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AN INVESTIGATION OF THE LANSING
WATER SUPPLY

Thesis for the Degree of B. S.

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1911

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The Investigation of the Lansing Water Supply.

Introduction.

The investigation of the Lansing City Water Supply was undertaken as our thesis with the idea of showing just what the prevailing conditions of the water system are, with the view of determining its adequacy for a growing city.

It is proposed to criticise favorably or otherwise, and to recommend improvements. We base our criticisms and recommendations on references taken from treatises on good water supply methods as presented by recognized authorities. We will not attempt to mention our authority on conclusions reached for each part of the system, but prefer to summarise here.

Our authorities are: "Public Water Supply" by Turneaure and Bubbel; "Water Work's Management and Maintenance" by Hubbard and Kiersted; "Treatise on Hydraulics" by Merriman; "Water Supply Engineering" by Folwell; Water Supply Papers, No. 172, U.S. Geological Survey; Geological Survey Reports; and "Chemical Analysis of Water" by Mason.

We are much indebted to Professors Vedder and Rosing for their suggestions, also to several city officials and state officials for data and information, some of whom are Mr. C. D. Dodge, superintendent of Water Board, Mr. W. B. Kirby, Secretary of Water and Light Board, and Mr. Hugo R. Delfs, Chief of Fire Dept.

and also to Dr.M.L.Helm, State Bacteriologicalist; Mr. Allen, State Geologist; Prof. Barrows, professor of geology at M.A.C. and also to the engineers at the various stations for their courtesy and help.

Source.

The water supply of the City of Lansing is a strictly ground water supply, as the city obtains its entire supply from deep bored wells ranging from 230 to 300 feet in depth. These wells are located in four different parts of the city, and while they are not of the same depth, the supply is derived from the same strata of sandstone. This is shown by the records of well borings and water analysis very conclusively.

Lansing is situated in the Coal Basin of Southern Michigan. This basin extends over a considerable portion of the southern peninsula, and has the general shape of a saucer. It is composed of a carboniferous limestone formation, followed by a complex sandstone and shale with intermediate seams of coal. From this coal measure is drawn the water for all deep driven wells of this vicinity.

The true source of the water is indefinite, but it is plain to be seen that it comes from some remote catchment area where the upper impervious strata cutrops allowing the percolating surface water to reach the coal measure.

As the knowledge of source is necessary for an intelligent analysis of water, it is known that Lansing's supply is entirely free from surface waters of the immediate vicinity by the fact that the boring of the wells has never affected the water in the shallow dug wells that are in the neighborhood.

hood, also the absence of tubercles in the pipes, which result from organisms which are found only in the surface waters. The temperature of the water is nearly constant, showing it to be a ground supply.

Very little trouble has been experienced by contamination in this system, as the wells which are bored, are protected by a six inch steel casing, which was driven, in all cases, as far as possible into the bed rock. By this method all surface waters are eliminated.

The only trouble in the way of epidemics, experienced in the system, occurred in the winter of 1908, when a serious outbreak of winter cholera was traced to the Townsend St. substation wells. Here it was found that the steel casing of one of the wells leaked, and this well was within a few feet of a trunk sewer. This was remedied by sinking a four inch pipe inside of the six inch one. After this no more trouble was experienced.

This experience, though, serves to make plain the necessity of the utmost precaution in the selection of the sites of wells, and the operation of boring and tubing.

The location of the wells is well adapted to the topographical conditions of the city. As the city is practically level, and with no chance of using a gravity system, the main station was located at a central point, and as the city grew, it became obviously true that substations would have to be placed in different parts of the city to increase the pressure, especially as the system was not designed and laid out to fulfill the present conditions.

Exhaustibility.

The exhaustibility of the Lansing water supply has never been truly tested. It is certain from the information to be obtained from the city officials that it merely a question of a short time when the city will have to take steps toward finding a new source, or to adding to the source now being used.

This is shown by the fact that the main pumping station could take care of the city's consumption, if it could be run at its full capacity, but the wells at the main station have by their actions, shown that they are being drawn from at their maximum capacity. We are positive that the sinking of new wells in connection with those already in operation at the main station would do no good, as ground water is governed by a few well known factors-

- 1.- No more water can be continuously taken out of the ground than goes into it.
- 2.- The yield of the ground water is dependent upon the character and extent of the catchment area, and the depth of the saturated water bearing material.
- 3.- The velocity of flow of ground water depends upon the character of the material through which it must pass in gravitating from a higher to a lower level.

The extent of the catchment area of these deep wells is difficult to determine, as it involves a study of the geological conditions, which we are not able to make in the time allotted. We are reasonable certain, though, that its boundaries are not in the immediate vicinity of Lansing.

The first well built for the system was a dug well merely penetrating the gravel. The well was 21 feet in diameter and 28 feet deep. It was built under the direction of Mr. Coates, a hydraulic engineer. The water obtained was of good quality, and the supply was reported to be inexhaustable, but in less than ten years it was entirely gone owing, no doubt to excavations into the bed at various places, which served as taps to draw off the water.

The deep wells were then bored, and while they will always no doubt, yield water of the same standard, they will become less efficient as new wells are sunk into the same strata in and about the city.

The seasons will have no effect upon the yield of the wells, except that the demands made upon the system by the consumer may make a shortage in the supply.

Plant.

The plant of the Lansing City Water Works consists of one main and three substations, which are located in different parts of the outskirts of the city.

The main station, which was constructed at the beginning of the present system, is located in connection with the City Lighting Plant on Cedar St., near Michigan Ave., and just about 100 yards from the bank of the Grand River. It is equipped with a 18 x 56 tandem Holly Pump, and has a capacity of 5,000,000. gallons per day. This pump was installed in 1893 and has been in almost continuous service since. It is however in good shape, and handles the exacting demands upon it in good fashion. In connection with this Holly pump are two 2,500,000 gallon Worthington Pumps, which are used as auxillaries

in case of brake down.

The main station pumps from 27 deep wells which are bored in a close vicinity to the station. The main station has been in operation since 1885 and owing to its central location will no doubt continue to be the site of the water works system.

The first one of the substations was established on Townsend St., at the intersection of Isaac St., and about 200 yards from the bank of the Grand River. At present this station is not in use, the services of the other stations being adequate. There are eleven deep wells at this station ranging from 215 to 245 feet deep.

The second station was built in North Lansing on the bank of the Grand River, near the Seymour St. bridge. This station is equipped with a 10 x 12 vertical triplex pump manufactured by the Gould Mfg. Co. It is run at a constant speed of 35 strokes per minute by a 75 horse power Allis Chambers Motor.

This station is operated 16 hours per day, thereby keeping the northend pressure up during the heaviest consumption hours of the day. At this station there are four six inch wells ranging from 335 to 360 feet in depth. The pump, when working at full capacity, operates on the wells with a vacuum of 23 inches under standard conditions, however a vacuum of 18.5 inches is generally maintained.

The third substation is located on Pennsylvania Ave., at the Grand Trunk R.R. crossing. It is equipped with a Fairbanks and Marsh 14 x 12 reciprocating pump with a capacity of 1 million gallons per day. This station is the newest

of the substations and is run intermittently to save the wells at the main station.

The pumps at the substations are motor driven. The current is supplied from the City Lighting Station, which is run in connection with the main water station.

All three of the substations are small neatly furnished brick structures. The pumping machinery is embedded on concrete at about 13 feet below the surface of the ground so as to make the lift as small as practicable. The stations are in charge of two men, one for night and one for day work, to operate the pumps upon orders from the main station.

Systems.

The water system of Lansing is what is known as the direct stand pipe system. The water is pumped from all the stations directly into the system from their various locations. On Cedar St. at the main station is located a steel standpipe, which is used as an equalizer for the system. The standpipe is 100 feet high and is 18 feet in diameter. Leading away from the station, to the standpipe, and in the other direction across the Grand River to Washington Ave. are 16 inch mains. These are the largest mains in the system, and have no auxiliaries in case of a break down. At the standpipe at Cedar St. and at the intersection of Washington and Allegan St. the large mains branch out into the system, distributing water to the various parts of the system. A condition showing that the system was installed when the city was smaller and less flourishing is shown by the fact that no regular lateral system, distributing water to the various portions of the city, is used.

but in such case, as can be seen by the map of the system accompanying this thesis, there are two mains in a single street or again the large mains drop to small ones. This can obviously be seen to be contrary to good practice as the accompanying loss of head would be considerable, and would largely depreciate the pressure and hence the efficiency of the mains.

The average size of the mains throughout the system is six inches through, though in short side streets these drop to four inches.

Along with the size of the mains comes another criticism of the system, that is in the number of dead ends, which can be seen in the accompanying map. Dead ends in mains are not only a violation of good practice, but are detrimental to the system and may become a positive menace to the consumers. As water soon becomes stagnant where there is no circulation, bacteria increase rapidly.

Consumption.

The quantity of water furnished to a city from its supply system is a most important item. Without an ample supply of water in an inland city, the city will suffer from a lack of proper sanitary practices.

The consumption of a city is figured per capita per day. In good American practice the amount should be from 75 to 100 gallons. In order to determine the consumption of Lansing, considerable trouble was encountered, because of the lack of apparatus to measure the true amount of water pumped. This comes about from the fact that the amount of water pumped at the two substations is not measured in any way, the pumping being done intermittently merely to remove the drain upon



the wells at the main station.

At the main station is a counter by which the number of strokes are recorded. This multiplied by the displacement of the pump, with a consideration for climate and load taken, gives a fair estimate of the amount of water pumped.

To find the amount pumped by the stations a careful record of the number of pumps and the time they were in operation is kept by the man in charge, and as the speed of these pumps are nearly constant, an estimate could be made by computing the displacement of them. These results are not absolutely correct, though, we feel they are as close an estimate of the amount pumped as ever has been taken.

Owing to the lack of time, the test made by us was for eight successive days, and the weather during this was fair spring weather which will give a fair average and not a maximum as desired.

The record from the main station was as follows:

	ST. NO. 3	ST. NO. 4	Total	Pumped.
April 21	10 000	0000	0 000	1 670 000
" 22	7 000	0000	13 000	11 601 50
" 23	8 400	0570	17 800	1 431 10
" 24	6 000	0000	17 100	1 710 700
" 25	8 410	0000	17 600	1 350 000
" 26	10 000	0000	10 000	1 500 000
" 27	8 400	0000	10 800	1 412 60
" 28	10 000	0400	10 400	1 612 000
				11 511 500 Total.

Data on pump at Klein Station.

Diameter of cylinder ----- 12 inches.

Length of stroke----- 10 "

Slip----- 5 %

Strokes recorded on gauge.

Displacement of pump,

$$\frac{3.1416 \times 18 \times 18 \times 36 \times 6}{4 \times 171} = 76.2 \text{ gallons per stroke.}$$

$$76.2 \times .05 = 3.81 \text{ gallons lost by slip.}$$

$$76.2 - 3.81 = 72.39 \text{ gallons per stroke.}$$

Record from Pennsylvania Substation.

Date.	Hours Run.	Amount Pumped in Gallons.
-------	------------	---------------------------

Apr. 11	5	300 000
12	12	480 000
13	5	220 000
14	7	360 000
15	12	480 000
16	6	340 100
17	7	360 200
18	6	340 100

3 408 600 Total.

Data for Pennsylvania Substation.

Diameter of cylinders ----- 14 inches.

Length of Stroke ----- 18 "

Strokes per Min. ----- 44 "

Kind of pump----- Double.

Displacement of pump.

$$\frac{3.1416 \times 14 \times 14 \times 44 \times 1 \times 18}{4 \times 171} = 43 700 \text{ gallons.}$$

An allowance of 5% was made for slippage.

$$43 700 \times .05 = 2 185 \text{ gallons.}$$

$$43 700 - 2 185 = 41 515 \text{ gallons pumped per hour.}$$

Record for the North End Substation.

Diameter of cylinder----- 18 inches.

Length of Stroke ----- 18 "

Strokes per Min. ----- 35 "

Slip ----- 5'

Triplex Pump.

Displacement.

$$5.1416 \times 10 \times 10 \times 10 \times 5 \times .00 = 25,700 \text{ Gallons per hour.}$$

$$4 \times .01$$

$$\text{Slip } 5' , \text{ or } 700 \times .00 = 1,750 \text{ gallons.}$$

$$1,750 = 1.00 = 17,250 \text{ gallons per hour.}$$

This station runs ten hours per day, therefore 171,750 gallons is the amount pumped per day.

Average per Day at Stations.

High Station----- 1,411,000 gal.

Pennsylvania----- 501,000

Serial Ind ----- 561,100

Total ----- 1,973,100

Amount per capita per day,

$$1,973,100 = 65.9 \text{ gallons per capita per day.}$$

The results as found are quite satisfactory for a city situated as Lansing; i.e. As a city having a population and location as Lansing, 65 gallons per capita should furnish an ample supply especially for many dry weather domestic supply from their own wells and soft water cisterns. But it must be also born in mind that this test was run in the spring, the weather being quite mild, therefore it might fall short during the hot summer months.

Quality.

The quality of the water is one of the most important topics in investigating the water supply of a city, and it is of vital importance in its health and welfare.

The qualities of water that users are most interested

in are:

- 1st. Its hortatory quality.
- 2nd. Its attractiveness.
- 3rd. Its hardness.
- 4th. Its temperature.

This is summarized in Doctor Jolje's definition, "An ideal water is clean, tasteless, gives no irritation in action, contains a small amount of calcium and sulphur salts, has no soluble solid gas, and has a temperature of about 15° Fahr."

The evaluation of water is of little value unless the kind of water is known, it must be kept in mind that the hortating rate is dry-ground water. The following analyses, both chemical and bacteriological were performed by Dr. H. H. Holm, a state bacteriologist. The water for these tests was taken from taps in the system so that fair conditions should be met with. One sample was collected in the spring of 1911 and the other late in the summer of 1912.

The results of these analyses show that under normal conditions, the water is perfectly satisfactory to all concerned. Its attractiveness is excellent. Its hardness is a little excessive making it unsatisfactory for laundry and boiler purposes.

Water from deep wells possesses entirely different characteristics and cannot be regarded in the same category as that from shallow wells. Usually such wells penetrate alternate layers of porous and impervious materials, or rock and are supplied by the rainfall on remote catchment areas.

As a rule the water from such wells is practically sterilized by a prolonged and slow process of filtration. A visual survey can scarcely be made of the catchment area, except where the geological structure of the surrounding territory is known by extended surveys. And a bacterial analysis of the water is scarcely necessary because prolonged filtration deprives the water of bacterial life. A chemical examination is practically all that is necessary, and this is to determine the amount and character of the dissolved mineral matter.

Analysis of City Water Collected at State Capitol.

Color-----	None.
Odor-----	None.
Turbidity-----	None.
Lead-----	None.
16 as Free ammonia-----	.010
16 as Ammonium Nitrogen -----	.010
16 as Nitrites -----	None.
16 as Nitrate -----	None.
Glycerine-----	1", 000 per
Oxygen Consumed -----	.0100 Million.
Alkalinity-----	0.1, 000
Hardness-----	1", 000
Dissolved Calcium-----	0.1, 000
Iron -----	.000

Bacteriological Analysis.

Colonies per cc. at Room Temp.-----	10.
" " " Inc. " -----	"

Presumptive Tests for E.Coli.

Ure Production on Lactose L.

1cc.-----	None.	Pet. Ureum L.-----	None.
1cc.-----	None.	E. coli -----	None.
Viscosity-----	None.	Potability -----	Fusobac.
Turbidity-----	Barred P.	Iodine Production-----	None.

This sample was taken on the second floor of the Capitol, drawn from the city tap after allowing the water to run about 5 minutes, and the water was still found to be warm, indicating that the water then coming had been for some time in the pipes of

THE BUILDING.

The building shown above (the Lucy Chapel) is the one at the bottom left of the original sketch of the town of New Haven to be built in 1650. That is, it is the one at the top of the sketch, and appears to be the largest building there. It is 30' wide by 40' long, and has a gabled roof. The following are approximate figures for its dimensions.

The front door is 3' wide, the main entrance, or porch, has a width of 12' and a depth of 8'. The front door is 3' wide, and the side doors are each 2' wide. The porch is 12' wide, and the entrance is 8' wide. The porch is 12' wide, and the entrance is 8' wide. The porch is 12' wide, and the entrance is 8' wide.

Figure 22 shows a plan of the building of Fig. 21.

Room	Length	Breadth	Height
Living room	12'	8'	8'
Kitchen	8'	6'	6'
Bedroom	8'	6'	6'
Bath	4'	4'	4'
Porch	12'	8'	8'
Total area	120'	64'	80'

THE GARDEN FENCE AND GATE.

Length	Breadth	Height	Material
12'	4'	6'	Wood and wire mesh
8'	4'	6'	Wood and wire mesh
8'	4'	6'	Wood and wire mesh
8'	4'	6'	Wood and wire mesh
8'	4'	6'	Wood and wire mesh

FIRE SERVICE.

To provide sufficient fire protection in a city the size of Lansing at the present time, the hydrant size and the water pressure should be such that fifteen streams of water would be available at any one time, discharging 1.73 gallons of water per minute in business and 1.56 gallons in residential districts.

To give streams of this size with nozzles 1 1/2 inches in diameter, such as are now used, would require a pressure of at least 25 pounds for 175 gallon streams. This with 400 feet of hose would require a hydrant pressure of at least 57 or 70 pounds to take care of the loss of 1 or 10 to 11 pounds incurred by friction in the hose.

For the 300 gallon streams, the pressure at the nozzle should be 47 pounds per square inch, and the loss for the hose from 11 to 14 pounds, making a hydrant pressure of at least 100 to 110 pounds.

These pressures refer to those obtained when the streams are flowing.

I find that as far as pressure is concerned, it is ample for the conditions expressed as the main carry pressure must be constant at 70 pounds, while the entrained air pressure is gotten from the auto fire engines, which have been recently installed in the department.

The auto fire engines in the Lansing Fire Department are the latest development in the fire fighting line, and in a level city like Lansing, they are particularly effective.

There are two engines, manufactured by the Webb Motor Fire

Apparatus to collect carbon dioxide will be provided
from hydrants with 20 pounds pressure and 10 pounds with hydrants
having the water at rest. Glass or glassware will be used for
ordinary filtration, but some configurations in a medium where fine
^{1/2}
hydrants of diameter 1/2" are available.

The number and distribution of these hydrants within the
city limits are not yet determined, but as they have not been
located in the supply area, there are no difficulties in obtaining
them from among the private wells.

As indicated, apparatus will consist of a small fire boat
and the following equipment and supplies:

Equipment and Tools Used in Work:

Fire Department Equipment - Hydrant Removal - Tools:

Two sets each of the following tools and supplies for use:

1	1" x 10' x 12' 1/2"	10'
1	1" x 10' x 12' 1/2"	10'
1	1" x 10' x 12' 1/2"	10'
1	1" x 10' x 12' 1/2"	10'

Each set consisting of a 1" x 10' x 12' 1/2" board, 10' long
by 10' wide by 12' high, two 1" x 10' x 12' 1/2" boards, 10' long
by 10' wide by 12' high, one 1" x 10' x 12' 1/2" board, 10' long
by 10' wide by 12' high, one 1" x 10' x 12' 1/2" board, 10' long
by 10' wide by 12' high.

For each procedure there will be one operator, three assistants
and the following apparatus and materials which will be provided
except those furnished by the fire department:

In conclusion of this test, this structure will be taken up
with the view of improving it.

Test of Auto Fire Engine on Banking Fire Dept.

Location of Test ----- Michigan Ave., and Capitol.

Size of main----- 8 inches.

Size of Hydrant barrel ---- 4 inches.

No.	Length of Lines on Fire	Nozzle size	Nozzle Press.	Engine Press.	Discharge in gallons per minute
1	2	7/8" 26.	1 1/2"	100 "	750
2	2	"	"	200 "	1100
3	2	"	"	250 "	1250
4	2	Siamese.		300 "	1300
5	2	"	1 1/2"	300 "	1300
6	2	"	1 2/3"	300 "	1300
7	2	"	2"	300 "	1300

Location of Test----- River St. and Monroe St.

Size of main----- 8 inches.

Size of Hydrant barrel---- 4 inches.

No.	Length of Lines on Fire	Nozzle size	Nozzle Press.	Engine Press.	Discharge in gal. per minute.
1	5	100 ft.	1 1/2" 26.	100 "	1600
2	5	"	2"	50 "	1100

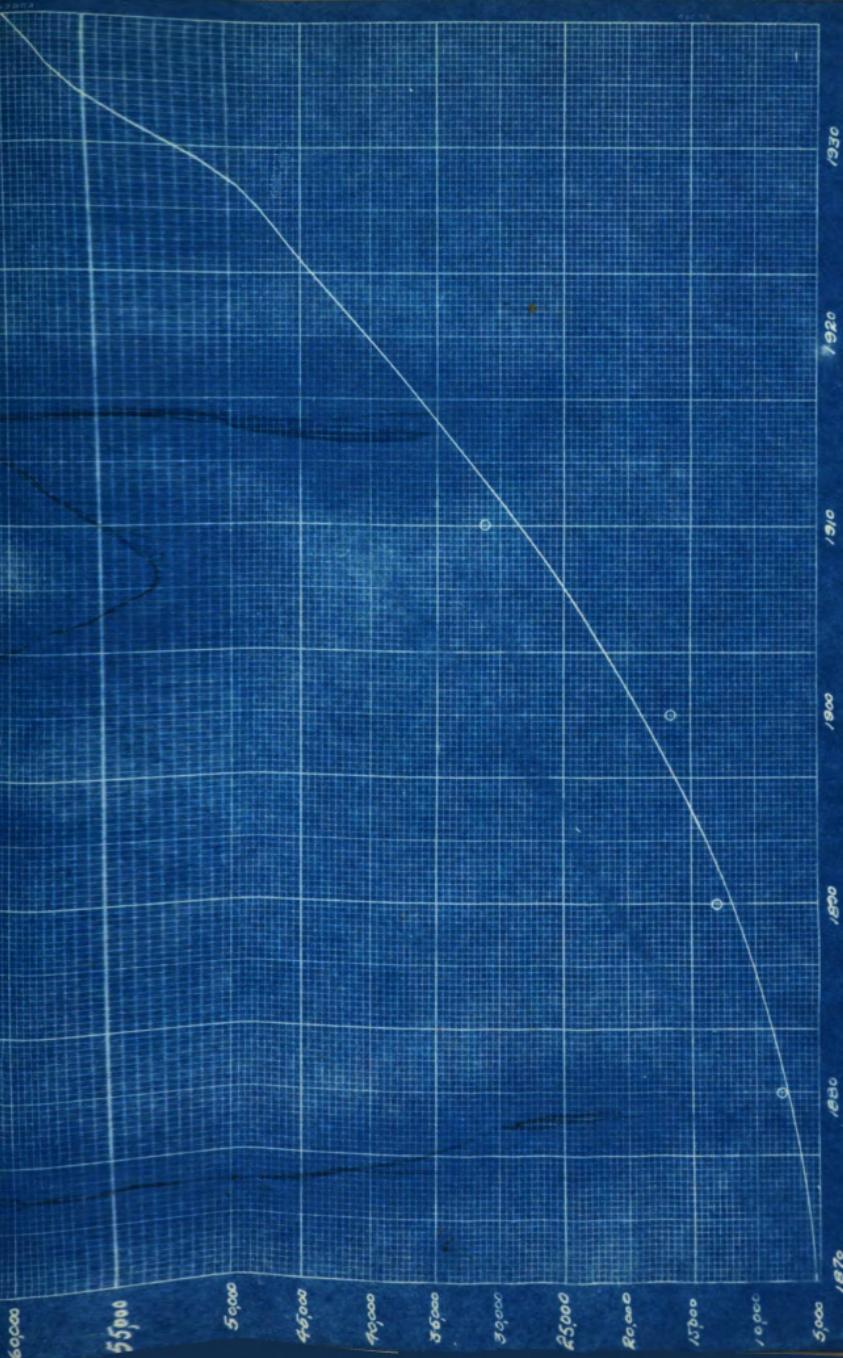
These tests show that with the aid of auto fire engines pressures of sufficient magnitude can be obtained at nearly all fire hydrants.

Population.

The population of Lansing has been very rapid in growth in the last ten years of city's development. This is shown by the population curve which accompanies this thesis.

This rapid growth, which is due to the large manufacturing interests which are being developed in Lansing, is causing the

POPULATION CURVE



city to be built up, and the coming of new additions to the city which will in a short time will have to be brought within the city's limits. This adding to the city will in a very short time overtax the extension of the present water system and will be another drain upon the already limited water supply of the city.

The extension of the system into these additions will also necessitate the operation of existing stations for their entire supply, or as to obtain the required pressure for good service, especially for fire protection.

This extension should be done with the view of further extension, that is larger mains should be laid which will carry to the distribution system the amount of water required without a large loss due to friction, which would be incurred by extension of the 6 inch mains which are now established in the outlying districts.

Administration.

The Water Works system of Lansing is associated with the Public Light system, being managed by a City Water and Light Board. The members of this board are appointed by the Mayor and confirmed by the council. The commissioner is appointed each year for a period of six years. The commission is strictly non-partisan and serves without pay.

Bates.

The entire revenue derived by the water works system is taken from the special tax levied upon the consumers or by the sale and special tax where meter are used.

A uniform rate has been established as follows.

Annual unmetered per year,

Sink and washstand-----	\$ 4.00
(General Purposed.) 18	

Closet-----	\$ 2.50
Bath -----	2.50
Lawn -----	2.00

When sold by meter rates, the cost is:

1000 gallons or less -----	.40 per thousand.
10 000 gal. to 50 000 gal.-----	.10 " "
50 000 " "100 000 " -----	.08 " "
1 000 000 gallons -----	.01 " "

The use of meters should be established especially in a system of this sort as rapidly as possible, as it is the best possible manner to prevent waste of water by consumers as well as making a uniform expense, the consumers paying for the water they use.

In Lansing the use of meters is especially desired, as the supply of water is limited, and should be used to the best possible maximum.

Finances.

The money with which the Lansing system was constructed was furnished by bond issue in the spring of 1885 when the system was organized.

The original bonded indebtedness was \$ 125 000.00 . Of this indebtedness, bonds to the value of \$ 75 000 have matured and been paid out of the earnings of the system, leaving bonds outstanding to the amount of \$ 50 000 unmatured.

The construction account up to June 1, 1890 was:

Main pipe line, 71 miles-----	\$ 320 701.61
6320 Service connections-----	63 632.41
<u>2035 Meters-----</u>	<u>40 538.36</u>

Real Estate -----	\$ 101 0007.17
Pumps, etc	
34 Wells -----	\$ 26 000.00
Total-----	\$ 1 957.63

A conservative estimate of the value of the system would be \$ 500 000.00 .

Conclusion.

As a result of our investigation we find that the water system at the present time fulfills the requirements as to quality, but the quantity is an item which must be considered chiefly in the further development of the Lansing system.

As shown by the consumption dealt with in this thesis, that the amount used per capita during the spring months amounts to 65 gallons. This amount falls within the amount that good authorities place in good practice but it is very probable that the maximum used during the summer months should approach 100 gallons per capita.

The amount that should be supplied is two times the average consumption plus the maximum fire service demand. This maximum is never fulfilled as the pressure in the outlying districts drops during the heavy consumption hours so that the domestic service is greatly impaired, to say nothing of the possible fire demand.

This feature we will attempt to discuss by the suggestion and criticism of two possible methods of improvement.

The first is the installation of a high pressure system as designed by Fossors C. A. E. Co and R.C. Ellis, in their 1911 Thesis report. This system as designed as being now under consideration by the Lansing Water Board.

This high pressure system is to be installed for public use only, and the water to be pumped directly into the system and drawn from the Grand River, which would be an inexhaustible supply.

The use of this system would greatly reduce the demands upon the present system, the supply from which would be used entirely for domestic purposes, thus saving to the people of Lansing their present water supply for drinking and cooking, as it could not be improved in quality.

The installation of this system as now proposed is to be laid by laying it first in the main business streets of the city, as a nucleus, and by extending a small amount each year, gradually completing the system. In the building of the system by this method, it relieves the necessity of incurring another bonded indebtedness, as the cost will be covered by the earnings of the systems and by special tax, which would be relieved by decreased insurance rates which would surely follow the installation of such a system.

The one serious fault of installing such a system by small pieces is that in all probability, that unless carried out under the direction of one engineer, it would not be laid in accordance with good practice. By this we mean the careful consideration and locations of principle mains followed by laterals and the distributing system which would give to the city the best possible results for the least expended amount, whereas if the system was entirely laid under properly executed plans and specifications it would immediately relieve the present situation and become a source of satisfaction to its users.

We have this advantage that we have available nearly all cities
desirous to file a petition off the river. I would consider cities as one.
The ones that would have to file would be those that supply
domestic water. New Haven is one of them. I would include those two
systems.

Another feature which is a pollution of a little more
serious character than the industrial waste is a sewage system
of course. This sewage system is now being treated at a plant in
the city of New Haven, and the 113 acre foot treatment plant, which will
have to be located in the city, will be the first stage of treated eff.

On the question of health, the first consideration of such a
system is a question that should be considered by the people of
New Haven in consideration of their system.

The other method which we will not take is taking the first
step in the installation of a filtration plant as important of the
immediate future.

The installation of such a plant comes from the fact that a
large city or city like New Haven must be equipped for treating its sewage
and the facilities can be set up in a short time. The
overhead cost of such a filtration plant is quite a bit more
than the cost of a plant to which you will have to add the cost of
treating. We might find it easier to start small in the river, and
its cost will be considerably less.

For the first two years I would propose to go ahead and construct the build-
ings and the equipment, and then start operating and gradually increase
and expand our facilities. The cost of 150,000 dollars over this
period of time is a sum of money which is reasonable which relates
the river at every street intersection.

the condition that it is a common service of American cities,
is a hardship to the city, & state officials have to correct it.

The question arises, could not the pollution waters down in the
river channel just be used for irrigation? This can be answered, but
for this purpose the stream must be converted and no water will
be available for other purposes. If the removal of silt
obstructs, the navigation will fail.

Other data, the location, a typical analysis of the water
etc. will be furnished.

San Joaquin River water.

This water may be taken from the San Joaquin River above "Middle
Ferry" at 11 miles.

	per cubic foot	per acre foot
22° salinity	0.000000	0.000000
Barium	0.000000	0.000000
Boron	0.000000	0.000000
Cadmium	0.000000	0.000000
Chlorine	0.000000	0.000000
Chromium	0.000000	0.000000
Copper	0.000000	0.000000
Dissolved oxygen	0.000000	0.000000
Ferrous	0.000000	0.000000
Iron	0.000000	0.000000
Manganese	0.000000	0.000000
Nickel	0.000000	0.000000
Potassium	0.000000	0.000000
Sodium	0.000000	0.000000
Sulfur	0.000000	0.000000
Tellurium	0.000000	0.000000
Thermal capacity	0.000000	0.000000
Zinc	0.000000	0.000000

It will be noted in this table that the first four items of analysis
are zero. The fifth item is also small, probably negligible, for
there is no appreciable iron in the San Joaquin, and the last seven items
are very minute. The remaining four items are all within
the normal range of values.

In general, it is found that the salt water, calculated as
the average of the first three, has little influence on chlorine
trioxide formation, the maximum difference being about 10% and
the minimum difference less than one per cent in chlorine.
The presence of chlorine and bromine is a favorable sign of the

absence of sewage in solution.

The filtration plant would be built in two parts. The first or sedimentation plant, to separate all solid matter from the water that would tend to clog the filter beds. The use of a coagulant would necessarily be called for to collect the suspended matter and for softening the water for domestic purposes.

In the suggestion of such an improvement as the filtration plant, we take into consideration these items:

It would be a permanent supply.

The centralization of the entire system could be made as efficient as possible.

The use of satisfactory water for all purposes would be secured.

One system would make the least complication possible.

In the present system there are several criticisms to be made. The most prominent is the number of dead ends contained in the system. No regular cross lateral plan seems to have been followed out. The lines are merely extended along a street as far as needed, and there are allowed to terminate. We are informed that the lines are regularly flushed, but even if flushed daily, the water could be nothing but unsatisfactory owing to the corrosion of the iron mains.

The lack of data and notes of the present system is quite apparent. The knowledge of the system seemingly to be vested in the memory of some of the officials.

No knowledge of the consumption of the water is known by the officials. An approximate amount is determined by records at the main station, but in checking their data, we found that errors must necessarily be introduced by the non-recognition

of slippage. The water from the evapotranspiration is turned directly into the drain and thus is available for infiltration. The infiltration of the rainfall should be quite rapid.

The infiltration of rainfall is dependent upon the soil's water holding capacity. If the soil has a low water holding capacity, infiltration will not be strong. If the soil has a high water holding capacity, infiltration will be strong.



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