NORTHBOUND LEFT.
- MOVEMENT NO. 3 SOUTHBOUND THRU.
- MOVEMENT NO. 4 SOUTHBOUND LEFT.
- MOVEMENT NO. 6 EASTBOUND LEFT.

*** NOTE ... CYCLE LENGTH COMPUTED FROM TIMING CARD.

INTERSECTION NAME	RUN NO. AND TITLE	CONTROLLER TYPE	DIAL	SEQUENCE N/S	E/W	PHASES	LOST TIME /PH TOTAL
MI AVE. AT NN87	1: NN87 SOAP84 RUN	PRETIMED	1	T	T	2	4.0 8.0

MEASURES OF EFFECTIVENESS

MOVEMENTS:	DELAY (VEH-HRS)	STOPS (%)	EXC (GAL)	FUEL (VEH)	EXC LEFT (VEH)	MAXIMUM QUEUE	V/C RATIO	LEFT TURN PROTECTION VEH/CYC	TREATMENT	PH 1	PH 2	PH 3	PH 4	PH 5	PH 6
NB THRU :	3.61	77.9	6.73			10.9	.50			XXXX					
SB THRU :	5.78	85.5	10.51			16.8	.70			XXXX					
EB THRU :	2.99	61.6	7.10			12.7	.48			XXXX					
LEFT :	.17	77.6	.29		.0	.4	.07	NONE	2.0						
WB THRU :	2.04	56.6	4.90			8.8	.36			XXXX					
LEFT :	.08	83.0	.13		.0	.2	.04	NONE	2.0	XXXX					

MEASURES OF EFFECTIVENESS

ANALYSIS PERIOD:	DELAY (VEH-HRS)	STOPS (%)	EX. (GAL)	FUEL (VEH)	EX. LEFT (VEH)	MAXIMUM QUEUE	V/C RATIO	ALL RED (SEC)	DIAL CYCLE NO.	PH 1 (%)	PH 2 (%)	PH 3 (%)	PH 4 (%)	PH 5 (%)	PH 6 (%)
1600-1700:	14.67	70.6	29.66	.0	.0	16.8	.70	.0	1	86.0:	40.7	59.3			

SUMMARY : 14.67 70.6 29.66 .0 16.8 .70 TIMINGS WERE INPUT BY THE USER.

< SIGNAL OPERATIONS ANALYSIS PACKAGE >

LEFT TURN CHECK (PER 60 MINUTE PERIOD)

PERIOD NO. TIME	NORTHBOUND VOL CAP	SOUTHBOUND VOL CAP	EASTBOUND VOL CAP	WESTBOUND VOL CAP
1 1600	0. 0.	0. 0.	0. 0.	30. 286.

INTERSECTION NAME	RUN NO. AND TITLE	CONTROLLER TYPE	DIAL N/S	SEQUENCE E/W	PHASES	LOST TIME /PH TOTAL
MI AVE. AT MN89	1: MN89 SOAP84 RUN PRETIMED	1	T	T	2	4.0 8.0

MEASURES OF EFFECTIVENESS

MOVEMENTS:	DELAY (VEH-HRS)	STOPS (%)	EXC FUEL (GAL)	EXC (VEH)	LEFT MAXIMUM QUEUE	V/C RATIO	LEFT TURN TREATMENT PROTECTION	VEH/CYC	PH 1	PH 2	PH 3	PH 4	PH 5	PH 6
NB THRU :	.33	81.2	.55	.8	.13	.13	XXXX							
EB THRU :	1.67	43.7	5.39	10.5	.44	.44	XXXX							
WB THRU :	1.17	39.8	3.83	7.5	.34	.34	XXXX							
LEFT :	.24	81.9	.39	.6	.10	.10	NONE	2.0						

MEASURES OF EFFECTIVENESS

ANALYSIS PERIOD :	DELAY (VEH-HRS)	STOPS (%)	EX. FUEL (GAL)	EX. (VEH)	LEFT MAXIMUM QUEUE	V/C RATIO	ALL RED (SEC)	DIAL CYCLE NO. (SEC)	PH 1 (%)	PH 2 (%)	PH 3 (%)	PH 4 (%)	PH 5 (%)	PH 6 (%)
1600-1700:	3.42	43.5	10.16	.0	10.5	.44	.0	1	86.0:	25.6	74.4			

SUMMARY : 3.42 43.5 10.16 .0 10.5 .44 TIMINGS WERE INPUT BY THE USER.

< SIGNAL OPERATIONS ANALYSIS PACKAGE >

LEFT TURN CHECK (PER 60 MINUTE PERIOD)

PERIOD NO.	TIME	NORTHBOUND VOL	SOUTHBOUND VOL	EASTBOUND VOL	WESTBOUND VOL
1	1600	0.	0.	66.	21.
		0.	0.	415.	277.

NO.	CARD ID	A	B	NBT	NBL	SBT	SBL	EBT	EBL	WBT	WBL	COMMENT
39	:	REGIN		1600	1700	60		25				MI AVE. AT NN92
40	:	CONTROL	60	1600	1	86	0					
41	:	HEADWAY	4.0	2.2	2.5	2.2	2.5	2.2	2.5	2.2	2.5	
42	:	VOLUME	60	1600	75	199		979	66	670	21	
43	:	CAPACITY	60	1600	1		1	2	1	2	1	
44	:	LEFT						2			2	
45	:	TIMING	60	1600	25	61						T T
46	:	SEQUENCE										NN92 SOAP84 RUN
47	:	RUN	1									COMMENT

*** WARNING: THE FOLLOWING MOVEMENTS ARE ASSUMED NOT TO EXIST:

MOVEMENT NO. 2 NORTHBOUND LEFT.
MOVEMENT NO. 4 SOUTHBOUND LEFT.

*** NOTE ... CYCLE LENGTH COMPUTED FROM TIMING CARD.

INTERSECTION NAME RUN NO. AND TITLE CONTROLLER DIAL SEQUENCE N/S E/W PHASES LOST TIME /PH TOTAL
 MI AVE. AT NN92 1: NN92 SOAP84 RUN PRETIMED 1 2 2 4.0 8.0 TIMINGS INPUT

M E A S U R E S O F E F F E C T I V E N E S S

DELAY (VEH-HRS)	STOPS (%)	EXC FUEL (GAL)	EXC LEFT (VEH)	EXC LEFT MAXIMUM	V/C RATIO	LEFT TURN TREATMENT PROTECTION VEH/CYC	PH 1	PH 2	PH 3	PH 4	PH 5	PH 6
.55	79.2	.92		1.4	.19		XXXX					
1.67	86.0	2.71		4.1	.50		XXXX					
1.99	48.1	5.90		11.3	.45		XXXX					
.43	74.6	.75	.0	1.2	.16	NONE	XXXX	2.0				
1.18	42.4	3.55		6.8	.31		XXXX					
.17	82.0	.27	.0	.4	.08	NONE	XXXX	2.0				

M E A S U R E S O F E F F E C T I V E N E S S

DELAY (VEH-HRS)	STOPS (%)	EX. FUEL (GAL)	EX. LEFT (VEH)	EX. LEFT MAXIMUM	V/C RATIO	ALL RED (SEC)	SIGNAL DIAL CYCLE NO. (SEC)	PH 1 (%)	PH 2 (%)	PH 3 (%)	PH 4 (%)	PH 5 (%)	PH 6 (%)
5.98	52.3	14.11	.0	11.3	.50	.0	1	86.0	29.1	70.9			

SUMMARY : 5.98 52.3 14.11 .0 11.3 .50 TIMINGS WERE INPUT BY THE USER.

NO.	CARD ID	A	B	NBT	NBL	SBT	SBL	EBT	EBL	WBT	WBL	COMMENT
48	BEGIN			1600	1700	60		25				MI AVE. AT NN91
49	CONTROL	60	1600	1	86	86	0					
50	HEADWAY	4.0	2.2	2.5	2.2	2.5	2.2	2.2	2.5	2.2	2.5	
51	VOLUME	60	1600	237	174		1062	23	806	71		
52	CAPACITY	60	1600	1	1		2	1	2	1		
53	LEFT							2	2			
54	TIMING	60	1600	26	60							
55	SEQUENCE											
56	RUN	1										NN91 SOAP84 RUN

*** WARNING: THE FOLLOWING MOVEMENTS ARE ASSUMED NOT TO EXIST:

MOVEMENT NO. 2 NORTHBOUND LEFT.
 MOVEMENT NO. 4 SOUTHBOUND LEFT.

*** NOTE ... CYCLE LENGTH COMPUTED FROM TIMING CARD.

< SIGNAL OPERATIONS ANALYSIS PACKAGE >

LEFT TURN CHECK (PER 60 MINUTE PERIOD)

PERIOD NO. TIME	NORTHBOUND VOL	SOUTHBOUND CAP	EASTBOUND VOL	WESTBOUND CAP	WESTBOUND VOL	CAP
1 1600	0.	0.	23.	339.	71.	244.

INTERSECTION NAME	RUN NO. AND TITLE	CONTROLLER TYPE	DIAL	SEQUENCE N/S	E/W	PHASES	LOST TIME /PH TOTAL
MI AVE. AT NN91	1: NN91 SOAPB4 RUN PRETIMED	1	T	2	4.0	8.0	TIMINGS INPUT

MEASURES OF EFFECTIVENESS

MOVEMENTS:	DELAY (VEH-HRS)	STOPS (%)	EXC (GAL)	FUEL (VEH)	EXC (VEH)	LEFT QUEUE	MAXIMUM V/C RATIO	LEFT TURN PROTECTION	TREATMENT VEH/CYC	SEQUENCE					
										PH 1	PH 2	PH 3	PH 4	PH 5	PH 6
NB THRU	2.02	87.0	3.27	4.9	.57			NONE	2.0	XXXX	XXXX	XXXX	XXXX	XXXX	XXXX
SB THRU	1.36	83.3	2.27	3.5	.42			NONE	2.0	XXXX	XXXX	XXXX	XXXX	XXXX	XXXX
EB THRU	2.41	51.6	6.93	13.1	.50			NONE	2.0	XXXX	XXXX	XXXX	XXXX	XXXX	XXXX
LEFT	.16	77.7	.28	.4	.07			NONE	2.0	XXXX	XXXX	XXXX	XXXX	XXXX	XXXX
WB THRU	1.61	46.3	4.70	8.9	.38			NONE	2.0	XXXX	XXXX	XXXX	XXXX	XXXX	XXXX
LEFT	.64	87.4	1.01	1.5	.29			NONE	2.0	XXXX	XXXX	XXXX	XXXX	XXXX	XXXX

MEASURES OF EFFECTIVENESS

ANALYSIS PERIOD:	DELAY (VEH-HRS)	STOPS (%)	EX. FUEL (GAL)	EX. LEFT (VEH)	LEFT QUEUE	MAXIMUM V/C RATIO	ALL RED (SEC)	DIAL CYCLE NO. (SEC)	SIGNAL TIMING						
									PH 1 (%)	PH 2 (%)	PH 3 (%)	PH 4 (%)	PH 5 (%)	PH 6 (%)	
1600-1700:	8.21	57.0	18.45	.0	13.1	.57	.0	1	86.0:	30.2	69.8				

SUMMARY : 8.21 57.0 18.45 .0 13.1 .57 TIMINGS WERE INPUT BY THE USER.

< SIGNAL OPERATIONS ANALYSIS PACKAGE >

SOAP INPUT ECHO

NO.	CARD ID A	B	NBT	NBL	SBT	SBL	EBT	EBL	WBT	WBL	COMMENT
57	BEGIN		1600	1700	60		25				MI AVE. AT NN90
58	CONTROL	60	1600	1	86	0					
59	HEADWAY	4.0	2.2	2.5	2.2	2.5	2.2	2.5	2.2	2.5	
60	VOLUME	60	1600	95	111		982	52	935	20	
61	CAPACITY	60	1600	1	1		2	1	2	1	
62	LEFT						2			2	
63	TIMING	60	1600	24	62						T T
64	SEQUENCE										
65	RUN	1									NN90 SOAP84 RUN

*** WARNING: THE FOLLOWING MOVEMENTS ARE ASSUMED NOT TO EXIST:

- MOVEMENT NO. 2 NORTHBOUND LEFT.
- MOVEMENT NO. 4 SOUTHBOUND LEFT.

*** NOTE ... CYCLE LENGTH COMPUTED FROM TIMING CARD.

LEFT TURN CHECK (PER 60 MINUTE PERIOD)

PERIOD NO.	TIME	NORTHBOUND VOL	SOUTHBOUND CAP	EASTBOUND VOL	WESTBOUND CAP
1	1600	0.	0.	52.	20.

< S I G N A L O P E R A T I O N S A N A L Y S I S P A C K A G E >

DESIGN AND EVALUATION SUMMARY

INTERSECTION NAME RUN NO. AND TITLE CONTROLLER SEQUENCE LOST TIME
 TYPE DIAL N/S E/W /PH TOTAL
 MI AVE. AT NN90 1: NN90 SOAP84 RUN PRETIMED 1 T T 2 4.0 8.0 TIMINGS INPUT

M E A S U R E S O F E F F E C T I V E N E S S

MOVEMENTS:	DELAY (VEH-HRS)	STOPS (%)	EXC FUEL (GAL)	EXC LEFT (VEH)	MAXIMUM QUEUE	V/C RATIO	LEFT TURN TREATMENT PROTECTION	S E Q U E N C E								
								PH 1	PH 2	PH 3	PH 4	PH 5	PH 6			
NB THRU :	.73	81.5	1.21		1.8	.25		XXXX								
SB THRU :	.87	82.3	1.43		2.2	.29		XXXX								
EB THRU :	1.87	46.5	5.69		10.9	.44		XXXX								
LEFT :	.41	82.2	.68	.0	1.0	.17	NONE	2.0								
WB THRU :	1.74	45.6	5.30		10.2	.42		XXXX								
LEFT :	.16	81.5	.26	.0	.4	.07	NONE	2.0								

M E A S U R E S O F E F F E C T I V E N E S S

ANALYSIS PERIOD:	DELAY (VEH-HRS)	STOPS (%)	EX. FUEL (GAL)	EX. LEFT (VEH)	MAXIMUM QUEUE	V/C RATIO	ALL RED (SEC)	DIAL CYCLE NO. (SEC)	S I G N A L T I M I N G							
									PH 1 (%)	PH 2 (%)	PH 3 (%)	PH 4 (%)	PH 5 (%)	PH 6 (%)		
1600-1700:	5.77	50.6	14.57	.0	10.9	.44	.0	1	86.0	27.9	72.1					

SUMMARY : 5.77 50.6 14.57 .0 10.9 .44 TIMINGS WERE INPUT BY THE USER.

NO. CARD ID A B NBT NBL SBT SBL EBT EBL WBT WBL COMMENT

66 :END

+++ END OF SOAP JOB +++

+++ GOOD NEWS: NO ERRORS ENCOUNTERED DURING THIS JOB +++

APPENDIX-3

REGRESSION ANALYSIS - PROGRAM IN BASIC

```

1 REM [SS/REGR-AN.BAS/CH99/APPENDIX-3]
2 REM BUI      03-86
3 REM
4 REM VARIABLES
5 REM B(I)      ESTIMATED COEFFICIENTS
6 REM B1       ESTIMATED CONSTANT
7 REM C(I,J)    MULTIPLE CORRELATION COEFFICIENTS
8 REM E        SUM SQUARE OF ERRORS
9 REM F        FISHER TEST
10 REM M       TOTAL NUMBER OF VARIABLES
11 REM N       NUMBER OF PERIODS OF OBSERVATION
12 REM R(I)    RESIDUAL VALUES
13 REM R2     CORRELATION COEFFICIENT
14 REM S(I,J)  VARIANCE-COVARIANCE MATRIX
15 REM V(I)    VECTOR OF STANDARD ESTIMATED ERRORS
16 REM X(I,J)  MATRIX OF INPUT VALUES
17 REM Y(I)    ESTIMATED Y-VALUES
18 REM
19 REM
20 PRINT : PRINT : PRINT
21 PRINT "MULTIPLE LINEAR REGRESSION"
22 PRINT "*****"
23 PRINT : PRINT : PRINT
24 INPUT "NUMBER OF VARIABLES? ";M: PRINT
25 INPUT "NUMBER OF OBSERVATIONS? ";N: PRINT
26 DIM A$(M),X(M,N),X1(M),S(M,M),S2(M,M),S3(M,M)
27 DIM B(M),Y(N),R(N),V(M),C(M,M)
28 FOR I=1 TO M: PRINT "NAME OF VARIABLE #";I;" ";
29 INPUT A$(I): NEXT I: PRINT
30 FOR I=1 TO M
31 PRINT "DATA GATHERED FOR VARIABLE ";A$(I);":"
32 FOR J=1 TO N
33 PRINT"- OBSERVATION #";J; TAB( 14);
34 INPUT X(I,J)
35 NEXT J
36 PRINT
37 NEXT I
38 REM      MEAN, STANDARD DEVIATION.
39 FOR I=1 TO M:T=0:FOR J=1 TO N
40 T=T+X(I,J)
41 NEXT J
42 X1(I)=T/N
43 NEXT I
44 FOR I=1 TO M:FOR K=1 TO M
45 S1=0
46 FOR J=1 TO N
47 S1=S1 + (X(I,J) - X1(I)) * (X(K,J) - X1(K))
48 NEXT J
49 S(I,K)=S1/(N-1)
50 S(K,I)=S(I,K)
51 NEXT K,I
52 M1=M-1
53 FOR J=1 TO M1: FOR K=1 TO M1:S2(J,K)=S(J+1,K+1): NEXT K,J
54 FOR I=1 TO M1: FOR J=1 TO M1
55 IF I < > J GOTO 400
56 S3(I,J)=1
57 GOTO 410
58 S3(I,J)=0
59 NEXT J,I
60 GOSUB 1330

```



```

430 REM      CALCULATE THE COEFFICIENTS B(I)
440 FOR I=1 TO M1
450 B(I)=0
460 FOR J=1 TO M1
470 B(I)=B(I)+S(1,J+1)*S2(J,I)
480 NEXT J,I
490 B1=0
500 FOR I=1 TO M1:B1=B1+X1(I+1)*B(I):NEXT I
510 B1=X1(1)-B1
520 REM      R2, ESTIMATIONS, SEE, F-TEST, CORRELATION
530 S3=0
540 FOR I=1 TO N: Y(I)=0
550 FOR J=2 TO M: Y(I)=Y(I)+B(J-1)*X(J,I): NEXT J
560 Y(I)=Y(I)+B1
570 R(I)=X(1,I)-Y(I)
580 S3=S3+R(I)^2
590 NEXT I
600 S(1,1)=(N-1)*S(1,1)
610 R2=(S(1,1)-S3)/S(1,1)
620 IF R2=1 GOTO 640
630 F=(R2/M1)/((1-R2)/(N-M))
640 E=SQR(S3/(N-M))
650 FOR J=1 TO M1
660 V(J)=E*SQR(S2(J,J)/(N-1))
670 NEXT J
680 S(1,1)=S(1,1)/(N-1)
690 C(1,1)=1
700 FOR J=2 TO M
710 C(J,J)=1
720 J1=J-1
730 FOR I=1 TO J1
740 C(I,J)=S(I,J)/SQR(S(I,I)*S(J,J))
750 C(J,I)=C(I,J)
760 NEXT I,J
770 FOR I=1 TO M
780 S(I,I)=SQR(S(I,I))
790 NEXT I
800 REM      OUTPUT
810 PRINT : PRINT : PRINT
820 PRINT "CORRELATION MATRIX"
830 PRINT "*****"
840 FOR I=1 TO M: FOR J=1 TO M
850 IF I<J THEN 870
860 PRINT INT(100*C(I,J))/100; TAB(10*J)
870 NEXT J: PRINT
880 NEXT I
890 PRINT : PRINT : PRINT
900 PRINT "VARIABLE      MEAN      STD DEVIATION"
910 PRINT "*****"
920 FOR I=1 TO M
923 PRINT A$(I); TAB( 14);X1(I); TAB( 27);S(I,I)
925 NEXT I
930 PRINT : PRINT
940 INPUT "CONTINUE? ";C$
950 PRINT : PRINT
960 PRINT "REGRESSION EQUATION"
970 PRINT "*****"
990 PRINT "DEPENDENT VARIABLE: ";A$(1)
1000 PRINT "*****"
1020 PRINT "INDEPNENT EST  BETA  ERRORS  T-TEST"

```

```

1030 PRINT "VARIABLE   COEF   %"
1040 PRINT "*****"
1050 PRINT
1060 FOR I=1 TO M1
1070 PRINT A$(I+1); TAB( 11); INT(100*B(I)+.5)/100; TAB( 18);
1080 PRINT INT(10000*(B(I)*(X1(I+1)/X1(1))))/100;
1090 PRINT TAB( 25); INT(100*V(I)+.5)/100;
1095 PRINT TAB( 35); INT(100*(B(I)/V(I)+.5)/100
1100 NEXT I
1110 PRINT "CONSTANT"; TAB( 11);B1
1120 PRINT "*****"
1125 DEF FN A(Z)=INT(100*Z+.5)/100
1130 REM          STATISTICS
1140 PRINT "DETERMINATION COEFFICIENT   = "; FN A(R2)
1150 PRINT "CORRELATION COEFFICIENT     = "; FN A(SQR(R2))
1160 PRINT "F-TEST                       = "; FN A(F)
1170 PRINT "DEGREES OF FREEDOM           = "; FN A(N-K)
1180 PRINT "SUM SQUARE OF ERRORS         = "; FN A(E)
1190 PRINT "*****"
1200 REM          RESIDUALS
1210 PRINT : PRINT
1220 INPUT "CONTINUE? ";C$
1230 PRINT : PRINT
1240 PRINT "TABLE OF RESIDUAL VALUES"
1250 PRINT "*****"
1260 PRINT " #  OBSERVATION  ESTIMATION  RESIDUAL  "
1270 PRINT "*****"
1280 FOR I=1 TO N
1290 PRINT " ";I; TAB( 8); INT(100*(X(1,I))+.5)/100;
1293 PRINT TAB( 18); INT(100*Y(I)+.5)/100;
1295 PRINT TAB( 30); INT(100*R(I)+.5)/100
1300 NEXT I
1310 PRINT "*****"
1320 END
1330 REM          MATRIX INVERSION
1340 FOR K=1 TO M1: FOR I=1 TO M1
1350 IF I>K THEN 1420
1360 PP=S2(K,K)
1370 FOR J=1 TO M1
1380 S2(K,J)=S2(K,J)/PP
1390 S3(K,J)=S3(K,J)/PP
1400 NEXT J
1410 IF I=K GOTO 1470
1420 PP=S2(I,K)
1430 FOR J=1 TO M1
1440 S2(I,J)=S2(I,J)-S2(K,J)*PP
1450 S3(I,J)=S3(I,J)-S3(K,J)*PP
1460 NEXT J
1470 NEXT I
1480 NEXT K
1490 FOR I=1 TO M1: FOR J=1 TO M1: S2(I,J)=S3(I,J): NEXT J,I
1500 RETURN

```

APPENDIX-4

***TRANSYT-7F SIMULATION RUNS FOR THE STUDY CASE-II,
IN CIREBON, INDONESIA***

NOTE :

- Simulation runs were established for traffic volume variation of .10S; .20S; .30S; and .40S.
- A set of output is presented for traffic volume of .30S.
- Only the system-wide performances are presented for traffic volumes of .10S; .20S; and .40S.


```

0---- PROGRAM NOTE --- A CARD TYPE 51 CAUSES JOB TO BE EXECUTED AS A SIMULATION RUN,
DELETING ANY OPTIMIZATION VALUES INPUT.
0---- PROGRAM NOTE --- NO ERRORS DETECTED. TRANSYT-7F NOW BEGINS FINAL PROCESSING.
IF ERRORS ARE DETECTED, FURTHER PROCESSING IS SUSPENDED.
0---- PROGRAM NOTE --- THERE ARE A TOTAL OF 4 NODES AND 16 LINKS (INCLUDING BOTTLENECKS, IF ANY) IN THIS RUN.
THERE WERE A TOTAL OF 3 WARNING MESSAGES ISSUED IN THE ABOVE REPORT.
1 TRANSYT-7F: RUN---[SS/CI.NTW/T7F/CH99]---SIMULATION RUN OF THE EXISTING SIGNAL FOR V/S= .30 62 SECOND CYCLE 60 STEPS
0 PERFORMANCE WITH INITIAL SETTINGS>
0 NODE LINK FLOW SAT DEGREE TOTAL TOTAL DELAY AVERAGE UNIFORM MAX BACK QUEUE FUEL PHASE LINK
NO NO (VEH/H)(VEH/H) (%) (VEH-MI/H)(VEH-H/H) (VEH-H/H) (VEH/H) (VEH/LK)(VEH/LK) (VEH/H) (VEH/LK) (VEH/LK) (GA/H) (SEC)
1 101 390 1300 124* .00 42.29 3.55 38.74 42.29 390.4 314.5( 81%) 11 > 32.71 22 101
1 103 870 2900 124* 428.15 102.74 5.44 85.22 90.65 375.1 660.2( 76%) 21 208 87.71 22 103
1 105 870 2900 56 .00 2.49 2.30 .18 2.49 10.3 519.4( 60%) 10 > 0 4.69 40 105
1 107 870 2900 .56 .00 2.49 2.30 .18 2.49 10.3 519.4( 60%) 10 > 0 4.69 40 107
0 1 : 3000 MAX = 124* 428.15 150.00 13.59 124.32 137.92 165.5 2013.5( 67%) 129.80 PI = 149.1
2 201 870 2900 124* 428.15 102.90 5.60 85.22 90.81 375.8 700.9( 81%) 18 208 88.13 22 201
2 203 870 2900 124* 428.15 104.26 6.96 85.22 92.17 381.4 699.2( 80%) 21 208 89.12 22 203
2 205 870 2900 56 922.79 28.35 2.11 .18 2.29 9.5 469.2( 54%) 9 448 40.26 40 205
2 207 390 1300 56 .00 1.19 1.01 .18 1.19 11.0 229.9( 59%) 4 > 0 2.14 40 207
0 2 : 3000 MAX = 124* 1779.09 236.70 15.67 170.80 186.47 223.8 2099.2( 70%) 219.66 PI = 198.1
3 301 870 2900 77 428.15 17.16 4.41 .66 5.07 21.0 668.2( 77%) 12 208 25.09 31 301
3 303 870 2900 77 4.64 4.64 3.98 .66 4.64 19.2 700.6( 81%) 13 > 0 7.27 31 303
3 305 870 2900 78 708.72 24.41 3.74 .66 4.40 18.2 689.6( 79%) 13 344 35.39 31 305
3 307 390 1300 78 .00 2.44 1.78 .66 2.44 22.5 314.1( 81%) 6 > 0 3.52 31 307
0 3 : 3000 MAX = 78 1136.87 48.65 13.91 2.64 16.55 19.9 2372.5( 79%) 71.27 PI = 29.7
4 401 870 2900 78 922.79 29.91 3.20 .66 3.86 16.0 616.1( 71%) 12 448 42.54 31 401
4 403 870 2900 78 .00 4.60 3.93 .66 4.60 19.0 696.4( 80%) 13 > 0 7.22 31 403
4 405 870 2900 77 4.60 3.93 .66 4.60 19.0 696.4( 80%) 13 > 0 7.22 31 405
4 407 870 2900 77 708.72 24.05 3.38 .66 4.04 16.7 610.3( 70%) 12 344 34.52 31 407
0 4 : 3480 MAX = 78 1631.51 63.16 14.44 2.65 17.09 17.7 2619.2( 75%) 91.49 PI = 31.6

```

<SYSTEM WIDE TOTALS INCLUDING ALL LINKS>

```

0 TOTAL DISTANCE TRAVELED (VEH-MI/H) 498.51 57.62 300.41 358.03 103.28 9104.4( 73%) 512.22 408.61 11.47 <TOTALS>
1 TRANSYT-7F: RUN---[SS/CI.NTW/T7F/CH99]---SIMULATION RUN OF THE EXISTING SIGNAL FOR V/S= .30 62 SECOND CYCLE 60 STEPS

```


0--- PROGRAM NOTE --- THIS IS THE INPUT DATA REPORT FOR TIME-SPACE DIAGRAM NO. 1
 0 TIME-SPACE DIAGRAM DATA

LINE NO.	CARD TYPE	NO.	NODES	TIME FLAG	TIME SCALE	DIST. SCALE
34)	60	3	0	0	200	0

LINE NO.	CARD TYPE	NO.	NODES	TIME FLAG	TIME SCALE	DIST. SCALE	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP
34)	60	3	0	0	200	0	0	0	0	0	0	0	0

0 LINE TITLE PLOT TITLE CARD

0 35) PLOT NODES : 01, 02, 03.

0 LINE CARD LINE PAIRS ALTERNATING BY DIRECTION PLOT LINK STREAM CARD

LINE NO.	CARD TYPE	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP
36)	61	103	101	203	201	303	301	0	0

1 TRANSYT-77: RUN---[SS/CI.MTW/T7F/CE99]---SIMULATION RUN OF THE EXISTING SIGNAL FOR V/S= .30 62 SECOND CYCLE 60 STEPS 0

0---- PROGRAM NOTE --- A CARD TYPE 51 CAUSES JOB TO BE EXECUTED AS A SIMULATION RUN,
 DELETING ANY OPTIMIZATION VALUES INPUT.
 0---- PROGRAM NOTE --- NO ERRORS DETECTED. TRANSYT-7F NOW BEGINS FINAL PROCESSING.
 IF ERRORS ARE DETECTED, FURTHER PROCESSING IS SUSPENDED.
 0---- PROGRAM NOTE --- THERE ARE A TOTAL OF 4 NODES AND 16 LINKS (INCLUDING BOTTLENECKS, IF ANY) IN THIS RUN.
 0---- PROGRAM NOTE --- THERE WERE A TOTAL OF 3 WARNING MESSAGES ISSUED IN THE ABOVE REPORT.
 1TRANSYT-7F: RUN---[SS/CI.NTW/T7F/CH99]---SIMULATION RUN OF THE EXISTING SIGNAL FOR V/S= .10 62 SECOND CYCLE 60 STEPS
 0PERFORMANCE WITH INITIAL SETTINGS>
 0 NODE LINK FLOW SAT DEGREE TOTAL TOTAL AVERAGE UNIFORM MAX BACK QURUB FUEL PHASE LINK
 NO NO (VEH/H)(VEH/H) (%) (VEH-MI/H)(VEH-H/H) (VEH-H/H) (VEH-H/H) (SEC/VEH) (VEH/H;%) (VEH/LK)(VEH/LK) (GA/H) (SEC) (GA/H) (SEC) NO

1	101	130	1300	41	.00	.77	.70	.07	.77	21.3	101.2(78%)	2	0	1.12	22	101
1	103	290	2900	41	142.72	5.48	1.38	.07	1.45	18.1	176.9(61%)	3	208	7.84	22	103
1	105	290	2900	19	.00	.58	.57	.01	.58	7.2	131.4(45%)	3	0	1.15	40	105
1	107	290	2900	19	.00	.58	.57	.01	.58	7.2	131.4(45%)	3	0	1.15	40	107
0	1	1000	MAX = 41	142.72	7.41	3.22	.17	.17	3.38	12.2	540.8(54%)			11.26	PI = 6.4	
2	201	290	2900	41	142.72	5.72	1.61	.07	1.69	20.9	242.5(84%)	4	208	8.51	22	201
2	203	290	2900	41	142.72	5.52	1.42	.07	1.49	18.6	193.8(67%)	4	208	8.00	22	203
2	205	290	2900	19	307.60	9.26	.56	.01	.57	7.1	122.3(42%)	2	448	13.02	40	205
2	207	130	1300	19	.00	.27	.26	.01	.27	7.5	59.8(46%)	1	0	.53	40	207
0	2	1000	MAX = 41	593.03	20.77	3.86	.17	.17	4.03	14.5	618.4(62%)			30.06	PI = 7.5	
3	301	290	2900	26	142.72	5.58	1.53	.02	1.55	19.3	226.6(78%)	4	208	8.29	31	301
3	303	290	2900	26	.00	1.05	1.03	.02	1.05	13.1	181.9(63%)	3	0	1.78	31	303
3	305	290	2900	26	236.24	7.65	.96	.02	.98	12.2	181.8(63%)	3	344	11.07	31	305
3	307	130	1300	26	.00	.48	.46	.02	.48	13.4	81.5(63%)	1	0	.81	31	307
0	3	1000	MAX = 26	378.96	14.77	3.98	.09	.09	4.07	14.7	671.8(67%)			21.94	PI = 7.8	
4	401	290	2900	26	307.60	9.66	.95	.02	.98	12.1	178.8(62%)	3	448	13.75	31	401
4	403	290	2900	26	.00	1.04	1.02	.02	1.04	12.9	181.3(63%)	3	0	1.77	31	403
4	405	290	2900	26	.00	1.04	1.02	.02	1.04	12.9	181.3(63%)	3	0	1.77	31	405
4	407	290	2900	26	236.24	7.64	.95	.02	.97	12.0	161.6(56%)	3	344	10.91	31	407
0	4	1160	MAX = 26	543.84	19.38	3.94	.09	.09	4.03	12.5	702.8(61%)			28.19	PI = 7.9	

<SYSTEM WIDE TOTALS INCLUDING ALL LINKS>

0	TOTAL DISTANCE TRAVELED (VEH-MI/H)	TOTAL TRAVEL TIME (VEH-H/H)	TOTAL UNIFORM DELAY (VEH-H/H)	TOTAL RANDOM DELAY (VEH-H/H)	TOTAL DELAY (VEH-H/H)	AVERAGE DELAY (SEC/VEH)	TOTAL UNIFORM STOPS (VEH/H-H)	TOTAL FUEL CONSUM (GA/H)	PERFORMANCE INDEX	SPEED (MI/H)
0	1658.54	62.34	14.99	15.51	13.42	2533.8(61%)	91.44	29.59	29.35	<TOTALS>
1	TRANSYT-7F: RUN	---	[SS/CI.NTW/T7F/CH99]	---	SIMULATION RUN OF THE EXISTING SIGNAL FOR V/S= .10	62 SECOND CYCLE	60 STEPS			

APPENDIX-5

***TRANSYT-7F OPTIMIZATION RUNS FOR THE STUDY CASE-II,
IN CIREBON, INDONESIA***

NOTE :

- Optimization runs were established for traffic volume variation of .10S; .20S; .30S; and .40S.
- A set of output is presented for traffic volume of .30S.
- Only the system-wide performances are presented for traffic volumes of .10S; .20S; and .40S.

0--- PROGRAM NOTE --- A CARD TYPE 52 CAUSES RUN TO BE OPTIMIZED USING THE DEFAULT NORMAL OPTIMIZATION STEP SIZES.
 IF CARD TYPE 4 WAS INPUT, IT IS IGNORED.
 0--- PROGRAM NOTE --- THE ABOVE WILL BE PROCESSED AFTER THE "BEST" CYCLE LENGTH HAS BEEN SELECTED.
 NO ERRORS DETECTED. TRANSYT-7F NOW BEGINS FINAL PROCESSING.
 0--- PROGRAM NOTE --- IF ERRORS ARE DETECTED, FURTHER PROCESSING IS SUSPENDED.
 IF ERRORS ARE DETECTED, FURTHER PROCESSING IS SUSPENDED.
 0--- PROGRAM NOTE --- THERE ARE A TOTAL OF 4 NODES AND 16 LINKS (INCLUDING BOTTLENECKS, IF ANY) IN THIS RUN.
 IRUN TITLE: RUN--[SS/CI.NTW/T7F/CH99]---OPTIMIZATION RUN OF THE SIGNAL TIMING FOR V/S= .30

FIELDS: 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16

0--- PROGRAM NOTE --- THERE WERE A TOTAL OF 1 WARNING MESSAGES ISSUED IN THE ABOVE REPORT.
 IRUN TITLE: RUN--[SS/CI.NTW/T7F/CH99]---OPTIMIZATION RUN OF THE SIGNAL TIMING FOR V/S= .30

0

 CYCLE EVALUATION SUMMARY PERFORMANCE

CYCLE LENGTH (SEC)	STEP SIZE (STEPS)	AVERAGE DELAY (SEC/VEH)	PERCENT STOPS (%)	FUEL CONSUMPTION (GAL/HR)	PERFORMANCE INDEX	NUMBER SATURATED LINKS
0	38	19.93	77	304.1	122.8	6
0	40	16.54	77	295.1	110.7	0
0	42	16.51	78	296.1	111.5	0
0	44	15.76	77	292.9	107.8	0
0	46	16.07	78	295.2	110.0	0
0	48	15.71	77	293.1	107.8	0
0	50	16.18	77	294.1	109.5	0
0	52	15.99	76	292.8	108.1	0
0	54	16.32	76	293.5	109.2	0
0	56	16.32	76	293.5	109.1	0
0	58	16.72	76	294.7	110.7	0
0	60	16.48	75	293.2	109.1	0
0	62	17.22	74	294.6	111.3	0
0	64	17.43	74	295.0	111.9	0
0	66	17.60	74	295.1	112.2	0
0	68	17.85	74	295.7	113.0	0

0-----TRANSYT-7F SIGNAL CONTROLLER SETTINGS-----

0-----NETWORK-WIDE SIGNAL TIMING DATA-----

OSYSTEM CYCLE LENGTH = 48 SECONDS
OMASTER OFFSET REFERENCE LOCATION = INTERSECTION NO. 1
OALL OFFSETS ARE REFERENCED TO THE START OF INTERVAL NO. 1 AT THIS SIGNAL.

-----INTERSECTION CONTROLLER SETTINGS-----

-----INTERSECTION 1-----

OINTERVAL NUMBER : 1 2 3 4
OINTVL LENGTH(SEC): 19 5 19 5
OINTVL LENGTH (%): 40 10 40 10
OPIN SETTINGS (%): 100/0 40 50 90
OPHASE START (PH#): 1 2
OINTERVAL TYPE : V Y V Y
OLINKS MOVING : 101 105
103 107

OOFFSET = 0 SEC. 0 %.
OTHIS IS THE MASTER CONTROLLER.

0+++ 193 +++ WARNING +

+ TRANSYT-7F: RUN----[SS/CI.NTW/T7F/CH99]---OPTIMIZATION RUN OF THE SIGNAL TIMING FOR V/S= .30 48 SECOND CYCLE 24 STEPS
THE OFFSET FALLS WITHIN 1% OF AN INTERVAL CHANGE POINT AT THE START OF INTERVAL NO 1.

-----INTERSECTION 2-----

OINTERVAL NUMBER : 1 2 3 4
OINTVL LENGTH(SEC): 19 5 19 5
OINTVL LENGTH (%): 40 10 40 10
OPIN SETTINGS (%): 100/0 40 50 90
OPHASE START (PH#): 1 2
OINTERVAL TYPE : V Y V Y
OLINKS MOVING : 201 205
203 207

OOFFSET = 12 SEC. 25 %.

INTERSECTION 3

OINTERVAL NUMBER : 1 2 3 4
 OINTVL LENGTH(SEC): 19 5 19 5
 OINTVL LENGTH (*): 40 10 40 10
 OPIN SETTINGS (*): 100/0 40 50 90
 OPHASE START (PH#): 1 2
 OINTERVAL TYPE : V Y V Y
 OLINKS MOVING : 301 305
 303 307

OOFFSET = 6 SEC. 12 %
 ITRANSYT-7F: RUN---[SS/CI.NTW/T7F/CH99]---OPTIMIZATION RUN OF THE SIGNAL TIMING FOR V/S= .30 48 SECOND CYCLE 24 STEPS

INTERSECTION 4

OINTERVAL NUMBER : 1 2 3 4
 OINTVL LENGTH(SEC): 19 5 19 5
 OINTVL LENGTH (*): 40 10 40 10
 OPIN SETTINGS (*): 100/0 40 50 90
 OPHASE START (PH#): 1 2
 OINTERVAL TYPE : V Y V Y
 OLINKS MOVING : 401 405
 403 407

OOFFSET = 44 SEC. 92 %
 ITRANSYT-7F: RUN---[SS/CI.NTW/T7F/CH99]---OPTIMIZATION RUN OF THE SIGNAL TIMING FOR V/S= .30 48 SECOND CYCLE 24 STEPS
 OLINE CARD
 NO. TYPE
 TERMINATION CARD

0---- PROGRAM NOTE ---- THIS IS THE INPUT DATA REPORT FOR TIME-SPACE DIAGRAM NO. 1

LINE NO.	CARD TYPE	NO. NODES	TIME FLAG	TIME SCALE	DIST. SCALE	TIME-SPACE DIAGRAM DATA									
34)	60	3	0	0	200	0	0	0	0	0	0	0	0	0	0
0 LINE NO.	TITLE					PLOT TITLE CARD									
0 35)	PLOT NODES : 01, 02, 03.					PLOT LINK STREAM CARD									
LINE NO.	CARD TYPE	LINK PAIRS ALTERNATING BY DIRECTION			PLOT LINK STREAM CARD										
		DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP	DOWN AND UP		
36)	61	103	101	203	201	303	301	0	0	0	0	0	0	0	0

1TRANSYT-7F: RUN---[SS/CI.NTW/T7F/CH98]---OPTIMIZATION RUN OF THE EXISTING SIGNAL FOR V/S= .3 48 SECOND CYCLE / 24 STEPS

BEST CYCLE LENGTH = 36 SEC. CYCLE SENSITIVITY = 2.2 %
 0--- PROGRAM NOTE --- TRANSYT-7F OPTIMIZES THE SYSTEM USING THE BEST CYCLE LENGTH AND HILL-CLIMB STEP SIZES AS INDICATED BY CARD TYPE 52.

1 TRANSYT-7F: RUN---[SS/CI.NTW/T7F/CH99]---OPTIMIZATION RUN OF THE EXISTING SIGNAL FOR V/S=.10 36 SECOND CYCLE 18 STEPS																		
0<PERFORMANCE WITH OPTIMAL SETTINGS>																		
0	NO	LINK NO	FLOW	SAT FLOW	DEGREE	TOTAL TRAVEL	TIME	UNIFORM	RANDOM	DELAY	TOTAL	AVERAGE	UNIFORM	MAX BACK	QUEUE	FUEL	PHASE	LINK
			(VEH/H)	(VEH/H)	(%)	(VEH-MI/H)	(VEH-H/H)	(VEH-H/H)	(VEH-H/H)	(SEC/VEH)	(SEC/VEH)	(VEH/H-%)	(VEH/LK)	(VEH/LK)	(VEH/LK)	(GA/H)	LENGTH	NO
1	101	130	1300	36		.00	.40	.35	.05	.40	11.0	87.3(67%)	1	>	0	.77	16	101
1	103	290	2900	36		142.72	4.81	.73	.05	.78	9.7	195.3(67%)	2	>	208	7.49	16	103
1	105	290	2900	26		.00	.57	.55	.02	.57	7.1	162.6(56%)	2	>	0	1.31	20	105
1	107	290	2900	26		.00	.57	.55	.02	.57	7.1	162.6(56%)	2	>	0	1.31	20	107
0	1	1000	MAX =	36		142.72	6.35	2.17	.15	2.32	8.3	607.7(61%)				10.89	PI =	5.7
2	201	290	2900	26		142.72	4.55	.50	.02	.52	6.5	149.3(51%)	2	>	208	6.94	20	201
2	203	290	2900	26		142.72	4.59	.54	.02	.56	6.9	156.8(54%)	2	>	208	7.03	20	203
2	205	290	2900	36		307.60	9.49	.76	.05	.81	10.0	194.7(67%)	2	>	448	13.75	16	205
2	207	130	1300	36		.00	.40	.35	.05	.40	11.0	87.3(67%)	1	>	0	.77	16	207
0	2	1000	MAX =	36		593.03	19.03	2.14	.15	2.28	8.2	588.2(59%)				28.49	PI =	5.6
3	301	290	2900	26		142.72	4.55	.50	.02	.52	6.5	146.0(50%)	2	>	208	6.92	20	301
3	303	290	2900	26		.00	.57	.55	.02	.57	7.1	162.6(56%)	2	>	0	1.31	20	303
3	305	290	2900	36		236.24	7.46	.74	.05	.79	9.8	194.5(67%)	2	>	344	11.03	16	305
3	307	130	1300	36		.00	.40	.35	.05	.40	11.0	87.3(67%)	1	>	0	.77	16	307
0	3	1000	MAX =	36		378.96	12.98	2.13	.15	2.28	8.2	590.4(59%)				20.03	PI =	5.6
4	401	290	2900	30		307.60	9.36	.64	.03	.67	8.4	169.7(59%)	2	>	448	13.46	18	401
4	403	290	2900	30		.00	.69	.65	.03	.69	8.5	178.7(62%)	2	>	0	1.49	18	403
4	405	290	2900	30		.00	.69	.65	.03	.69	8.5	178.7(62%)	2	>	0	1.49	18	405
4	407	290	2900	30		236.24	7.35	.65	.03	.68	8.5	178.5(62%)	2	>	344	10.83	18	407
0	4	1160	MAX =	30		543.84	18.08	2.60	.13	2.73	8.5	705.6(61%)				27.26	PI =	6.6

1 TRANSYT-7F: RUN---[SS/CI.NTW/T7F/CH99]---OPTIMIZATION RUN OF THE EXISTING SIGNAL FOR V/S=.10 36 SECOND CYCLE 18 STEPS

<SYSTEM WIDE TOTALS INCLUDING ALL LINKS>

0	TOTAL	TOTAL	TOTAL	TOTAL	AVERAGE	TOTAL	TOTAL	PERFORMANCE	SPEED
	DISTANCE	TRAVEL	UNIFORM	RANDOM	DELAY	UNIFORM	FUEL	INDEX	(MI/H)
	TRAVELED	TIME	DELAY	DELAY	(SEC/VEH)	STOPS	CONSUM		
	(VEH-MI/H)	(VEH-H/H)	(VEH-H/H)	(VEH-H/H)	(VEH-H/H)	(VEH/H-%)	(GA/H)		
0	1658.54	56.43	9.04	.57	9.61	8.31	2491.8(60%)	23.45	31.79 <TOTALS>
1	TRANSYT-7F: RUN---[SS/CI.NTW/T7F/CH99]---OPTIMIZATION RUN OF THE EXISTING SIGNAL FOR V/S=.10 36 SECOND CYCLE 18 STEPS						86.67		

1TRANST-7F: RUN---[SS/CI.NTW/TTF/CH99]---OPTIMIZATION RUN OF THE SIGNAL TIMING FOR V/S= .40 88 SECOND CYCLE 44 STEPS
 0<PERFORMANCE WITH OPTIMAL SETTINGS>

0	MODE LINK NO	FLOW (VEH/H)	SAT FLOW (VEH/H)	DEGREE (%)	TOTAL TRAVEL (VEH-MI/H)	TOTAL TIME (VEH-H/H)	UNIFORM DELAY (VEH-H/H)	RANDOM DELAY (VEH-H/H)	TOTAL DELAY (SEC/VEH)	AVERAGE DELAY (VEH/H-%)	UNIFORM STOPS (VEH/H-%)	MAX OF QUEUE CAPACITY (VEH/LK)	QUEUE (VEH/LK)	FUEL CONSUM (GA/H)	PHASE LENGTH (SEC)	LINK NO
1	101	520	1300	93	.00	5.82	3.28	2.54	5.82	40.3	454.6(87%)	12	0	6.78	44	101
1	103	1160	2900	93	570.87	24.50	5.66	2.72	8.39	26.0	922.6(80%)	25	208	34.88	44	103
1	105	1160	2900	93	.00	10.05	7.32	2.72	10.05	31.2	1014.1(87%)	26	>	12.96	44	105
1	107	1160	2900	93	.00	10.05	7.32	2.72	10.05	31.2	1014.1(87%)	26	>	12.96	44	107
0	1 :	4000	MAX = 93		570.87	50.42	23.59	10.71	34.30	30.9	3405.5(85%)			67.59	PI = 53.2	
2	201	1160	2900	93	570.87	26.74	7.90	2.72	10.62	33.0	1060.0(91%)	27	208	37.58	44	201
2	203	1160	2900	93	570.87	24.59	5.74	2.72	8.47	26.3	957.1(83%)	26	208	35.21	44	203
2	205	1160	2900	93	1230.39	45.24	7.77	2.72	10.50	32.6	997.5(86%)	25	418	62.00	44	205
2	207	520	1300	93	.00	5.82	3.28	2.54	5.82	40.3	454.6(87%)	12	0	6.78	44	207
0	2 :	4000	MAX = 93		2372.12	102.39	24.70	10.71	35.41	31.9	3469.2(87%)			141.57	PI = 54.7	
3	301	1160	2900	93	570.87	24.45	5.61	2.72	8.33	25.9	882.9(76%)	24	208	34.54	44	301
3	303	1160	2900	93	.00	10.05	7.32	2.72	10.05	31.2	1014.1(87%)	26	>	12.96	44	303
3	305	1160	2900	93	944.96	36.03	6.63	2.72	9.35	29.0	963.4(83%)	25	344	50.08	44	305
3	307	520	1300	93	.00	5.82	3.28	2.54	5.82	40.3	454.6(87%)	12	0	6.78	44	307
0	3 :	4000	MAX = 93		1515.82	76.35	22.84	10.71	33.56	30.2	3315.0(83%)			104.36	PI = 52.0	
4	401	1160	2900	93	1230.39	44.04	6.58	2.72	9.30	28.9	1015.7(88%)	26	448	61.26	44	401
4	403	1160	2900	93	.00	10.05	7.32	2.72	10.05	31.2	1014.1(87%)	26	>	12.96	44	403
4	405	1160	2900	93	.00	10.05	7.32	2.72	10.05	31.2	1014.1(87%)	26	>	12.96	44	405
4	407	1160	2900	93	944.96	36.13	6.73	2.72	9.45	29.3	945.4(81%)	25	344	50.01	44	407
0	4 :	4640	MAX = 93		2175.35	100.26	27.95	10.89	38.85	30.1	3989.4(86%)			137.20	PI = 61.0	

1TRANST-7F: RUN---[SS/CI.NTW/TTF/CH99]---OPTIMIZATION RUN OF THE SIGNAL TIMING FOR V/S= .40 88 SECOND CYCLE 44 STEPS

<SYSTEM WIDE TOTALS INCLUDING ALL LINKS>

0	TOTAL DISTANCE TRAVELED (VEH-MI/H)	TOTAL TRAVEL TIME (VEH-H/H)	TOTAL UNIFORM DELAY (VEH-H/H)	TOTAL RANDOM DELAY (VEH-H/H)	TOTAL DELAY (VEH-H/H)	AVERAGE DELAY (SEC/VEH)	TOTAL UNIFORM STOPS (VEH/H-%)	TOTAL FUEL CONSUM (GA/H)	PERFORMANCE INDEX	SPEED (MI/H)	TOTALS>
0	6634.16	329.43	99.09	43.03	142.12	30.75	14179.1(85%)	450.72	220.89	25.35	<TOTALS>

REFERENCES

REFERENCES

1. **'A Manual on User Benefit Analysis of Highway and Bus Transit Improvements'**, American Association of State Highway and Transportation Officials, Washington D.C. 20001, 1977.
2. Courage, Kenneth., **'Microcomputer Applications in Traffic Engineering'**, Transportation Research Record 932, pp. 13 - 16, Transportation Research Board, Washington D.C. 20418, 1983.
3. Deakin, Elizabeth E., et al., **'Assessing the Traffic Impacts of Transportation and Land Development Scenarios'**, Transportation Quarterly, vol. 39, No. 4, October 1985, pp. 605 - 626.
4. Dudeck, Gerald R., et al., **'TRANSYT-7F and NETSIM Comparison of Estimated and Simulated Performance Data'**, ITE Journal, August 1983, pp. 32 - 34.
5. Gibson, David., **'Available Computer Models for Traffic Operation Analysis'**, Transportation Research Board, Special Report 194, Washington D.C. 20418, 1981, pp. 12-22.
6. Homburger, Wolfgang S., et al., **'Fundamentals of Traffic-Engineering'**, [11th edition], Institute of Transportation Study, University of California Berkeley, California 94720, 1984.
7. Homburger, Wolfgang S., et al., **'Transportation and Traffic Engineering Handbook'**, [2nd edition], Institute of Transportation Engineers, Prentice-Hall, Inc., Englewood Cliff, New Jersey 07632, 1982.
8. Kell, James H., et al., **'Manual of Traffic Signal Design'**, Institute of Transportation Engineers, Prentice-Hall, Inc., Englewood Cliffs, New Jersey 07632, 1982.
9. May, Adolf D. Jr., et al., **'Developments in Traffic Signal Systems'**, Transportation Research Circular, Transportation Research Board, Washington D.C. 20418, 1984.

10. **'National Signal Timing Optimization Projects'**, Federal Highway Administration, Office of Traffic Operations, Washington D.C. 20590, 1982.
11. Nemeth, Zoltan A., et al., **'NETSIM and SOAP a Test of Compatibility'**, ITE Journal, October 1982, pp. 38 - 41.
12. Paquette, R., et al., **'Transportation Engineering'**, John Wiley and Sons, Inc., New York, 1982.
13. Pignataro, L. J., **'Traffic Engineering Theory and Practice'**, Prentice-Hall, Inc., Englewood Cliffs, New Jersey 07632, 1973.
14. Powel, James L., **'Network Evaluation using TRANSYT'**, ITE Journal, July 1982, pp. 13 - 17.
15. **'Signal Operations Analysis Package - Executive Summary'**, University of Florida, Transportation Research Center, FHWA Implementation Package 78-4, January 1978.
16. **'Signal Operations Analysis Package - 1., Computational Methodology'**, University of Florida, Transportation Research Center, FHWA Implementation Package 78-4, January 1978.
17. **'Signal Operations Analysis Package - 2., User's Manual'**, University of Florida, Transportation Research Center, FHWA Implementation Package 78-4, January 1978.
18. Tarnoff, P. J., et al., **'Selecting Traffic Signal Control at Individual Intersections'**, NCHRP Report 233, June 1981.
19. **'Traffic Control Devices and Traffic Signal Systems'**, Transportation Research Record 881, Transportation Research Board, Washington D.C. 20418, 1982.
20. **'Traffic Engineering Practices in Developing Countries, an Informational Report'**, ITE Technical Council Committee 4A-20, Institute of Transportation Engineers, Washington D.C. 20024-2729, 1985.

21. **'Traffic Signal Modernization'**, Institute of Transportation Engineers, Technical Notes, April 1981.
 22. Transportation Research Board, **'Highway Capacity Manual'**, TRB Special Report 209, Washington D.C. 20418, 1985.
 23. Transportation Research Board, **'The Applications of Traffic Simulation Models'**, TRB Special Report 194, Washington D.C. 20418, 1981.
 24. U.S. Department of Transportation, Federal Highway Administration, Office of Research and Development, **'Handbook of Computer Models for Traffic Operations Analysis'**, Technology Sharing Report, FHWA-TS-82-213, Washington D.C. 20590, December 1982.
 25. U.S. Department of Transportation, Federal Highway Administration, **'Manual on Uniform Traffic Control Devices for Streets and Highways'**, U.S. Government Printing Office, Washington D.C. 20402, 1978.
 26. U.S. Department of Transportation, Federal Highway Administrations, **'SOAP84 - Data Input Manager'**, Implementation package, FHWA-IP-85-8, Washington D.C. 20590, Jan. 1985.
 27. U.S. Department of Transportation, Federal Highway Administration, **'SOAP84 - User's Manual'**, Implementation Package, FHWA-IP-85-7, Washington D.C. 20590, January 1985.
 28. Wallace, Charles E., **'At Last A TRANSYT Model Designed for American Traffic Engineers'**, ITE Journal, August 1983, pp. 28 - 31.
 29. Wallace, Charles E., et al., **'TRANSYT-7F User's Manual'**, University of Florida, Transportation Research Center, Prepared for the Federal Highway Administration, Office of Traffic Operation, Washington D.C., 1983 (Revised).
-

MICHIGAN STATE UNIV. LIBRARIES



31293007968740